PETERSON I PURITAN OU2 SLOPE STABILITY ANALYSIS

CERCLA Docket No. 1-87-1064 CUMBERLAND, RHODE ISLAND

Prepared for:

Shield Environmental Associates, Inc 2456 Fortune Drive, Suite 100 Lexington, Kentucky 40509

Prepared by:

4301 Taggart Creek Road Charlotte, NC 28208 Project Number 1030105

January 29, 2004

1.0 LIMITATIONS

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Our conclusions and recommendations are based upon our site observations, provided survey and provided field data. We have assumed that information provided to us by others is correct and true, unless otherwise noted. If additional information or changes in information is available in the future, we request the chance to review and change our recommendations, if necessary.

The Slope Stability Analysis was prepared under my direct supervision:

By:

Thomas M. Vick, P.E.

Principal Engineer

I assisted with the preparation of this project:

By: John M. Long

StaffEngineer

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Attachment A: Slope Stability Geometry Attachment B: Slope Stability Data and Graphical Output

1.0 LIMITATIONS

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By: *Homas M. Vick, P.E.* JAN 2 9 2004

Principal Engineer

I assisted with the preparation of this project:

By: ohn M. Long

Staff Engineer

2.0 INTRODUCTION

Shield Engineering, Inc. (Shield) is pleased to provide slope stability analysis of the J. M. Mills Landfill located in the Peterson/Puritan OU2 site in Cumberland, Rhode Island. The slope-stability analysis was preformed on both sides (slopes) of the landfill using GSTABL 7 with STEDwin©. In-situ testing was performed by CONETEC of West Berlin, New Jersey. The results were presented in their "Field Report for the Peterson/Puritan OU2 Superfund Site, Cumberland, Rhode Island" dated September 18, 2003. We understand this report is included elsewhere in the DBSR. Nine (9) cone penetration tests (CPTs) were performed at various locations around the landfill. The primary purpose of this testing was to establish strength and lithologic characteristics of native (including alluvial and/or fill materials in and near the landfill) and waste materials found onsite. Figure I illustrates the site layout and general orientation of the landfill and Figure 2 illustrates the location of the CPT test borings.

3.0 FIELD TESTING

CPT testing indicated that native lithology was relatively homogeneous. Native material consists of predominately sand with some silt. There appears to be some minor bedding in the sand and silty sand layers. Layers are thin and the strength of the bulk material relies on the larger grained matrix material, sand. The material is likely alluvial in origin, based on the close proximity of the Blackstone River. This is consistent with the boring logs provided to us by Shield Environmental Associates, Inc., of Lexington Kentucky. The material in the landfill itself appeared to be heterogeneous, as expected. Shear strength values for the landfill materials were very erratic resulting in a range of engineering properties.

Onsite observations provided insight to the stability of the landfill. A few of the observations made onsite were:

- The landfill is standing without any lateral support (it is free standing)
- The landfill had little to no cover soil (varies by location)
- The slopes range from an estimated 2:1 to 3:1
- The landfill seems to contain large voids, consistent with anticipated conditions
- Materials included in the landfill appear to be a combination of construction and demolition (C&D) waste and municipal solid waste (MSW)
- High temperatures at the bottom of the landfill indicated that organic degradation is occurring, this could be confirmed by methane monitoring
- Surface failure was not clearly identified (due to heavy vegetation) however, topographic data indicates that localized failures may have occurred
- No cracks were observed at the top of the landfill

Soil and landfill testing by CONETEC consisted of approximately 355 linear feet of boring data. In each location probes were pushed until refusal was encountered. Pore pressure dissipation tests were performed in each of the applicable locations. The water table was intersected during six of the nine tests. The CONETEC report includes basic principles of cone technology, testing results in graphical form, results in numeric form along with calculations and pore pressure dissipation test data.

The locations of the borings are depicted on Figure 2. Boring locations were chosen based on the geometry of the landfill. Site restrictions did not allow for a CPT sample to be collected along the railroad tracks. Subsurface strata were assumed to be continuous from data collected northwest and southeast of the landfill, CPT-7 and CPT-9, respectively.

4.0 ANALYSIS

In order to perform the stability analysis it was necessary to make assumptions, based on site-specific project information and on-site observations. A sensitivity analysis was preformed on the phi angle (ϕ) , also known as the internal friction angle) and the unit weight (y) of the landfill material. The result of the sensitivity analysis indicated that the unit weight was less of an influential factor than the phi angle in the stability of the landfill. Experience, engineering judgement and comparison to observed field conditions were used whenever possible to establish the most representative analysis.

In this case, two modes of slope failure are likely. The first is global failure of the landfill. Global failure typically occurs along deep circular failure surfaces and is characterized by massive slope failures. The second failure mode is surface failure. Evidence of surface failure is typically seen in the form of slumping and/or subsidence along the surface of the slope. Evidence exists for surface failure based on topographic data and field observations. Saturation of the upper layer of waste materials during a precipitation event may cause the material to become heavier than the underlying material causing surface instability.

