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Via Electronic Mail

February 18, 2026

Mr. Alexander Carli-Dorsey
EPA Project Manager
U.S. Environmental Protection Agency
New England Region
Five Post Office Square, Suite 100
Boston, MA 02109

Re: GE-Pittsfield/Housatonic River Site
Rest of River (GECD850)
Phase 1 Inspection/Evaluation Report for Rising Pond Dam

Dear Mr. Carli-Dorsey:

On November 20, 2025, GE's consultants from GZA GeoEnvironmental, Inc. performed a biennial Phase 1 inspection/evaluation of Rising Pond Dam. Enclosed is GE's report on that inspection/evaluation, prepared by GZA. That inspection was conducted and this report prepared in accordance with GE's August 14, 2019 Operation, Monitoring, and Maintenance (OM&M) Plan for Woods Pond Dam, as amended in September 2020, because GE's more recent revised OM&M Plan for this dam – which was initially submitted in December 2024, conditionally approved on October 21, 2025, and required to be revised again – was not revised and re-submitted until December 22, 2025. That revised OM&M Plan was approved by EPA in a letter dated February 12, 2026. All future inspections will be conducted in accordance with that December 2025 revised OM&M Plan, starting with the first quarterly inspection in 2026.

Please let me know if you have any questions about the enclosed report.

Sincerely,

Rachel B. Leary. P.E.
Project Manager

Enclosure

Cc: (via electronic mail)
Joshua Fontaine, EPA
John Kilborn, EPA

Christopher Ferry, ASRC Federal
Thomas Czelusniak, HDR Inc.
Scott Campbell, Taconic Ridge Environmental
Izabella Zapisek, Taconic Ridge Environmental
Emily Caruso, MassDCR, Office of Dam Safety
Michael Gorski, MassDEP
Tamara Cardona-Marek, MassDEP
Ben Guidi, MassDEP
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Lance Hauer, GE
Kevin Mooney, GE
Eric Merrifield, GE
Jonathan Andrews and Seth Krause, GZA
James Bieke, Counsel for GE
Public Information Repository at David M. Hunt Library in Falls Village, CT
GE Internal Repository

RISING POND DAM

PHASE 1

INSPECTION / EVALUATION REPORT

Dam Name: Rising Pond Dam

NID ID#: MA00250

Owner: General Electric Company

Town: Great Barrington

Consultant: GZA GeoEnvironmental, Inc.

Date of Inspection: November 20, 2025

Date of Report: Submitted February 18, 2026

EXECUTIVE SUMMARY

This report summarizes the results of the Phase 1 visual dam inspection conducted by GZA GeoEnvironmental, Inc. (GZA) on behalf of the General Electric Company (GE) on November 20, 2025 at Rising Pond Dam in Great Barrington, Massachusetts (the Dam).

This inspection was conducted in accordance with GE's revised Operation, Monitoring, and Maintenance Plan (OM&M Plan) for Rising Pond Dam, dated August 14, 2019, as approved by the United States Environmental Protection Agency (EPA) on August 27, 2019 and amended on September 14, 2020. Although a further revised OM&M Plan for Rising Pond Dam was submitted to EPA on December 19, 2024, that revised plan was not conditionally approved by EPA until October 21, 2025 and, as required by EPA, was revised again on December 22, 2025, with that further revised plan approved by EPA on February 12, 2026. Since the November 2025 Phase 1 dam inspection was conducted prior to submission and approval of that further revised plan, it was conducted, and this Phase 1 report was prepared, in accordance with the previously approved OM&M Plan.

As required by the 2019 OM&M Plan, this Phase 1 Inspection was performed with the spillway dewatered (pond lowered below spillway crest). In the meantime, in July 2025, an underwater dive inspection had been performed at the Rising Pond Dam. That inspection is described in a memorandum attached as **Appendix I** to this Phase 1 report.

In general, the overall condition of the Rising Pond Dam during the November 20, 2025 Phase 1 visual inspection was judged to be **SATISFACTORY**. This overall condition rating remains unchanged from the previous Phase 1 inspection conducted in November 2023, as reported in GE's Phase 1 Inspection/Evaluation Report submitted on February 9, 2024. Based on the results of the latest inspection, the Dam is in compliance with Massachusetts Department of Conservation and Recreation (MassDCR) Office of Dam Safety (ODS) regulations.

During the November 20, 2025 visual inspection, the Dam was found to have the following unusual conditions of note (several of which had also been noted in the prior Phase 1 inspection and/or in intervening quarterly inspections):

1. Some rust was present on the forebay trash rack. The rust appears similar to that observed during prior inspections and does not appear to be affecting the trash rack integrity based on the July 2015 dive inspection.
2. Vegetation growth was observed in the right stone masonry wall of the forebay.
3. A horizontal joint/crack was observed in the right forebay wall during the July 2025 dive inspection, as summarized in the memorandum provided in **Appendix I** to this report.
4. A crack in the forebay concrete was visible on the upstream face below the abandoned operator. This crack has been observed in prior inspections and appears to be a stable condition.
5. Minor cracking and efflorescence were observed on the downstream face of the gate platform (low-level outlet). This condition has been observed in prior inspections and appears to be a stable condition.
6. A crack was observed in the concrete step to the left of the gate platform. The crack does not affect use of the step.

7. A diagonal crack approximately 0.4 inch wide was observed in the brick wall of the pumphouse near the downstream right corner. This is a new observation made during the 2025 Phase 1 inspection.
8. A low area in the penstock, indicating a possible settled area or "belly," was observed, as it had been during previous inspections and has been investigated during the past three years. Repairs are recommended as part of the ongoing penstock investigations.
9. A surficial depression was observed at the former railroad trestle about 130 feet downstream of the gate platform and about 4½ feet left of the penstock springline (sidewall). This triangular, vertical-sided depression measured up to two feet deep and one to two feet in plan dimensions.
10. Floating timber debris was observed upstream of the safety buoys.
11. Timber debris was observed in the energy dissipators at the downstream (right side) toe of the spillway.
12. Minor damage to one of the energy dissipators (fifth from the right) was observed; this damage is not anticipated to impact the overall effectiveness of the energy dissipators.
13. Minor leakage was observed from a four-inch long crack or joint in the right side of the downstream face of the spillway near the sloping ogee concrete and the apron concrete. This is a new observation made during the 2025 dewatered Phase 1 inspection.
14. Areas of minor diffuse leakage were observed from the downstream left masonry training wall.
15. There was a missing gate in the right spillway training wall fence. This opening has been temporarily sealed using plywood.
16. A low area of riprap stones was observed at the downstream toe of slope on right embankment, as it has been in prior inspections, and appears to be a stable condition.
17. Minor sloughing of the riprap downstream of the right training wall. This condition has been observed in prior inspections and appears to be a stable condition.
18. Iron staining was observed along the shoreline at the toe of the right embankment, but no active seepage was observed.
19. Standing water was observed beyond the toe of the right training embankment. This condition has been periodically observed during inspections and does not appear to be the result of seepage through the embankment.
20. The protective casing for monitoring well/piezometer GZ-6 was observed to be slightly tilted, as it has been during prior inspections; this condition appears to be stable.
21. Minor buildup of sediment/silt was measured in monitoring wells GZ-2, GZ-5, and GZ-7. However, the current sediment/silt levels is not impacting the water level readings at these monitoring wells.

GZA recommends that specific activities be conducted to address the above-described conditions observed during the November 20, 2025 Phase 1 inspection, in addition to complying with the regular maintenance and repair requirements specified in Sections 4.1 and 4.2 of the OM&M Plan. Those recommended activities and their current status are as follows:

Studies and Analyses

There are no studies or analyses recommended at this time. The current status of the studies and analyses that were recommended in the 2024 Phase 1 Evaluation/Inspection Report on the 2023 Phase 1 inspection are provided below:

Prior Recommendation	Current Status/Schedule
1. Investigate the low area in the penstock.	Ongoing. The penstock investigations were documented in 2023, 2024, and 2025 Penstock Investigations End-of-Year Reports, submitted to EPA on May 24, 2024, December 19, 2024, and January 9, 2026, respectively. Repairs and ongoing monitoring were recommended in the 2025 End-of-Year Report. See Section 3.3 of this report for more detail.
2. Perform an updated dam break analysis in accordance with current Massachusetts Department of Conservation and Recreation (DCR) Office of Dam Safety (ODS) guidelines.	Completed. An updated dam break analysis was performed and included in the 2024 Emergency Action Plan (EAP) update for the Dam, as described in Section 2.6 of this report.
3. Establish Design Basis Values (DBVs) for the observation wells (OWs) and vibrating wire piezometers (VWPs)s by determining the maximum allowable reading that will meet current MassDCR ODS stability criteria. Threshold and action levels should also be established as early warning signs of a potential dam safety concerns. DBVs and threshold /action levels should be established for critical instrumentation (as determined during the evaluation).	Completed. The existing analysis of record on file with MassDCR ODS (ca 2012) was reviewed and Response Values for OWs and VWPs critical to dam safety have been established, as discussed in Section 2.7.4 of this report.