The slope stability analysis was performed using the *Simplified Bishop Method* of slices to discretize the soil mass to establish the minimum factor of safety of the slope. The method satisfies vertical force equilibrium for each slice. The moment is then calculated about the center of the trial surface. One of the main assumptions of the Bishop method is zero interstice shear force. The failure surface is also assumed to be circular. The Bishop method is one of the most widely used slope stability methods. Factors of safety (FS) have been calculated to be within 5% of Janbu's formulation (a more rigorous set of assumptions).

In order to assess the stability of the slope, made of unknown materials, data was taken from the results of the CPT testing and compared to hypothetical conditions. Sensitivity of the parameters γ and ϕ was assessed. Data indicated that as unit weight increased the factor of safety increased. It should be noted that dry and wet unit weight affect the stability differently. Typically saturated materials have less strength than like dry materials. The sensitivity analysis also indicated that as the internal friction angle increased the factor of safety increased. This is consistent with previously held notions. Table 3.1 below presents the sensitivity of the dry/wet unit weight and Table 3.2 shows the sensitivity of the internal friction angle.

To access a conservative, likely range for the unit weight and internal friction angle the values of $\gamma_{\text{dry}} = 55$ pcf, $\gamma_{\text{sat}} = 65$ psf and 22.5° were used, respectively as an overall indicator of stability. Using these values, a factor of safety of approximately 1.1 is realized. The FS of 1.1 should not be considered comprehensive as the waste material is heterogeneous and will certainly vary from location to location. A range of FS should be considered in this situation along with a range of the parameters γ and ϕ . Likely ranges of phi vary between 20° and 45°, making the FS vary between 1.0 and 2.3.

Figure 3 illustrates the location of the two (2) slope sections that were analyzed for slope stability. The slopes were chosen based on proximity to boring locations and also areas of steep slope to yield worst case scenario conditions. The first section runs from the top of the landfill towards Blackstone River. The second section line runs from the top of the landfill into the wetlands or swampy area. The water table was assumed to somewhat follow the topography in the landfill. Attachment 1 includes the landfill slope sections analyzed. Attachment 2 contains the slope stability data and graphical outputs.

Table 3.1

γ_d/γ_w (psf)	Φ (degrees)	Factor of Safety
25/35	21	0.48
35/45	21 TEVER	0.75
45/55	21	0.88
55/65	21	0.96
65/75	21	1.02
75/85	21	1.03
85/95	21	1.03
105/115	21	1.03

Table 3.2

4.0 CONCLUSIONS AND RECOMMENDATIONS

Slope strength is traditionally gauged by the resulting factor of safety (FS). Due to the inherent nature of landfill-type material, engineering judgement was considered throughout the project. The fact that the landfill appeared to be relatively stable was considered along with the fact that the strength of the slope directly depends on the material contained within. Interlocking steel beams could provide significant increases in the FS, however, organic materials may appear strong until degradation occurs, then lowering the FS. Since this landfill appears to be a combination of materials it is necessary to consider both scenarios.

The results of the analysis, using conservative input values to the model indicate that the overall landfill slopes are stable and may be for some period, however, some localized failures have likely occurred. These rather conservative input values do result in FS(s) lower than typical minimum FS of 1.5 required for water retention by earthen embankments. However, acceptable FS(s) of mounds of solid material such as landfills typically are in the range of 1.2 to 1.3. Since the landfill is free standing, the factor of safety must be equal to at least 1.0. Due to the high variability of the unit weight and the internal friction angle inherent to waste materials, it is likely that localized failure will occur in some location sometime in the future. The high temperatures near the bottom of the landfill indicate that degradation is occurring. Factors of safety will decrease with time as the organic material degrades and weakens. The native sand and silty sand appear strong enough to provide sufficient support for the landfill. Failure surfaces are most likely to occur along the interface between the waste material and the native materials and also along the surface of the slope (depending on the chosen phi angle).

Geographic considerations must be taken into account for this particular site. The close proximity of the Blackstone River could influence the global and surface stability of the landfill. During flood or other high water conditions, the erosion of the bank could scour the toe of the slope compromising the stability of the landfill. The river channel should be stabilized and scour protections should be installed to reduce this potential.

To help maintain the integrity of the landfill slopes, a few remediation techniques should be considered. One technique would be to reduce the overall side slopes of the landfill. To do this, material would have to be removed from the top of the landfill and deposited at another location or along an extended landfill embankment. A capping system should then be designed for the waste material to minimize water infiltration thus controlling the piezometer surface in the landfill material and reducing leachate generation potential (and degradation of the waste materials). Minimum slopes should be maintained to prevent sliding of the cap. If heavy equipment is used during the remediation process thought should be given to the type and weight of the equipment. Dynamic loading as a result of the heavy equipment could reduce the strength of the slope. Stability of the waste materials should be reassessed once a remedial plan is devised to ensure that the design has an adequate FS and that construction proceeds in a safe manner. Further investigation could provide specific construction details and specifications on capping systems, slope gradients and remedial alternatives.

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Based on the evidence presented herein the landfill is not likely to experience global failure. However, localized surface failures are likely, as they may have already occurred in some locations. Although the landfill materials may exhibit appreciable strength properties because of reinforcement by interlocking debris, these properties will likely decrease with time. If capping is used as a remedial technique the current steepness of the slope will cause sliding of the cover unless modifications are made.

REFERENCES

- Kulhawy, F.H. and P.W. Mayne. (1990), Manual on estimating soil properties for foundation design. Final Report 1493-6, EL-6800, Electric Power Research Institute, Palo Alto, CA.
- Robertson, P.K. and R.G. Campanella. (1983), Interpretation of cone penetration tests-Part 1: sand. *Can. Geotech. Journ.,* 20:718-733.

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Figures:

Attachment A:

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••• ••• *** GSTABL7 ***
** GSTABL7 by Garry H. Gregory, P.E. ** •• Original Version 1.0, January 1996; Current Version 2.004, June 2003 ** (All Rights Reserved- Unauthorized Use Prohibited) SLOPE STABILITY ANALYSIS SYSTEM Modified Bishop, Simplified Janbu, or GLE Method of Slices. (Includes Spencer & Morgenstern-Price Type Analysis} Including Pier/Pile, Reinforcement, Soil Nail, Tieback, Nonlinear Undrained Shear Strength, Curved Phi Envelope, Anisotropic Soil, Fiber- Reinforced Soil, Boundary Loads, Water Surfaces, Pseudo-Static & Newmark Earthquake, and Applied Forces. Analysis Run Date: 11/24/03
Time of Run: 09:37AM Time of Run: Run By: Shield Engineering, Inc
Input Data Filename: H:\Geo-Environmental (0) H:\Geo-Environmental (0120)\2003\1030105-01 Peterson-Puritan OU2\CPT\Puritan #1.in
Output Filename: H:\Geo-Environmental (0120)\2003\1030105-01 Peterson-Puritan OU2\CPT\Puritan #1.OUT Unit System: English Plotted Output Filename: H:\Geo-Environmental (0120)\2003\1030105-01 Peterson-Puritan OU2\CPT\Puritan #1.PLT PROBLEM DESCRIPTION: Cross Section **11** Petterson/Puritan OU2 BOUNDARY COORDINATES Note: User origin value specified. Add 10.00 to X-values and 0.00 to Y-values listed. 7 Top Boundaries 9 Total Boundaries Boundary X-Left Y-Left X-Right Y-Right Soil Type Boundary X-Left Y-Left X-Right Y-Right Soil Type

No. (ft) (ft) (ft) (ft) Below Bnd

1 10.00 56.00 63.00 51.00 1 ¹10 .00 56.00 63.00 51.00 ¹ 2 63.00 51.00 116.00 56.00 1 3 116.00 56.00 133.00 62.00 1 $\begin{array}{ccccccccc} 3 & & 116.00 & & 56.00 & & 133.00 & & 62.00 & & 1 \ 4 & & 133.00 & & 62.00 & & 207.00 & & 64.00 & & 2 \ 5 & & 207.00 & & 64.00 & & 261.00 & & 70.00 & & 2 \ \end{array}$ 5 207.00 64.00 261.00 70.00 2 6 261.00 70.00 521.00 140 .00 2 7 521.00 140. 00 579.00 140.00 2 8 133.00 62.00 163 .00 52.00 1 9 163.00 52.00 579.00 52.00 1 9 163.00 52.00
Default Y-Origin = 0.00(ft) Default X-Plus Value = 0.00 (ft) Default Y-Plus Value = 0.00 (ft) ISOTROPIC SOIL PARAMETERS 2 Type(s) of Soil Total Saturated Cohesion Friction Pore Pressure Piez. Type Unit Wt. Unit Wt. Intercept Angle Pressure Constant Surface
No. (pcf) (pcf) (psf) (deg) Param. (psf) No. No. (pcf) (pcf) (psf} (deg) Param. {psf) No. 1 86.0 111.4 0.0 37.0 0.00 0.0 ¹ 2 55.0 65.0 0.0 22.5 0.00 0.0 1 l PIEZOMETRIC SURFACE(\$) SPECIFIED Unit Weight of Water - 62.46 (pcf) Piezometric Surface No . 1 Specified by 5 Coordinate Points Pore Pressure Inclination Factor = 0.50
Point X-Water Y-Water oint X-Water Y-Wate
No. (ft) (ft) oint x-water r-wate
No. (ft) (ft) 1 10.00 56.00
2 156.00 56.00 *2* 156.00 56.00 261.00 4 521.00 100.00 5 579.00 100 . 00 A Critical Failure Surface Searching Method, Using A Random Technique For Generating Circular Surfaces, Has Been Specified. 2000 Trial Surfaces Have Been Generated.