It is recommended that the Response Values for OWs and VWPs for the instruments critical to dam safety (as identified in Section 2.7.4 of this report) be incorporated into the approved 2025 OM&M Plan and replace the historic action level shown in the current OM&M Plan.

Monitoring and Maintenance

GZA recommends that the Dam be maintained and monitored in accordance with the applicable OM&M Plan. In addition, the following items should be monitored and maintained:

Recommendation	Current Status/Schedule
1. Continue to monitor the rust on the trash rack.	This condition is monitored during quarterly and biennial inspections, as well as during dive inspections, which are completed every five years. The most recent dive inspection was conducted in July 2025 (see **Appendix I**) and the next one is scheduled for 2030.
2. Remove the vegetation growth observed in the right stone masonry wall of the forebay.	Vegetation will be cleared as part of ongoing vegetation maintenance in 2026.
3. Continue to monitor the cracking and efflorescence below the abandoned operator in the downstream forebay concrete wall (gate platform upstream face), and the downstream face of the gate platform.	This condition is monitored during quarterly and biennial inspections.
4. Continue to monitor the crack at gate platform concrete step for potential future repair if needed.	This condition is monitored during quarterly and biennial inspections.
5. Monitor the diagonal crack in the downstream pumphouse brick wall near the right corner.	This condition will be monitored during future quarterly and biennial inspections.
6. Monitor the surficial depression adjacent to the former railroad trestle.	This condition will be monitored during future quarterly and biennial inspections
7. Remove the floating timber debris upstream of the safety buoys.	The debris will be removed during planned maintenance activities in 2026.
8. Continue to monitor the debris on tailrace, forebay trash racks, area around forebay, spillway, upstream buoys, and energy dissipators for potential removal where needed.	These conditions are monitored during quarterly and biennial inspections and debris is removed when necessary.
9. Continue to monitor the minor damage to the energy dissipator (fifth from the right) and the overall effectiveness of the energy dissipators. Conduct repairs if needed.	The energy dissipators are monitored during quarterly and biennial inspections to assess their effectiveness.
10. Monitor the minor leak emanating from a four-inch long crack or joint located below the second from right weephole at the interface between the sloping ogee concrete and the apron concrete, and the minor diffuse leakage at the downstream left masonry training wall. (This leak is typically obscured by spillway flow.)	This leakage will be monitored during the planned 2029 Phase 1 inspection under dewatered conditions and, if visible, during low-flow or dewatered conditions that occur during other future inspections.

Recommendation	Current Status/Schedule
11. Continue to monitor the location of the leakage through the left spillway training wall.	This area is monitored for leakage during quarterly and biennial inspections.
12. Continue to visually monitor the low area of riprap stones at the downstream toe of slope on the right embankment.	This condition is monitored during quarterly and biennial inspections.
13. Monitor the minor sloughing of the riprap downstream of the right training wall.	This condition is monitored during quarterly and biennial inspections.
14. Continue to monitor for seepage (only iron staining observed), if active, at the downstream toe of the right embankment.	This area is monitored for potential seepage during quarterly and biennial inspections.
15. Continue to monitor the slight tilt in well GZ-6.	This condition is monitored during quarterly and biennial inspections.
16. Continue to monitor sediment buildup in observations wells.	Minor sediment buildup was measured in OWs GZ-2, GZ-5, and GZ-7. This condition will continue to be monitored, and if sediment becomes excessive and begins to impact readings, the wells will be flushed.

Minor Repairs

GZA recommends that the following minor repairs be performed at the Dam:

Recommendation	Schedule
1. Fill the crack in the forebay platform step.	The joint will be filled in 2026.
2. Repoint the diagonal crack in the brick pumphouse facade.	Repointing will be performed in 2026.
3. Fill the surficial depression adjacent to the former railroad trestle with friable material such as washed peastone.	The surficial depression will be filled during planned maintenance activities in 2026.
4. Replace the missing gate along the top of the right spillway training wall. In the interim, continue securing the gate opening to prevent unauthorized access.	The gate will be replaced during planned maintenance activities in 2026.
5. Fill horizontal joint in right forebay wall (below normal pool and likely source of right spillway training wall leakage).	Joint filling will be performed during next scheduled dive inspection in 2030.

Remedial Modifications

GZA recommends that the following remedial modification be performed at the Dam:

Recommendation	Current Status/Schedule
1. Construct structural lining within the penstock as described in the 2025 Penstock Investigations End-of-Year Report.	As recommended in the 2025 Penstock Investigations End-of-Year Report, initiate the design for a structural lining system in 2026, with final design and construction of the structural lining system targeted for 2027.

Dam Evaluation Summary Detail Sheet

1. NID ID:	**MA00250**	**4. Inspection Date:**	**November 20, 2025**
2. Dam Name:	**Rising Pond Dam**	**5. Last Insp. Date:**	**November 15, 2023**
3. Dam Location:	**Great Barrington, MA**	**6. Next Inspection:**	**November 20, 2027**
7. Inspector:	**Jonathan D. Andrews, P.E.**		
8. Consultant:	**GZA GeoEnvironmental, Inc.**		
9. Hazard Code:	**Significant**	**9a. Is Hazard Code Change Requested?:**	**No**
10. Insp. Frequency:	**5 Years**	**11. Overall Physical Condition of Dam:**	**SATISFACTORY**
12. Spillway Capacity (% SDF)	**>100% SDF w/ no actions by Caretaker**		

E1. Design Methodology:	**4**	**E7. Low-Level Discharge Capacity:**	**5**
E2. Level of Maintenance:	**5**	**E8. Low-Level Outlet Physical Condition:**	**4**
E3. Emergency Action Plan:	**5**	**E9. Spillway Design Flood Capacity:**	**5**
E4. Embankment Seepage:	**4**	**E10. Overall Physical Condition of the Dam:**	**4**
E5. Embankment Condition:	**4**	**E11. Estimated Repair Cost:**	**Not applicable**
E6. Concrete Condition:	**4**		

Evaluation Description

E1: DESIGN METHODOLOGY
1. Unknown Design – no design records available
2. No design or post-design analyses
3. No analyses, but dam features appear suitable
4. Design or post design analysis show dam meets most criteria
5. State of the art design – design records available & dam meets all criteria

E2: LEVEL OF MAINTENANCE
1. Dam in disrepair, no evidence of maintenance, no O&M manual
2. Dam in poor level of upkeep, very little maintenance, no O&M manual
3. Dam in fair level of upkeep, some maintenance and standard procedures
4. Adequate level of maintenance and standard procedures
5. Dam well maintained, detailed maintenance plan that is executed

E3: EMERGENCY ACTION PLAN
1. No plan or idea of what to do in the event of an emergency
2. Some idea but no written plan
3. No formal plan but well thought out
4. Available written plan that needs updating
5. Detailed, updated written plan available and filed with MADCR, annual training

E4: SEEPAGE (Embankments, Foundations, & Abutments)
1. Severe piping and/or seepage with no monitoring
2. Evidence of monitored piping and seepage
3. No piping but uncontrolled seepage
4. Minor seepage or high volumes of seepage with filtered collection
5. No seepage or minor seepage with filtered collection

E5: EMBANKMENT CONDITION (See Note 1)
1. Severe erosion and/or large trees
2. Significant erosion or significant woody vegetation
3. Brush and exposed embankment soils, or moderate erosion
4. Unmaintained grass, rodent activity and maintainable erosion
5. Well maintained healthy uniform grass cover

E6: CONCRETE CONDITION (See Note 2)
1. Major cracks, misalignment, discontinuities causing leaks, seepage or stability concerns
2. Cracks with misalignment inclusive of transverse cracks with no misalignment but with potential for significant structural degradation
3. Significant longitudinal cracking and minor transverse cracking
4. Spalling and minor surface cracking
5. No apparent deficiencies

E7: LOW-LEVEL OUTLET DISCHARGE CAPACITY
1. No low level outlet, no provisions (e.g. pumps, siphons) for emptying pond
2. No operable outlet, plans for emptying pond, but no equipment
3. Outlet with insufficient drawdown capacity, pumping equipment available
4. Operable gate with sufficient drawdown capacity
5. Operable gate with capacity greater than necessary

E8: LOW-LEVEL OUTLET PHYSICAL CONDITION
1. Outlet inoperative needs replacement, non-existent or inaccessible
2. Outlet inoperative needs repair
3. Outlet operable but needs repair
4. Outlet operable but needs maintenance
5. Outlet and operator operable and well maintained

E9: SPILLWAY DESIGN FLOOD CAPACITY
1. 0 - 50% of the SDF or unknown
2. 50-90% of the SDF
3. 90 - 100% of the SDF
4. >100% of the SDF with actions required by caretaker (e.g. open outlet)
5. >100% of the SDF with no actions required by caretaker

E10: OVERALL PHYSICAL CONDITION OF DAM
1. UNSAFE – Major structural, operational, and maintenance deficiencies exist under normal operating conditions
2. POOR - Significant structural, operation and maintenance deficiencies are clearly recognized under normal loading conditions
3. FAIR - Significant operational and maintenance deficiencies, no structural deficiencies. Potential deficiencies exist under unusual loading conditions that may realistically occur. Can be used when uncertainties exist as to critical parameters
4. SATISFACTORY - Minor operational and maintenance deficiencies. Infrequent hydrologic events would probably result In deficiencies.
5. GOOD - No existing or potential deficiencies recognized. Safe performance is expected under all loading including SDF

E11: ESTIMATED REPAIR COST

Estimation of the total cost to address all identified structural, operational, maintenance deficiencies. Cost shall be developed utilizing standard estimating guides and procedures

Changes/Deviations to Database Information since Last Inspection

Owner conducts biennial inspections (every 2-years) in accordance with the current Operations, Monitoring, and Maintenance Plan.