1 Surface(s) Initiate(s) From Each Of 2000 Points Equally Spaced

H: Puritan #1.OUT Page

Along The Ground Surface Between $X = 10.00(ft)$
and $X = 510.00(ft)$ Each Surface Terminates Between X = 521.00 (ft) Each Surface Terminates Between $X = 521.00(ft)$
and $X = 579.00(ft)$ Unless Further Limitations Were Imposed, The Minimum Elevation At Which A Surface Extends Is $Y = 10.00$ (ft) 15.00 (ft) Line Segments Define Each Trial Failure Surface. Following Are Displayed The Ten Host Critical Of The Trial Failure Surfaces Evaluated. They Are Ordered - Host Critical First. * • Safety Factors Are Calculated By The Modified Bishop Method • • Total Number of Trial Surfaces Attempted = 2000 Number of Trial Surfaces With Valid FS = 2000 Statistical Data On All Valid FS Values: FS Max = 34.260 FS Min = 1.181 FS Ave = 2.543 Standard Deviation = 2.008 Coefficient of Variation = 78.97 % Failure Surface Specified By 22 Coordinate Points
Point X-Surf Y-Surf Point X-Surf Y-Surf No. (ft) (ft) No. (ft) (ft)
1 268.377 71.986
2 282.306 66.420 2 282.306 66.420
3 296.561 61.750 3 296.561 61.750 4 311.083 5 325.814 55.167 6 340.695 53.281 7 355.666 52.343 7 355.666 52.343
8 370.666 52.358
9 385.634 53.324 53.324
55.238 10 400.512 55.238
11 415.238 58.093 11 415.238 58.093
12 429.752 61.877 12 429.752 13 443.998 66.574 14 457.917 72.166
15 471.452 78.631 15 471.452 78.631 16 484.550 85.941 16 484.550 85.941
17 497.158 94.068
18 509.224 102.979 18 509.224 102.979
19 520.701 112.637 19 520.701 112.637
20 531.541 123.005 20 531.541
21 541.702 21 541 .702 134.040 21 541.702 134.040
22 546.528 140.000
Circle Center At X = 362.913; Y = 288.143 ; and Radius = 235.925 Factor of Safety
*** 1.181 *** Individual data on the 24 slices Water Water Tie Tie Earthquake
Force Force Force Force Force Force Force Force Force Force Surcharge Slice Width Weight Top Bot Norm Tan Hor Ver Load
No. (ft) (lbs) (lbs) (lbs) (lbs) (lbs) (lbs) (lbs) No. (ft) (lbs) (lbs) (lbs) (lbs) (lbs) (lbs) (lbs) (lbs) 0.0 1 13.9 3568.5 0.0 0 . 0 0. 0. 0.0 0.0 $2 6.5 4043.4 0.0 0.0 0.0 0.0 0.0 0.0$ $\frac{2}{3}$ 7.7 6737.3 0.0 934.0 0. 0. 0.0 0.0 0.0 ⁴14.5 18261.6 o.o 6219.3 0. 0 . 0 .0 0.0 0.0 5 14.7 25189.0 0.0 11351.4 0. 0. 0.0 0.0 0.0 $\begin{array}{cccccccccccc}\n6 & 14.9 & 31317.9 & 0.0 & 15643.1 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
\end{array}$ 14.9 31317.9 0.0 15643.1 0. 0. 0.0 0.0 0.0
7 15.0 36526.7 0.0 19077.2 0. 0. 0.0 0.0 0.0
8 15.0 40718.1 0.0 21639.8 0. 0. 0.0 0.0 0.0 0.0 8 15.0 40118.1 0.0 21639.8 0. 0 . 0.0 0.0 $\frac{9}{9}$ 15.0 43819.8 0.0 23320.7 0. 0. 0.0 0.0 0.0 10 14.9 45786.6 0.0 24112.9 0. 0. 0.0 0.0 0.0 10 14.9 45786.6 0.0 24112.9 0. 0. 0.0 0.0 0.0 0.0
11 14.7 46600.9 0.0 24013.4 0. 0. 0.0 0.0 0.0 0.0
12 14.5 46272.5 0.0 23022.4 0. 0. 0.0 0.0 0.0 12 14.5 46272.5 0.0 23022.4 0. 0. 0.0 0.0 0.0
13 14.2 44838.9 0.0 21144.2 0. 0. 0.0 0.0 0.0 12 14.3 16272.3 0.0 23022.1
13 14.2 44838.9 0.0 21144.2 0. 0. 0.0 0.0 0.0 14 13.9 42364.2 0.0 18386.1 0. 0. 0.0 0.0 0.0 14 13.9 42364.2 0.0 18386.1 0. 0. 0.0 0.0 0.0
15 13.5 38938.1 0.0 14759.3 0. 0. 0.0 0.0 0.0
16 13.1 34674.2 0.0 10278.4 0. 0. 0.0 0.0 0.0 0.0 16 13.1 34674.2 0.0 10278.4 o. 0. 0.0 0.0 0.0 $\begin{array}{cccccccccccc} 10 & 13.1 & 12.6 & 29707.7 & 0.0 & 4961.6 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\ 18 & 3.9 & 8265.9 & 0.0 & 336.5 & 0. & 0. & 0.0 & 0.0 & 0.0 \end{array}$ $0.9.$