PREFACE

The assessment of the general condition of the Rising Pond Dam reported herein was based upon available data and visual inspections. Detailed investigations and analyses involving topographic mapping, subsurface investigations, testing and detailed computational evaluations were beyond the scope of this report unless reported otherwise.

In reviewing this report, it should be realized that the reported condition of the Dam was based on observations of field conditions at the time of inspection, along with data available to the inspection team.

It is critical to note that the condition of the Dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the reported condition of the Dam will continue to represent the condition of the Dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

_____

Jonathan D. Andrews, P.E.

Massachusetts License No.: 46462
Principal-In-Charge
GZA GeoEnvironmental, Inc.

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FIGURES

APPENDICES

1.0 DESCRIPTION OF PROJECT

1.1 GENERAL

1.1.1 Authority

The General Electric Company (GE) retained GZA GeoEnvironmental, Inc. (GZA) to perform a visual inspection/evaluation and develop a report of conditions for the Rising Pond Dam (the Dam) along the Housatonic River in Great Barrington, Berkshire County, Massachusetts, as required by GE's revised Operation, Monitoring, and Maintenance Plan (OM&M Plan) for Rising Pond Dam, dated August 14, 2019, as approved by the United States Environmental Protection Agency (EPA) on August 27, 2019, and amended on September 14, 2020 (approved by EPA on October 6, 2020). That inspection was conducted on November 20, 2025.

It should be noted that a further revised OM&M Plan for Rising Pond Dam was submitted to EPA on December 19, 2024. That further revised OM&M Plan was conditionally approved by EPA in a letter dated October 21, 2025, which required GE to revise that plan again by December 22, 2025. GE submitted a further revision of that plan addressing EPA's conditions on December 22, 2025, and that plan was approved by EPA in a letter dated February 12, 2026. In the meantime, since this Phase 1 visual dam inspection was conducted prior to submission and approval of that further revised OM&M Plan, it was conducted in accordance with the 2019 OM&M Plan as amended, and this Phase 1 report was prepared in accordance with that plan, except that the descriptive information in Sections 1.2 and 1.3 has been updated from the 2025 OM&M Plan.

This inspection was performed and this report was prepared in compliance with Section 3.2 of the 2019 OM&M Plan (which requires biennial Phase 1 engineering inspections of this Dam) and with 302 CMR 10.07 of the Massachusetts dam safety regulations. This report is subject to the Limitations in **Appendix A**.

1.1.2 Purpose of Work

The purpose of the November 2025 Phase 1 engineering investigation/evaluation was to inspect and evaluate the present condition of the Dam and appurtenant structures in order to provide information that will assist in both prioritizing dam repair needs and planning/conducting maintenance and operations.

The investigation was divided into three parts: (1) obtain and review available prior reports, investigations, and data pertaining to the Dam and appurtenant structures; (2) perform a visual inspection of the Dam; and (3) prepare and submit a report presenting the evaluation of the structure, including recommendations for maintenance, repair, and remedial actions (if warranted).

1.1.3 Definitions

To provide the reader with a better understanding of the report, definitions of commonly used terms associated with dams are provided in **Appendix E**. Many of these terms may be included in this report. The terms are presented under common categories associated with dams, which include: (1) orientation; (2) dam components; (3) size classification; (4) hazard classification; and (5) miscellaneous.

Elevations used in this report are referenced to the National Geodetic Vertical Datum of 1929 (NGVD29).

1.2 DESCRIPTION OF PROJECT

1.2.1 Location

Town: Great Barrington

County: Berkshire

The left (north) abutment of Rising Pond Dam is on Route 183 (at 285 Park Street, the Hazen Paper Mill) near the intersection of Park Street and Mountain Street. To access the right (south) abutment from Route 183, one turns west onto Division Street, continues for 0.9 mile and proceeds north on Van Deusenville Road for about 1.1 miles to a railroad access gate on the right. On the east, a 0.3-mile path, capable of passing vehicular traffic, leads to the right side of the Dam. The location of the Dam is shown on the United States Geological Survey (USGS) Great Barrington, MA topographic map (see **Figure 1**). The approximate coordinates are:

Latitude: 42.2424 N Longitude: 73.3577 W

1.2.2 Owner/Caretaker, Dam Safety Engineer, and Contractor

GE is the owner of the Rising Pond Dam and is responsible for overseeing the operations and maintenance of the Dam. The current Caretaker on GE's behalf is:

Kevin Mooney
GE Aerospace
1 Plastics Avenue
Pittsfield, MA 01201
Daytime Phone: (413) 448-6610
Cell Phone: (413) 441-4619

The current Alternate Caretaker on GE's behalf is:

Rachel Leary
GE Aerospace
1 Plastics Avenue
Pittsfield, MA 01201
Phone: (413) 741-0933

GE has retained a professional engineer experienced in dam safety and engineering to perform routine inspections of the Dam, including inspections following flooding or storm events as needed, conduct biennial Phase 1 inspections/evaluations, and review other dam issues on an as-needed basis. The current consulting dam safety engineer is:

GZA GeoEnvironmental, Inc. (Jonathan D. Andrews, P.E., or other qualified dam safety engineer)
249 Vanderbilt Avenue
Norwood, Massachusetts 02062
Office: (781) 278-5808
Cell: (781) 983-2881

GE has also retained a licensed contractor experienced with operation and maintenance of dams to perform routine maintenance and operation activities at the Dam, as well as to respond to dam safety issues on an as-needed basis. The current contractor is:

LB Corporation (Steve Garrity)
95 Marble Street
Lee, Massachusetts 01238
Office: (413) 243-1072
Cell: (413) 441-1412

See **Table 1.1** (in Section 1.4 below) for additional information.

1.2.3 Purpose of the Dam

The original construction date, details, and purpose of the Rising Pond Dam are not known, but it is likely that Rising Pond Dam was constructed in the 1800s. The impoundment formerly provided water to power machinery in the adjacent mill complex. Sometime after 1934, the dam was used for power generation, which likely ceased in 1953, but certainly prior to 1979. The right embankment of the Dam was previously used as a railroad embankment/bridge abutment. The purpose of the current Dam is to impound Rising Pond, including impounding existing sediments that are impacted by polychlorinated biphenyls.

1.2.4 Description of the Dam and Appurtenances

The Rising Pond Dam is a run-of-the-river structure and currently consists of left and right earth embankments, with a spillway and outlet works. The outlet works consists of a low-level outlet controlled by a gate, an underground penstock pipe, a surge chamber, and a diversion channel.

An aerial photograph of the Dam and its appurtenances is shown below. A site plan is included as **Figure 2**.

Source: GZA ArcGIS Mapper Tool

The right earthen embankment is approximately 38 feet high, with upstream and downstream slopes of approximately two horizontal to one vertical (2H:1V). Slope protection at the waterline consists of a combination of steel sheetpiles and riprap. A shed for instrumentation leads has been installed at the crest near the right spillway training wall.

The spillway consists of a concrete facing with steel crest plate. The spillway is approximately 127 feet wide and 30 feet high, with a crest elevation of approximately 716.7 feet on the left side to 716.1 feet on the right side, with an average elevation of about 716.4 feet. The approximate 0.6-foot difference in elevation across the spillway crest is likely an as-built condition; there are no records of past settlement and no indications of active settlement of the spillway. Spillway training walls are a combination of concrete, grouted stone masonry, and steel sheetpile.

The low-level outlet is located directly to the left of the spillway. The low-level outlet works consist of a grouted stone masonry forebay with a steel trash rack, a concrete-walled gate chamber with sluice gate and a 14-foot diameter steel penstock that extends approximately 220 feet downstream to a surge chamber next to the mill. The surge chamber is drained by an open diversion channel reinforced concrete tailrace that discharges to the Housatonic River approximately 250 feet downstream. The penstock invert elevation is 699 feet. In the past, gate chamber drainage was provided by a 12-inch-diameter well drain that discharged through the left downstream training wall. Well drain flow was controlled by a valve located in a covered pit between the gate chamber and

left training wall. The well drain is no longer needed because the diversion channel now provides gravity drainage for the penstock and the valve has not been operated in years. A fire protection pumphouse which services the mill building is located on the left embankment crest to the left of the forebay.

A wide embankment/fill area is present on the left side of the spillway and outlet structures. The upstream slopes are steep and have some riprap protection near the low-level outlet. The downstream slope is poorly defined and consists of the mill.

The shores of Rising Pond are generally wooded. Route 183 extends parallel to the east bank of the impoundment/river. An abandoned railroad bridge abutment and center pier are located immediately upstream of the dam. The western railroad bridge pier was formerly integral with the right embankment. The immediate downstream area includes the mill and wooded riverbanks. A USGS gaging station is located on the Division Street Bridge approximately one mile downstream of the Dam.

Active instrumentation at the Rising Pond Dam includes three staff gages installed at the spillway, and five observation wells (OWs) and 23 vibrating wire piezometers (VWPs) installed in the left embankment, spillway, and right embankment. GE has installed automated pond water sampling equipment at the right and left training walls. The sampling equipment serves no dam-related function.

The most recent topographic/bathymetric survey and dive inspection of the Dam was conducted during the week of July 7, 2025 (with supplemental topographic surveys conducted in October and November 2025), as discussed in Sections 1.3.7 and 2.1.3 below.

1.2.5 Operations and Maintenance

As stated in Section 1.2.2, GE is the owner of the Rising Pond Dam and is responsible for overseeing the operations and maintenance of the Dam. See Section 1.2.2 and **Table 1.1** (in Section 1.4 below) for additional information.

1.2.6 DCR Size Classification

Rising Pond Dam has a height of approximately 40 feet and a maximum storage capacity of 710 acre-feet. Refer to **Appendix E** for definitions of height of dam and storage. In accordance with the classification procedures of the MassDCR Office of Dam Safety (ODS), under the Massachusetts dam safety regulations in 302 CMR 10.00, Rising Pond Dam is an **Intermediate** size structure based on maximum storage between 50 and 1,000 acre-feet.

1.2.7 DCR Hazard Potential Classification

In accordance with MassDCR ODS classification procedures, under the Massachusetts dam safety regulations, Rising Pond Dam is classified as a dam with **Significant Hazard** potential. This hazard class assessment is consistent with the hazard class for the Dam on record with the MassDCR ODS.

1.3 PERTINENT ENGINEERING DATA

1.3.1 Drainage Area

The drainage area for Rising Pond Dam is approximately 279 square miles and encompasses a large portion of western Massachusetts along the New York border. The drainage area includes large areas of agricultural and residential development, wooded mountainous terrain, and several small urban areas. The drainage area is delineated on **Figure 3,** and the dam and downstream area are shown on **Figure 4**.

1.3.2 Reservoir

Reservoir storage volume data presented below are based on previous analyses, as well as data presented in a 1979 U.S. Army Corps of Engineers Phase I Inspection Report. They are:

Condition	Elevation (feet)	Storage Volume (acre-feet)
Normal Pool	716.4	195
Maximum Pool	726.2	710
Spillway Design Flood (SDF) Pool	724.4	±600

See **Table 1.1** (Section 1.4 below) for additional information.

1.3.3 Discharges at the Dam Site

Rising Pond Dam's run-of-the-river spillway constantly discharges water unless the low-level sluice gate is opened to reduce the level of the impoundment.

1.3.4 General Elevations (feet, NGVD29)[1]

A. Top of Dam Embankment: 728.0 (right side); 727.0 (left side)
B. Spillway Design Flood Pool: 724.4 (100-year flood)
C. Normal Pool: 716.4±
D. Spillway Crest: 716.6 (left side), 716.3 (middle), 716.0 (right side)
E. Low Level Outlet Invert: 699.0 ± (Foresight, 1990)
F. Streambed at Toe of the Dam: 692±
G. Upstream Water at Time of Inspection: 715.3± (below the spillway crest)
H. Downstream Water at Time of Inspection: 693.8±

[1] These elevations have been updated based on the 2025 topographic and bathymetric survey.

1.3.5 Main Spillway Data

A.	Type:	Concrete and steel -faced rock filled timber crib, ogee-shaped
B.	Weir Length:	127.0 feet
C.	Weir Crest Elevation:	716.6 (left), 716.3 (middle), 716.0 (right) feet NGVD29
D.	Upstream Channel:	Housatonic River/Rising Pond
E.	Downstream Channel:	Housatonic River
F.	Channel Bottom Elevation:	692.0± feet, NGVD29

1.3.6 Intake/Outlet Works

A.	Intake Type:	Slide gate-controlled penstock
B.	Outlet Size:	14-foot-diameter
C.	Intake Invert El:	699.0 ± NGVD29 (Foresight, 1990)

1.3.7 Key Elevations to be Monitored

The most recent topographic/bathymetric survey of the Dam was conducted by Hill Engineers, Architects, Planners during the week of July 7, 2025, with supplemental surveys conducted in October and November 2025. The 2025 surveys were performed to allow for a comparison of key elevations to the previous topographic and bathymetric surveys, conducted by Foresight Land Services in February and August 2020 and shown on a plan dated October 6, 2020. Overall, the 2020 and 2025 topographic and bathymetric surveys were similar, with no notable differences in key elevations or apparent sediment accumulation noted.

The key elevations monitored are shown on the most recent topographic and bathymetric survey plan, dated July 2025, with supplemental surveys conducted in October and November 2025. The survey plan and a separate table presenting the tabulated bathymetric survey data points with reference locations (GPS coordinates) are provided in **Appendix F-1.** The key elevations surveyed during the 2025 topographic surveys are presented in the following table.

Point	Location	2025 Surveyed Elevation, feet NGVD 29 (see Appendix F-1)
A	Right side top of spillway training wall at top of dam	728.4
B	Right side spillway training wall at angle	703.8
C	Right side top of spillway training wall at bottom of slope	702.2
D	GZ-2-OW top of casing	732.2
E	GZ-5-OW/PZ top of casing	727.7
F	GZ-7-OW top of casing	715.7
G	Right end of right side sheetpile wall	726.2
H	Upstream, right corner of left side forebay sheetpile wall	723.2
I	Center gate mount	729.2
J	Centerline downstream end of concrete spillway	695.5
K	Left side corner of downstream end of concrete spillway	695.5
L	Right end downstream end of concrete spillway	695.5

Point	Location	2025 Surveyed Elevation, feet NGVD 29 (see Appendix F-1)
M	Centerline spillway crest	716.3
N	Right end spillway crest (at wall)	716.0
O	Left end spillway crest (at wall)	716.6
P	Rebar monitoring point at low area	700.6
Q	Monitoring Well D-9 top of casing at the ground	726.9

Note that the casing for well GZ-5 was previously repaired after the second quarterly inspection for 2022, and the top of the casing for the D-9 piezometers was not surveyed after installation. The top of casing elevations for GZ-5 and D-9 were surveyed during the 2025 topographic surveys. These surveyed top of casing elevations are presented in the table below.

1.3.8 Design and Construction Records and History

The Dam was originally constructed in the 1800s. The embankment was constructed of alluvial sand and gravel excavated from the west riverbank. Original upstream slopes ranged from 1.5H:1V to 4H:1V, and downstream slopes ranged from 1H:1V to 1.5H:1V. The original embankment height was about 17 feet. The spillway and railroad bridge abutment were reportedly constructed on rock-filled timber cribbing over grouted cobbles in timber cribbing laid on the original river bottom. The original spillway was about 17 feet high and was faced with wooden planks laid at a 1H:1V slope.

In 1934, the embankments and spillway were reportedly raised by about 10.5 feet and spillway flashboards were added. Rock-filled timber cribbing was placed above the original structure and new wooden facing was placed on the spillway.

In 1953, the Dam was reportedly raised to elevation 716.5 feet. The stone masonry outlet channel training walls were replaced with a headgate and 14-foot diameter steel penstock. The downstream timber plank spillway facing was demolished and replaced with a concrete slab. The spillway crest was rebuilt with a concrete slab faced with a steel plate. The upstream timber plank facing was covered with sand and gravel fill covered by a new concrete apron slab and an upstream concrete wall was added.