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4 269.632 55.812
5 284.491 53.761 5 284.491 53.761 6 299.430 52.408 7 314.416
8 329.415 8 329.415 51.806
9 344.397 52.557 9 344.397 52.557
10 359.326 54.008 359.326 54.008
374.172 56.157 11 374.172 56.157
12 388.900 58.997 12 388.900 58.997
12 3.480 62.524 13 403.480 62.524 417.878
432.064 15 432.064 71.603
16 446.007 77.135 16 446.007 77.135
17 459.675 83.314 459.675 83.314
473.039 90.125 18 473.039 90.125
19 486.070 97.555 486.070 97.555
498.739 105.586 20 498.739
21 511.019 114.201 22 522.882 123.381 23 534.302 133.105 241.670 140.000
 220.846 ; $Y =$ Circle Center At X = 320.846 ; Y = 371.909 ; and Radius = 320.227 Factor of Safety
*** 1.227 *** ~·· 1.227 *** Failure Surface Specified By 24 Coordinate Points Point X-Surf Y-Surf
No. (ft) (ft) (ft) (ft)
1 262.124 70.303 262.124 70.303
276.452 65.865 2 276.452 65.865
3 290.976 62.115 290.976 62.115
305.662 59.061 4 305.662 59.061
5 320.477 56.711 320.477 56.711
335.387 55.070 6 335.387 55 .070 350.358 54.142
365.356 53.928 8 365.356 53.928 9 380.348 54.429
10 395.299 55.645 395.299 55.645
410.174 57.572 11 410.174 57.572
12 424.941 60.205 424.941 60.205
439.566 63.540 13 439.566 63.540
14 454.015 67.568 14 454.015 67.568
15 468.256 72.280 15 468.256 72.280
16 482.256 77.665 16 482,256 77.665
17 495,983 83.712 17 495.983 18 509.406 90.406
19 522.495 97.733 522.495 20 535.220 105.675
21 547.552 114.215 21 547.552 114.215
22 559.463 123.332 22 559.463
23 570.926 23 570.926 133.007 24 578.451 140.000
Circle Center At X = 362.322 ; Y = 368.232 ; and Radius = 314.327 Factor of Safety
*** 1.233 *** Failure Surface Specified By 24 Coordinate Points Point X-Surf Y-Surf Point X-Surf Y-Surf
No. (ft) (ft)
1 209.598 64.289 209.598 64.289
224.237 61.016 2 224.237 61.016
3 238.999 58.358 3 238.999 58.358 4 253.860 56.319 5 268.793 6 283.773 54.115 7 298 . 772 53 . 953 8 313.764 54.418 54.418 9 328.725 55.510
0 343.626 57.226 10 343.626

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PROFIL H:\Geo- Environmenta1 (0120)\2003\1030105- 01 Peterson-Puritan OU2\CPT\Puritan **tl.in** Versi Cross Section #1 Petterson/Puritan 002 9 7 10. 56. 63. 51. 1 63. 51. 116. 56. 1 116. 56. 133. 62. 1 133. 62. 207. 64. 2 207. 64. 261. 70. 2 261. 70. 521. 140. 2 521. 140. 579. 140. 2 133. 62. 163. 52. 1 163. 52. 579. 52. 1 0. 0. 0. SOIL Silt Trash *2* 85.96 111.43 0. 37. 0. 0. 1 55. 65. 0. 22. 5 0. 0. 1 WATER 1 62.46 5 0.5 10. 56. 156. 56. 261. 60. 521. 100. 579. 100. CIRCL2

Cross Section #2 Petterson/Puritan OU2

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*** **GSTABL7** ***
** GSTABL7 by Garry H. Gregory, P.E. ** ** Original Version 1.0, January 1996; Current Version 2.004, June 2003 ** (All Rights Reserved-Unauthorized Use Prohibited) ······•••***************•···················**********•-························· SLOPB STABILITY ANALYSIS SYSTEM Modified Bishop, Simplified Janbu, or GLB Method of Slices. (Includes Spencer & Morganstern-Price Type Analysis) Including Pier/Pile, Reinforcement, Soil Nail, Tieback, Nonlinear Undrained Shear Strength, CUrved Phi Envelope, Anisotropic Soil, Fiber-Reinforced Soil, Boundary Loads, Water Surfaces, Pseudo-Static & Newmark Earthquake, and Applied Forces. *** Analysis Run Date: 11/24/03
Time of Run: 01:48PM Time of Run:
Run By: Run By: Shield Engineering, Inc
Input Data Filename: h:\Geo-Environmental (0120)\ h:\Geo-Environmental (0120)\2003\1030105-01 Peterson-Puritan OU2\CPT\puritan #2.in
Output Filename: h:\Geo-Environmental (0120)\2003\1030105-01 Peterson-Puritan OU2\CPT\puritan #2.0UT Unit System: **English** Plotted Output Filename: h: \Gao-Environmental (0120) \2'003\1030105-01 Peterson-Puritan OU2\CPT\puritan #2.PLT PROBLBM DESCRIPTION: Cross Section #2 Patterson/Puritan OU2 BOUNDARY COORDINATES Note: User origin value specified. Add 10.00 to X-values and 0.00 to Y-values listed. 8 Top Boundaries 12 Total Boundaries
Boundary X-Left mdary X-Left Y-Left X-Right Y-Right Soil Type
No. (ft) (ft) (ft) (ft) Below Bnd No. (ft) (ft) (ft) (ft) Below Bnd 1 10.00 62 .00 451.50 62.00 3 2 451.50 62.00 452.50 66.00 3 3 452.50 66.00 455.50 66.00 3 4 455.50 66.00 456.50 64.00 1
5 456.50 64.00 491.00 72.00 2 5 456.50 64.00 491.00 72.00 2 6 491.00 72.00 703.60 136.00 2 7 703.60 136.00 . 760.60 140.00 2 B 760.60 140 .00 798.60 140. 00 2 9 456.60 64.00 466.00 54.00 1
10 466.00 54.00 798.60 54.00 1 10 466.00 54.00 798.60 54.00 1 11 10.00 52.00 440.00 52.00 1 440.00 52.00 451.50 Default Y -Origin = 0.00 (ft) Default X-Plus Value = $0.00(ft)$ Default Y-Plus Value = $0.00(ft)$ ISOTROPIC SOIL PARAMETERS 3 Type(s) of Soil Soil Total Saturated Cohesion Friction Pore Pressure Piez. Type Unit Wt. Unit Wt. Intercept Angle Pressure Constant Surface No. (pcf) (pcf) (psf) (deg) Param. (psf) No. (pcf) (pcf) (psf) (deg) Param. (psf) No. 1 86.0 111.4 0.0 37.0 0.00 0.0 ¹ $2 55.0 65.0 0.0 22.5 0.00 0.0 1$
3 43.0 89.0 0.0 15.0 0.00 0.0 1 3 43.0 89.0 0.0 15.0 0.00 0.0 1 1 PIEZOMETRIC SURFACB(S) SPBCIFIBD Unit Weight of Water = 62.46 (pcf) Piezometric Surface No. 1 Specified by 4 Coordinate Points Pore Pressure Inclination Pactor = 0.50
Point X-Water Y-Water oint X-Water Y-Water
No. (ft) (ft) No. (ft) (ft) 1 10.00 61.00
2 456.50 63.00 2 456.50 63.00 3 703.60 85.00 4 798.60

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Circular Surfaces, Has Been Specified. 2000 Trial Surfaces Have Been Generated.

1 Surface(s) Initiate(s) From Each Of 2000 Points Equally Spaced

Along The Ground Surface Between $X = 210.00(ft)$

and $X = 700.00(ft)$

Each Surface Terminates Between $X = 710.00(ft)$ and $X = 798.00(ft)$

Unless Further Limitations Were Imposed, The Minimum Elevation

At Which A Surface Extends Is $Y = 10.00(ft)$

15.00(ft) Line Segments Define Each Trial Failure Surface.

**** ERROR - RC11 ****

>>200 attempts to generate failure surface have failed. Revise limitations The Factor Of Safety For The Trial Failure Surface Defined By The Coordinates Listed Below Is Misleading.

Failure Surface Defined By 25 Coordinate Points

Factor Of Safety For The Preceding Specified Surface = 2.766 Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Evaluated. They Are

Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Bishop Method * * Total Number of Trial Surfaces Attempted = 2000 Number of Trial Surfaces with Misleading $FS = 1$ Number of Failed Attempts to Generate Trial Surface = 1001 Number of Trial Surfaces With Valid FS = 998 Percentage of Trial Surfaces With Non-Valid FS Solutions of the Total Attempted = 50.1 % Statistical Data On All Valid FS Values:

FS Max = 3.894 FS Min = 1.172 FS Ave = 2.265 Standard Deviation = 0.373 Coefficient of Variation = 16.46 % Failure Surface Specified By 22 Coordinate Points