Between 1991 and 1993, the Dam was significantly rehabilitated. The rehabilitation generally included: installation of an upstream steel sheetpile cutoff wall; removal of the upstream timber plank spillway facing; filling of voids in the timber cribbing with peastone and sandy gravel; flowable fill placement in voids below the crest and upstream slab; installation of tiedowns and passive H-pile shear keys in the downstream apron; replacement of deteriorated areas of spillway training wall concrete; repointing of the forebay walls and floor; replacement of forebay trash racks; construction of a concrete tailrace outlet channel and plugging of the former penstock where it entered the mill; placement of riprap slope armor; and raising of the right embankment to elevation 727.0 feet. In addition, piezometers and observation wells were installed in and below the embankment and spillway. In 2002, the forebay walls and right downstream training wall were repaired, and riprap was placed and reworked at the upstream slope and downstream toe of the right embankment.

By the early 2000s, the 14-foot sluice gate stem had become inoperable due to a broken gate stem. A replacement slide gate was installed in 2005. In 2010, new steel plate covers, fencing, bollards, and ladders were installed for the gate platform and well drain platform.

In 2011 through 2013, the Dam underwent repairs and rehabilitations to address embankment depressions that had formed behind the right downstream training wall and to address undermining of the downstream spillway apron and downstream right training wall. During this rehabilitation, sheetpiles were installed along the upstream edge of the right embankment crest. The new sheetpiles overlapped the existing crest sheetpiles and extended the line of sheeting approximately 60 feet westward (right). The right embankment was regraded, including levelling the crest to a uniform elevation 728.0 feet. The right training wall at the top of the embankment was extended upward by two feet to accommodate the crest levelling.

As part of the 2011 through 2013 repairs, a row of sheetpiles was installed at the downstream end of the spillway apron and a new two-foot-thick reinforced concrete downstream spillway apron was constructed. The timber cribbing and rubble fill underneath the apron slab was filled with un-reinforced, high-slump concrete. Concrete energy dissipaters were constructed on the downstream end of the apron. Riprap was refreshed in the discharge channel adjacent to the spillway apron. The downstream portion of the right training wall was refaced and raised by up to two feet. Voids under the downstream portion of the right training wall were filled with diver-placed grout bags and a two-phase program of cementitious grouting was performed within the embankment adjacent to the training wall. Riprap was placed behind (right of) the right training wall, including placement of geomembrane within the riprap to help convey training wall splash-over downstream. Right training wall splashover occurs adjacent to the energy dissipators during high flow events. Lexan panels were placed in the fence (in lieu of chain-link fabric) to mitigate the splash-over and help reduce ice formation during winter months. Areas of the left side stone masonry training wall were repointed. Piezometers were rewired to the instrument shed and anchors for warning buoys were installed upstream of the spillway.

At EPA's direction, GE installed warning signs at Rising Pond Dam in November and December of 2020, with one installed at a temporary location and later moved to its permanent location in October 2021. The format, wording, and locations of those signs were approved by EPA.

In August 2021, the penstock slide gate underwent inspection, repairs, and rehabilitation to address gate leakage. This work included the installation of new gate seals, which was completed with the gate removed. During the rehabilitation, a gap was observed between the gate and sill where leakage had previously been observed. A stainless-steel shim was fabricated and welded onto the bottom of the gate to help match the gate bottom to the sill. The gate was reset, and the leakage rate was found to be reduced by an order of magnitude or two. After repair, personnel could approach the gate for close inspection, probing, and measurements. This was a marked improvement over the 2015 condition, which prevented the inspector from getting within four feet of the gate.

In the fall of 2021, the left embankment was raised (by about 1.2 feet next to the fire protection pumphouse and tying into natural ground about 25 feet to the east) to bring the top elevation of the embankment above the 500-year flood pool level.

In November 2021, deteriorated railings around the forebay were replaced, along with steel plates to provide forebay overtopping protection during a 500-year flood event. The concrete walking surface was also replaced in November 2021. In November 2021, the impoundment was lowered, and low-flow conditions were present at the dam, allowing for the removal of debris stuck on the spillway and cleaning of the weepholes. In 2023, repairs were made to deteriorated brick masonry on the left forebay wall.

A low area in the penstock was observed during a visual inspection conducted by GZA in October 2021, indicating a possible settled area or "belly." The low area in the penstock is between 100 feet and 180 feet downstream of the Low-Level Outlet, with the lowest area at about 150 feet downstream of the Low-Level Outlet. As a result,

penstock investigations were conducted in 2023 and 2024 to determine the potential cause of the possible settled area and whether there is active movement or settlement of the penstock.

As of the end of 2025, a series of penstock investigations had been performed. The investigations included topographic surveys of the area above the penstock and the invert, crown and springlines of the penstock; penstock ovality measurements; a ground penetrating radar (GPR) survey of the area above the penstock; a dive inspection in the forebay upstream of the intake gate / penstock; test borings; test pits; ultrasonic thickness (UT) testing; and internal visual inspections.

The 2023 penstock investigations were documented in a report entitled *2023 Penstock Investigations End-of-Year Report*, submitted to EPA on May 24, 2024. The 2024 penstock inspections were documented in a report entitled *2024 Penstock Investigations End-of-Year Report*, submitted to EPA on December 19, 2024. The 2025 penstock inspections were documented in a report entitled *2025 Penstock Investigations End-of-Year Report,* submitted to EPA on January 9, 2026.

In July of 2025, the gate actuator was replaced by GE's contractor, LB Corporation, to mitigate a small grease leak in the previous actuator. The gate actuator manufacturer, Rotork, conducted a site visit on October 26, 2025 to commission the actuator and verify proper installation and functionality. During this site visit, the upper gate stop was set to 12 feet. It is noted that the new gate actuator readout for gate opening is in percent relative to gate stop setting (e.g., readout of 10 percent open equals 0.1 times 144 inches or 14 inches). The gate actuator manufacturer's installation statement summarizing its commissioning site visit on October 26, 2025 is provided in **Appendix F-2**.

A list of previous reports and references is included in **Appendix D**.

1.3.9 Operating Records

Quarterly visual inspections of the dam include readings of the vibrating wire piezometers and open standpipe observation wells. These records are maintained by the Caretaker and submitted to EPA in the quarterly inspection reports.

A summary of the collected instrumentation data and water level records is included as **Appendix G**.

1.4 SUMMARY DATA TABLE

See the following page for **Table 1.1, Summary Data Table**.

1.1 Summary Data Table

Required Phase 1 Report Data	Data Provided by the Inspecting Engineer
National ID #	MA00250
Dam Name	Rising Pond Dam
Dam Name (Alternate)	Rising Paper Company Dam, Rising Dam
River Name	Housatonic River
Impoundment Name	Rising Pond
Hazard Class	Significant
Size Class	Intermediate
Dam Type	Earthfill embankment with gravity spillway
Dam Purpose	Impound Rising Pond Reservoir / PCB sediments
Structural Height of Dam (feet)	38
Hydraulic Height of Dam (feet)	30
Drainage Area (sq. mi.)	279
Reservoir Surface Area (acres)	441
Normal Impoundment Volume (acre-feet)	195 at El. 716.4
Max Impoundment Volume ((top of dam) acre-feet)	710 at El. 726.2
SDF Impoundment Volume (acre-feet)	710
Spillway Type	Ogee overflow weir
Spillway Length (feet)	130
Freeboard at Normal Pool (feet)	10.3
Principal Spillway Capacity (cfs)	17,093 at El. 726.2 (500-year flood)
Auxiliary Spillway Capacity (cfs)	Not applicable
Low-Level Outlet Capacity (cfs)	±3,300 at El. 726.2
Spillway Design Flood* (flow rate - cfs)	100-year / 11,700
Winter Drawdown (feet below normal pool)	Not applicable
Drawdown Impoundment Vol. (acre-feet)	Not applicable
Latitude	42.2424 N
Longitude	73.3577 W
City/Town	Great Barrington
County Name	Berkshire
Public Road on Crest	No
Public Bridge over Spillway	No
EAP Date (if applicable)	12/22/2025
Owner Name	General Electric Company
Owner Address	1 Plastics Avenue
Owner Town	Pittsfield, MA 01201
Owner Phone	413-448-6610
Owner Emergency Phone	413-441-4619
Owner Type	Private
Caretaker Name	Kevin Mooney, Senior Project Manager
Caretaker Address	1 Plastics Avenue
Caretaker Town	Pittsfield, MA 01201
Caretaker Phone	413-448-6610
Caretaker Emergency Phone	413-441-4619
Date of Field Inspection	11/20/2025
Consultant Firm Name	GZA GeoEnvironmental, Inc.
Inspecting Engineer	Jonathan D. Andrews, P.E.
Engineer Phone Number	(781) 983-2881

2.0 INSPECTION

2.1 VISUAL INSPECTION

Rising Pond Dam was inspected on November 20, 2025 by Jonathan Andrews, P.E., Leslie DeCristofaro, E.I.T., and Mengxuan Zhao, E.I.T. of GZA. Tom Czelusniak (HDR representing EPA) and Steve Garrity of LB Corporation were also present during the inspection.