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123.733 726.079 21 133.077 22 737.813 23 744.469 138.868 517.941 ; $Y = 397.185$; and Radius = 343.652 Circle Center At $X =$ Factor of Safety 1.246 *** *** Failure Surface Specified By 25 Coordinate Points $Y-Surf$ Point X-Surf (f_{td}) (f_t) No. 62.000 401.682 ${\bf 1}$ $\overline{2}$ 416.497 59.653 57.809 $\overline{\mathbf{3}}$ 431.383 56.469 446.323 4 5 461.300 55.635 476.296 6 55.308 $\overline{7}$ 491.295 55.488 56.176 506.279 8 57.370 $\mathbf{9}$ 521.232 59.069 10 536.135 61.271 550.973 11 12 565.727 63.973 67.173 580.382 13 594.920 70.866 14 75.050 609.325 15 79.717 16 623.580 84.865 17 637.670 90.486 651.577 $1B$ 96.574 19 665.286 678.781 103.122 20 21 692.047 110.122 705.069 117.568 22 23 717.831 125.449 730.320 133.758 24 25 736.703 138.323 478.463 ; $Y = 498.785$; and Radius = 443.482 Circle Center At $X =$ Factor of Safety *** *** 1.247 Failure Surface Specified By 26 Coordinate Points Point X-Surf Y-Surf (f_t) (f_t) No. 385.749 62.000 ${\bf 1}$ 59.591 $\overline{\mathbf{z}}$ 400.554 57.665 $\overline{\mathbf{3}}$ 415.430 56.225 4 430.361 5 445.331 55.272 54.806 6 460.323 54.829 $\overline{7}$ 475.323 8 490.315 56.339 9 505.281 10 520.208 57.824 535.078 59.795 11 12 549.875 62.249 65.183 564.586 13 68.595 14 579.193 72.480 593.681 15 76.836 16 608.034 17 622.239 81.656 636.279 86.936 18 92.670 19 650.140 98.852 20 663.806 21 105.476 677.264 690.500 112.535 22 120.021 23 703.498 127.926 24 716.246 728.730 136.242 25 26 731.092 137.929

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Cross Section #2 Petterson/Puritan OU2

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Slice No. 1 *2* 3 4 5 6 7 7 691.695 131.878 B 695.939 133.694 Circle Center At $X = 524.163$; $Y = 540.505$; and Radius = 441.591 Factor of Safety
*** 1.383 *** Individual data on the 7 slices Water Water Tie
Force Force Force Force Force Width Weight Top Bot Norm Tan (ft) (lbs) (lbs) (lbs) (lbs) (lbs) 14.7 567.7 0.0 0.0 0. 0. 14.6 1479.6 0.0 0.0 0. 0. 14.5 1956.9 0.0 0.0 0. 0. 14.3 2008.4 0.0 0.0 0. 0. 14.2 1646.1 0.0 0.0 0. 0. 14.0 885.4 0.0 0.0 0. 0. 4.2 62.8 0.0 0.0 0. 0. Tie Barthquake Force Surcharge
Tan Hor Ver Load Norm Tan Hor Ver Load
(1bs) (1bs) (1bs) (1bs) (1bs) (1bs) (lbs) (lbs) 0.0 0.0 o.o 0.0 0.0 0.0 0.0 0 . 0 0.0 o.o 0.0 0.0 0.0 0.0 o.o 0.0 0.0 0.0 0.0 0.0 0.0 4.2 62.8 0.0 0.0 0. 0.
Failure Surface Specified By 8 Coordinate Points
Point X-Surf Y-Surf Point X-Surf Y- Surf No. (ft) (ft) 105.770 1 603.178 105.770 2 617.893 108.681 3 632.502 112.082 4 646.991 115.966 5 661.342 120.331 6 675.539 125.170 7 689.568 130.480 8 700.741 135.139
Circle Center At X = 523.683 ; Y = 546.573 ; and Radius = 447.914 Factor of Safety $\begin{array}{ccc}\n & \text{Factor of } & \text{Safety} \\
\hline\n & \text{1.384} & \text{1.47}\n\end{array}$ Failure Surface Specified By 8 Coordinate Points
Point X-Surf Y-Surf Point X-Surf Y-Surf
No. (ft) (ft) 1 602 . 517 105.571 2 617.228 108.504 3 631.820 111 .980 4 646.273 115.994 5 660.567 120.541

6 674.683 125.614

7 688.601 131.206

8 690.532 132.066

Circle Center At X = 531.295 ; Y = 501.596 ; and Radius = 402.379 Factor of Safety
*** 1.384 *** Failure Surface Specified BY 8 Coordinate Points Point X-Surf Y-Surf
No. (ft) (ft) $\begin{array}{cccc} 1 & 605.045 & 106.331 \\ 2 & 619.775 & 109.166 \end{array}$ 2 619.775 109.166 3 634.378 112.591 4 648.832 116.603 5 663.112 121.194 6 677.196 126.357 $7 691.059 132.084$ 8 691.942 132.490 8 691.942 132.490
Circle Center At X = 542.418 ; Y = 471.781 ; and Radius = 370.776
Factor of Safety *** 1.385 *** Failure Surface Specified By 8 Coordinate Points
Point X-Surf Y-Surf Point X-Surf Y-Surf No. (ft) (ft) (f_t) No. (ft)
1 608.683
2 623.417 107.427 2 623.417 110.240 3 638.035 113.606 4 652.515 117.520