On September 14, 2020, in response to a prior directive from EPA, GE submitted an Amendment to the 2019 OM&M Plan stating that the biennial Phase 1 inspections of Rising Pond Dam would be conducted on a schedule that alternates between normal-flow and low-flow conditions, so that the spillway could be dewatered and observed under low-flow conditions every four years. The previous Phase 1 inspection, conducted on November 15, 2023, was conducted under normal-flow conditions, and this (latest) Phase 1 inspection, conducted November 20, 2025, was conducted under low-flow conditions with the impoundment water level lowered to allow visual observation of the spillway in the dry.

At the time of the November 20, 2025, inspection, the weather was mostly cloudy, and the temperature was in the mid-40s °F. Photographs to document the current conditions of the Dam were taken during the inspection and are included in **Appendix B**. A site plan with photograph locations is provided on **Figure 5**, and a site sketch showing unusual conditions of note is provided on **Figure 6**. The approximate elevation of the impoundment at the time of inspection was about El. 715.3± feet, approximately one foot below the spillway crest (crest elevation at the middle of the spillway is about 716.3 feet). Underwater areas were not inspected during this inspection, although, as noted above, the impoundment was lowered so the spillway could be observed in the dry. Housatonic River flow at the downstream United States Geological Survey (USGS) Division Street River gage (USGS Gage 01197500) was reported to be approximately 492 to 422 cubic feet per second (cfs)[2] prior to and after gate operation. A copy of the inspection checklist is provided in **Appendix C**.

In accordance with the OM&M Plan, a dive inspection is to be conducted, at a minimum, every five years and when deemed warranted by a dam safety engineer. The last dive inspection was performed in 2020. As required, an additional underwater dive inspection was performed on July 8 and 9, 2025. This inspection indicated that the overall conditions and integrity of the upstream spillway and training walls, forebay, slide gate, sheet pile, and riprap underwater areas were similar to those observed during the dive inspection conducted in 2020. No progressive deterioration or undermining of the underwater structures was observed which would impact the overall condition and integrity the structures. A summary of the dive inspection is provided in **Appendix I** and significant observations from the dive inspection are included in this report.

2.1.1 General Findings

In general, the Dam was found to be in **SATISFACTORY** condition, which remains unchanged from the previous Phase 1 inspection conducted in November 2023. Specific conditions identified during this Phase 1 inspection are described in more detail in the sections below.

[2] Measured at the USGS Division Street River Gage 01197500. https://waterdata.usgs.gov/monitoring-location/USGS-01197500/#dataTypeId=continuous-00065-0&showFieldMeasurements=true&startDT=2025-11-20&endDT=2025-11-20

2.1.2 Dam

Left and Right Embankment

The top (crest) of the left and right embankment is grassed. Although the ground is slightly uneven, signs of sinkholes, puddles, or depressions in the embankment crests were not observed. The top of the right embankment can be used as vehicular access to the instrumentation shed, and for vegetation and other maintenance. Erosion and rutting were not observed along the top of the Dam.

The right embankment downstream slope is mostly grassed. At the right training wall groin and along the toe, the slope is protected by riprap. There is a slight "low area" in the downstream riprap near the toe of the right embankment. Its approximate dimensions are ±9 feet (left to right) by ±7½ feet (upstream to downstream) by ±4 inches deep. The low area was observed to be of similar size to that observed in past inspections and continues to remain stable since it was marked in February 2020.

Minor sloughing of the riprap downstream of the right training wall at the toe of the right embankment was observed, as was iron staining along the shoreline downstream of the right embankment. These conditions were similar to those during previous quarterly inspections.

Standing water was present beyond the downstream toe of the slope of the right embankment, which is a periodic condition (possibly associated with fluctuations in tailwater and/or surface runoff from the adjacent wooded area). The standing water does not appear to be a result of seepage through the embankment.

The upstream slope of the right embankment is protected with riprap and steel sheet piling. The upstream slope of the left embankment includes steel sheet piling and a soil slope. Overall, no signs of settlement, sliding, unusual movement, or erosion were observed. The steel sheeting on both the left and right embankments appeared to be in good condition.

2.1.3 Appurtenant Structures

Spillway

As noted above, this Phase 1 inspection was conducted under dewatered conditions, allowing for a mostly unobstructed observation of the downstream face and toe of the spillway. Observation of and access to some areas of the spillway were limited by the upstream pool and steepness of the ogee.

Overall, the downstream face of the spillway was observed to be in satisfactory condition. The concrete was uniform and appeared sound with no notable signs of deterioration. The spillway was observed to be in good alignment, with no signs of movement or displacement. Floating timber debris was observed upstream of the safety buoys. After the inspection, the gate was closed by GE's contractor, LB Corporation, and spillway flow was re-established. Generally, the ogee flow appeared to be fairly smooth and steady.

The fifth energy dissipator from the right training wall was damaged with a portion of the concrete missing and with exposed steel reinforcing. This condition of the damaged energy dissipator appeared similar to that during prior inspections. However, there are 37 energy dissipators at the toe of the spillway, and all other energy dissipators were observed to be aligned and intact. Since the energy dissipators function as a unit, it is not

expected that damage to one dissipator will significantly impact their overall function. A log was caught in the energy dissipators on the right downstream spillway apron, but did not appear to be impeding flow.

The fence gate on the right training wall was missing. Prior to and after the inspection, the fence gate was temporarily blocked with plywood to help prevent unauthorized access. The plywood was temporarily removed during the inspection to allow ladder insertion and access to the dewatered spillway.

During the inspection, each spillway weephole was cleaned by LB Corp. The cleaning was performed by inserting a metal rod metal rod into each weephole. The weepholes were sounded during rod insertion to feel for material or debris buildup. No debris or buildup of material was observed or felt during sounding and the upstream end of each weephole felt firm.

During the inspection, a minor leak was observed emanating from an approximately four-inch long crack or joint located below the second from right weephole at the interface between the sloping ogee concrete and the apron concrete. The crack or joint was too narrow to be probed with a folding ruler. Leakage flow was clear with an estimated flow rate less than about ½ gallon per minute (gpm).

Additional areas of minor diffuse leakage were observed emanating from the downstream left masonry training wall with frequency and seepage rate increasing towards the downstream end of the spillway. The leakage was clear with a total estimated flow rate less than about one to two gpm. However, two other areas of leakage through the stone masonry left training wall that had been observed during previous site visits were not observed during the dewatered Phase 1 inspection. As indicated in the 2025 Underwater Dive Inspection Summary (**Appendix I**), tracer dye testing indicates that the leakage source is reservoir water entering through a horizontal joint/crack in the forebay right wall. The left training wall masonry will continue to be visually observed for changes in seepage.

There were no obvious signs of scour or erosion at the toe of the spillway. No separation between the downstream concrete apron, steel sheet piles, and downstream riprap was observed. Riprap coverage along the toe of the dam was continuous and appeared to be in sound condition with no gaps on coverage or contact. Grout bags and riprap along the right downstream training wall appeared to be in place with no scoured areas.

Low-Level Outlet

The low-level outlet structure was accessed via the locked gate to the left of the structure. The intake / penstock gate was opened by the GE's contractor, LB Corporation, prior to the inspection and closed at the end of the inspection to lower the pool and allow dewatered inspection of downstream spillway areas. The gate closing was sequenced so that river flow was maintained. No operability issues were reported.

A new gate actuator was installed on July 30, 2025. The previously reported grease leaks were not observed. The new actuator digital readout indicated the gate was 21 percent open (about 30 inches) during the inspection.

Minor surficial rusting of the trash racks upstream of the low-level outlet was observed. This condition is similar to that observed in previous inspections.

A portion of the left brick forebay wall was repaired with a concrete overlay in the fall of 2023. The repair was observed to be in good condition.

Minor cracking and efflorescence on the upstream and downstream faces of the low-level outlet gate platform concrete structure were observed, along with a crack on the upstream face (downstream forebay wall) below the abandoned operator. These conditions appear similar to those observed in previous inspections.

A crack was observed in the concrete step to the left of the forebay platform, as has been observed since the 2023 Phase 1 inspection. In addition, an approximately 0.4-inch-wide diagonal crack was observed in the lower six courses of the pumphouse downstream brick wall near the right corner, which is a new observation noted during this inspection.

A low area in the penstock was observed during a visual inspection conducted by GZA in October 2021, indicating a possible settled area or "belly." The low area in the penstock is between 100 feet and 180 feet downstream of the low-level outlet, with the lowest area at about 150 feet downstream of the low-level outlet. As a result, penstock investigations began in 2022- 2023 to determine the potential cause of the possible settled area and to determine whether there is active movement or settlement of the penstock. As of the end of 2025, a series of penstock investigations have been performed, including the following: topographic surveys of the area above the penstock and the invert, crown and springlines of the penstock; penstock ovality measurements; a ground penetrating radar (GPR) survey of the area above the penstock; a dive inspection in the forebay upstream of the intake gate and penstock; test borings; test pits; ultrasonic thickness (UT) testing; and internal visual inspections. These investigations were described in the 2023, 2024, and 2025 Penstock Investigations End-of-Year Reports referenced above.

The grassy area above the buried penstock was observed during this inspection. Except as noted below, no areas of settlement were observed. Note that the area above the penstock is topographically surveyed every year as described in the Penstock Investigations End-of-Year Reports. During test pit investigations to support the 2023 penstock investigations, a former railroad trestle was discovered crossing the penstock alignment. This surficial concrete structure consists of a series of interconnected grade beam structures diagonally crossing the penstock alignment about 120 to 140 feet downstream of the gate.

During the November 20, 2025 Phase 1 inspection, GZA's inspection team observed a depression in the area above and to the left of the penstock adjacent to the concrete grade beam structure (former railroad crossing) at about Station 1+30. This depression was located about 4½ feet left of the penstock springline (sidewall). The depression measured up to two feet deep and one to two feet in plan dimensions. The depression was vertical sided and triangular-shaped (in plan view), likely due to the geometry of the adjacent concrete structure. This depression had not been observed during annual penstock inspections (including in October 2025) or during prior quarterly inspections (the most recent of which was conducted in August 2025).[3]

Based on discussion with mill staff, the previously observed "three parallel shallow linear surficial depressions" that were observed during quarterly inspections in 2025 were intentionally created by the mill owners to direct rain runoff away from the mill building. These surficial depressions are not considered a dam safety concern.

[3] A similar depression had been previously observed about four to five feet further left and downstream during a December 1, 2005 visual inspection conducted by GZA. That depression measured about four feet deep and was subsequently filled with concrete. It was documented in a January 2006 visual inspection report.

2.1.4 Instrumentation

Active instrumentation at the Rising Pond Dam includes three staff gages installed at the spillway and five observation wells (OWs) and 23 vibrating wire piezometers (VWPs) installed in the left embankment, spillway, and right embankment. Their locations are shown on **Figures 5** and **6**.

Observation wells and piezometers were measured, and data were collected in accordance with Section 3.1.2 of the OM&M Plan. Observation well and piezometer data, along with the impoundment water levels, from 2010 through the date of the November 2025 inspection are presented in **Appendix G**. The OW and VWP readings between the 2023 Phase 1 and 2025 Phase 1 inspections were reviewed and evaluated as part of this 2025 Phase 1 inspection. Overall, the OW and VWP readings remained within action levels and the 2011 to 2016 expected range (based on historic readings, as required by the OM&M) during this review period. A few readings during this review period were noted as slightly outside of the action levels and 2011 to 2016 expected range; however, these readings were not considered to be a dam safety concern and no action was required. Minor buildup of sediment/silt was measured in monitoring wells GZ-2, GZ-5, and GZ-7. However, the current sediment/silt levels is not impacting the water level readings at these monitoring wells.

The protective casing for monitoring well/piezometer GZ-6 was observed to be slightly tilted, as noted during previous inspections. The cause of the tilt in the casing is unknown. The condition and the tilt of the casing do not appear to be worsening over time, and the tilt has not affected the well or piezometer readings.

Since May 2022, annual grout bag and riprap measurements have been performed at the Dam to monitor potential scour below the downstream right spillway training wall. Measurements are taken in accordance with Section 3.1.2 of the OM&M Plan. To date, measurements have indicated no scour beneath the downstream right spillway training wall, including the latest measurements taken during the second quarterly inspection in 2025. Additionally, no scour was visually observed during the July 2025 dive inspection or during the November 2025 dewatered Phase 1 inspection, confirming the results of grout bag measurements.

Instrumentation plots showing all available readings for each active OW and VWP, and the grout bag measurements at each monitoring point, are included in **Appendix G**. Updates to the action levels are discussed in Section 2.7.4 of this report.

Since the last Phase 1 inspection in 2023, two new staff gages have been installed – one on the right upstream spillway training wall and one on the left side of the forebay. The third staff gate on the right downstream spillway training wall was repainted since the last Phase 1 inspection.

2.1.5 Downstream Area

The downstream area of the Dam is the Housatonic River. Downstream of the Dam are mostly homes, businesses, and secondary roads. USGS stream gage #01197500 is located at the Division Street Bridge, about one mile downstream of the Dam.

2.1.6 Reservoir Area

The reservoir is an impoundment of the Housatonic River.

2.2 CARETAKER INTERVIEW

Kevin Mooney, GE's current Dam Caretaker, was available prior to the visual inspection of the Dam. In addition, GE's dam contractor, LB Corporation, was on-site during the inspection. GZA maintains regular communication with both the Caretaker and the dam contractor regarding the operations, monitoring, and maintenance of the Dam.

2.3 INSPECTION AND MAINTENANCE PROCEDURES

This section briefly summarizes the inspection and maintenance procedures that were specified in the August 2019 OM&M Plan, as amended, and were thus in effect at the time of the November 2025 Phase 1 inspection. Note that the future inspection and maintenance procedures will be those specified in the revised OM&M Plan for Rising Pond Dam that was submitted on December 22, 2025 and approved by EPA on February 12, 2026.

2.3.1 Inspection Procedures

Section 3 of the 2019 OM&M Plan requires the Dam to be inspected by GZA personnel on GE's behalf on a quarterly basis using the forms in Appendix C to that OM&M Plan. In addition to visually inspecting the Dam, the quarterly inspections include photographing specific locations and recording observation well and vibrating wire piezometer levels. That OM&M Plan also requires biennial Phase 1 inspections, as well as post-storm, ice-out, and post-earthquake inspections as warranted.

GE monitors the USGS Advanced Hydrologic Prediction Service, which forecasts river flows on the Housatonic River in Great Barrington. Generally, the sluice gate is left in a closed position; however, during extreme weather events, it may be operated to allow river flow to pass.

The following inspections have been conducted since the 2023 Phase 1 inspection/evaluation:

- Post-storm inspection on December 21, 2023;
- Quarterly Inspections on February 26, May 21, September 3, and November 19, 2024, and February 11, May 20, and August 12, 2025;
- Dive Inspection on July 8 and 9, 2025;
- Topographic and bathymetric surveys conducted the week of July 7, 2025 (with supplemental topographic surveys conducted in October and November 2025); and
- The subject Phase 1 inspection (in a dewatered condition) on November 20, 2025.

Since the 2023 Phase 1 inspection/Evaluation, the following have been performed in support of the ongoing penstock investigations: Topographic survey and ovality measurements performed on December 12, 2023, on December 19, 2024, and on October 23, 2025. As mentioned above, the 2023, 2024, and 2025 Penstock Investigations End-of-Year Reports were submitted to EPA on May 24, 2024, December 19, 2024, and January 9, 2026,.

2.3.2 Maintenance of Dam and Operating Facilities

Sections 4.1 and 4.2 of the 2019 OM&M Plan set forth requirements for maintenance of the Dam. Maintenance includes, but is not limited to, vegetative maintenance, cleaning of the spillway and low-level outlet, gate system maintenance, minor erosion repair, rodent damage control, slope traffic damage control, seepage damage control, riprap damage control, sediment removal where necessary, weephole cleaning, concrete and masonry maintenance, metal component maintenance, spillway toe riprap maintenance, instrumentation repair, security item repair, access road maintenance, and sign maintenance. There is no quantitative action level for sediment removal; the criterion is whether the build-up interferes with the flow of water through flow control structures. GE performs additional maintenance activities as they are required after identification during inspections.

A summary of the monitoring, maintenance, and repair items that were identified at or since the 2023 Phase 1 inspection (including those observed during the November 20, 2025 Phase 1 inspection) and that had not been addressed as of the date of the last prior quarterly inspection (in August 2025) is provided in the maintenance tracking table in **Appendix H**, along with their current status (e.g., completed, scheduled, subject to ongoing monitoring).

2.4 EMERGENCY WARNING SYSTEM

There is no physical early warning system at Rising Pond Dam. Quarterly inspections of the Dam are conducted by dam safety engineers. An Emergency Action Plan (EAP) was developed in 2000, and is updated annually, with the most recent major revision in December 2025, which was attached as Appendix B to the revised OM&M Plan, submitted to EPA on December 22, 2025.

Note that GZA performed an updated dam break analysis and generated updated inundation maps for Rising Pond Dam in accordance with current industry standards. Results were included in the December 2024 EAP and are summarized at the end of Section 2.6 of this report. The updated inundation maps are provided in **Appendix F-3** to this report.

2.5 AWARENESS OF POTENTIAL DAM RELATED SAFETY HAZARDS AT, NEAR, AND ON DAMS

The dam has chain-link fencing to deter spillway access on the left side. Access on the right side is deterred by a locked railroad gate and chain link fencing along the training wall. As indicated in Section 2.1.3, the fence gate on the right training wall is missing. The gate opening is temporarily secured with plywood to help deter unauthorized access. Access to the gate platform and spillway is deterred by locked gates in the fencing. Dam warning signs are installed upstream and downstream of the Dam, and a buoy line is installed upstream of the Dam.