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5 666.837 121.978 $\begin{array}{cccc} 6 & 680.982 & 126.972 \\ 7 & 694.928 & 132.495 \end{array}$ 094.928 132.495
701.347 135.322 8 701.347 135.322
Circle Center At X = 542.414 ; 1 Circle Center At X = 542.414 ; Y = 495.423 ; and Radius = 393.615 Factor of Safety
*** 1.385 *** Failure Surface Specified By 8 Coordinate Points Point X-Surf Y-Surf Point X-Surf Y-Surf
No. (ft) (ft)
1 608.258 107.29 107.299 1 608.258 107.299
2 622.975 110.197 2 622.975
3 637.589 113.580
117.445 4 652.082 117.445 5 666.440 121.788 6 680.646 126.603 694.685 131.886
704.811 136.085 8 704.811 136.085
Circle Center At X = 528.888 ; Y = 549.839 ; and Radius = 449.602 Factor of Safety
*** 1.385 *** Failure Surface Specified By 8 Coordinate Points Point X-Surf Y-Surf Point X-Surf Y-Surf
No. (ft) (ft)
1 609.489 107.66 1 609.489 107.669 2 624.224 110.475
3 638.847 113.821 3 638.847 4 653.336 117.702 $5 667.673$
681.838 6 681.838 127.047 7 695.812 132.499 8 703.952 136.025
:le Center At X = 541.469 ; Y = Circle Center At $X = 541.469$; $Y = 505.796$; and Radius = 403.896 Factor of Safety
*** 1.385 *** Failure Surface Specified By 8 Coordinate Points Point X-SUrf Y-Surf Point X-Surf Y-Surf
No. (ft) (ft)
1 602.472 105.557 602.472 105.557
617.224 108.278 2 617.224 3 631.861 111.557 4 646.363 115.387 5 660.710 6 674.881 124.684 7 688.855 130.136
8 699.531 134.775 8 699.531 134.775
:le Center At X = 538.676 ; Y = Circle Center At $X = 538.676$; $Y = 493.298$; and Radius = 392.954 Factor of Safety *** 1.386 *** Failure Surface Specified By 7 Coordinate Points
Point X-Surf Y-Surf $X-Surf$ $Y-Sur$
(ft) (ft) No. (ft) (ft) No. (ft) (ft)
1 604.304 106.109
2 619.051 108.851 2 619.051 108.851 3 633.665 112.232 4 648.118 116.246
5 662.383 120.885 5 662.383 120.885 6 676.432 126.140 7 690.044 131.919
le Center At X = 548.681 ; Y = Circle Center At $X = 548.681$; $Y = 446.255$; and Radius = 344.665 Factor of Safety *** 1.387 *** Failure Surface Specified By 8 Coordinate Points Point X-Surf Y-Surf
No. (ft) (ft) No. (ft)
1 600.350 1 600.350 104.918

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2 615.118 107.548 3 629.769 110.766 644 . 279 114 . 568 5 658.625 118.947
6 672.785 123.897 6 672 . 785 123 .897 7 686.736 129.409 8 697.462 134.152
Circle Center At $X = 542.569$; $Y = 472.776$; and Radius = 372.368
Factor of Safety 8 697.462 134.152 Factor of Safety
*** 1.387 *** **** END OF GSTABL7 OUTPUT **** the first in teacher line of fifthering

PROFIL h:\Geo-Environmental (0120)\2003\1030105-01 Peterson-Puritan OU2\CPT\puritan #2.in V
Petterson/Puritan OU2 Petterson/Puritan OU2 12 8 10, 62. 451.5 62. 3 451.5 62. 452.5 66. 3 452.5 66. 455.5 66. 3 455.5 66. 456.5 64. 1 456.5 64. 491. 72. 2 491. 72. 703.6 136 . 2 703.6 136. 760.6 140. 2 760.6 140. 798.6 140. 2 456.6 64. 466. 54. 1 466. 54. 798.6 54. 1 10. 52. 440. 52. 1 440. 52. 451.5 62. 1 0. 0. 0. SOIL SandSiltTrash Organic 3 85.96 111.43 0. 37 ..0. o. 1 55. 65. 0. 22.5 0. 0. 1 43. 89. 0. 15. 0. 0. 1 WATER 1 62 . 46 4 0.5 10. 61. 456.5 63. 703.6 85. 798.6 85. CIRCL2 2000 1

600. 610. 690. 710. 10. 15. 0. o.

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