2.6 HYDROLOGIC/HYDRAULIC DATA

Hydrologic and hydraulic (H&H) analyses had previously been performed by GZA as part of the Phase 2 evaluation in 2012. The conclusion of that Phase 2 evaluations was that the spillway has sufficient capacity to safely pass the 100-year Spillway Design Flood (SDF). An evaluation of the 500-year return period flood was also performed. The results of that evaluation indicated that the spillway has sufficient capacity to safely pass the 500-year flood without overtopping the embankments (conservatively assuming that the penstock gate is closed). Specifically, the evaluation indicated that the maximum 500-year flood pool elevation is 726.2 feet, which would allow 0.8

foot of freeboard at the left embankment (El. 727.0 feet) and 1.8 feet of freeboard at the right embankment (El. 728.0 feet). However, the evaluation also indicated that the 500-year flood would overtop the existing concrete forebay section and berm immediately adjacent and to the left of the structure by about 0.2 foot (El. 726.0 feet vs. El. 726.2 feet).

As a result of the latter finding, the elevation of the left earthen berm adjacent to the concrete forebay section and the forebay section itself were raised to elevation of 727.0 feet, allowing 0.8 foot of freeboard above the 500-year flood. This raising project for the concrete forebay and left earthen berm was completed in October and November 2021. The raising project was documented in the GZA letter report titled "Summary of Penstock Gate Inspection/Rehabilitation and Left Embankment/Forebay Dam Raising Activities," dated February 16, 2022 and attached as Appendix I to the February 17, 2022 report on the November 2021 Phase 1 inspection.

The H&H data compiled from previous reports referenced above are as follows:

A.	SDF Return Period	100 year
B.	SDF Elevation	724.4 feet
C.	SDF Inflow (cubic feet per second [cfs])	11,700 cfs
D.	SDF Outflow (cfs)	11,700 cfs
E.	Principal Spillway Capacity (cfs)	17,093 cfs (500-year flood)
F.	Auxiliary Spillway Capacity (cfs)	Not applicable
G.	Low-level Outlet Capacity (cfs)	3,300 cfs (at El. 726.2 feet)
H.	Percentage of the SDF passed	>100%
I.	Maximum Depth of Overtopping for SDF (ft)	Not applicable
J.	Maximum Duration of Overtopping for SDF (hours)	Not applicable

As also mentioned above, an updated dam break analysis for the Rising Pond Dam was conducted by GZA and presented in the December 2024 EAP. Attachment A to the December 2024 EAP describes the current dam break analysis as follows:

> "The objective of this analysis was to determine the resultant flooding depths caused by a breach of the Rising Paper Dam and to estimate the travel time of the flood wave as it progresses downstream. Rising Paper Dam is located on the Housatonic River in Risingdale (Great Barrington), Massachusetts. This analysis was performed using the U.S. Army Corps of Engineers HEC-RAS 2D version 6.5. The dam break scenario is consistent with Massachusetts simplified dam breach standards for significant hazard dams as specified in 302 CMR 10 Dam Safety 10.11(2). The dam break consists of a full breach of the dam earthen embankment while the impoundment level is at the top of embankment elevation and the downstream area has fair-weather conditions. HEC-RAS calculates the peak outflow of the dam based on the breach geometry and the size of the reservoir. The model then estimates the flood wave as it travels downstream.
>
> The results of the dam breach analysis are presented on an inundation map created using Esri ArcMap 10.8.2. The elevation reference datum used in the model was the North American Vertical Datum of 1988

(NAVD88). This map provides comprehensive information about the area affected by the modelled dam breach, including roadway names, stream names, and railroad locations. It also details the flooding impact, showing the incremental rise in water levels, the depth of the floodwaters, the time it takes for the flood wave to travel downstream, and the peak flow at various points."

Based on the information contained in the 2012 Phase 2 Evaluation Report, it is GZA's opinion that the Dam has sufficient spillway capacity to accommodate the SDF as required by MassDCR ODS regulations.

2.7 STRUCTURAL AND SEEPAGE STABILITY

2.7.1 Spillway Structural Stability

Stability and seepage analyses for the spillway had been performed by GZA as part of the Phase 2 evaluation in 2012. GZA evaluated the sliding stability of the concrete/timber crib spillway of the Dam using the gravity method of analysis per Federal Energy Regulatory Commission (FERC) guidelines. The gravity analysis conservatively neglected the contribution of the upstream and downstream spillway aprons to overall stability of the force equilibrium model. A reduction in uplift forces observed in the piezometers at the Dam due to the presence of steel sheeting upstream of the spillway was accounted for in the analysis. The results of the analysis indicated that the calculated factors of safety against sliding meet the criteria contained in the Massachusetts Dam Safety Regulations.

No visual indications of structural instability of the spillway were observed during this inspection.

2.7.2 Embankment Structural Stability

Stability and seepage analyses for three sections along the right embankment had also been performed by GZA as part of the Phase 2 evaluation in 2012. Calculated factors of safety for sudden drawdown from normal and maximum (SDF) pool, steady state seepage at maximum (SDF) pool and seismic loading at normal pool were adequate based on minimum required factors of safety by current Massachusetts Dam Safety Regulations. These evaluations were based on the 100-year SDF pool elevation of 724.4 feet.

As part of the Phase 2 evaluations, liquefaction susceptibility of embankment soils and spillway foundation soils was evaluated using the criteria outlined in the Massachusetts State Building Code (8th Edition). The results of the liquefaction assessment indicated that the foundation and embankment soils are not susceptible to liquefaction during a seismic event less than or equal to a Magnitude 6.0 Richter scale event (approximate return period of 2,475 years).

Although not required by the Dam Safety Regulations, the Phase 2 evaluation also included estimated slope stability factors of safety for rapid drawdown and steady state seepage at the 500-year flood elevation of 726.2 feet. The factors of safety against slope instability for the 500-year flood elevation were greater than 1.0.

No visual indications of structural instability of the embankment were observed during the November 2025 Phase 1 inspection.

2.7.3 Embankment Seepage Stability

Seepage evaluations had been performed by GZA as part of the 2012 Phase 2 evaluation. The Phase 2 evaluations indicated that the upstream sheeting provides an effective groundwater cutoff through the embankment fill and underlying sand/gravel glaciofluvial soils and the embankment provides adequate resistance to seepage with calculated factors of safety against seepage-induced piping of about 4.5.

No visual indications of seepage instability of the embankment were observed during the November 2025 Phase 1 inspection.

Historical seepage has been intermittently observed at the toe of the right embankment close to the river during previous inspections, but does not appear to be a dam safety concern at this time. However, this area will continue to be monitored for potential active seepage.

2.7.4 Instrumentation Action Levels

As previously mentioned, OW water elevation and VWP digit measurements are compared to previously established action levels and historically observed water elevations to help identify potential dam safety concerns. The prior piezometric action levels were based on conditions existing prior to the ca 2012 repairs and included a single action level of elevation 715 feet. As part of the latest Phase 1 evaluation, the existing action level and historic water elevations were re-evaluated to help provide better indicators of potential dam safety concerns. To accomplish this, the 2012 Phase II stability evaluation models for the downstream slope and spillway were used to compare piezometric levels to MassDCR-required factors of safety. The outcome of this update was to establish a two-tier set of Response Values (RVs) to replace the prior single action level, to apply the new RVs to critical instruments, and to conservatively lower the new RVs below the prior action level. The new RVs were separated into Threshold Levels (TLs) and Limiting Levels (LLs) as described below.

TLs denote an upper-limit piezometric level which requires further evaluation by a qualified dam safety engineer. Exceeding a TL is anticipated to require the dam safety engineer to consider more frequent inspection and instrumentation monitoring as well as further engineering evaluations which could consider reservoir level during instrument readings, forecast precipitation/river flow, and review of stability evaluations for specific piezometric readings and reservoir levels. Depending on the results of the TL monitoring and evaluations, further actions may be recommended by the qualified dam safety engineer.

LLs denote an upper-limit piezometric level which requires actions to be taken at the Dam, along with further evaluation by a qualified dam safety engineer. Exceeding an LL is anticipated to require the TL monitoring and evaluations outlined above, along with consideration of specific risk-reduction actions such as lowering the pool or activating the Emergency Action Plan.

The revised Response Values for critical instruments are specified in the tables below. GZA recommends that these RVs be incorporated into the revised OM&M Plan submitted in December 2025 and approved on February 2026 to replace the historic action level shown in the current OM&M Plan. To accomplish this, an amendment to the December 2025 OM&M Plan to include these RVs will be submitted at the same time as the report on the first quarterly inspection for 2026. In the interim, the instruments will be evaluated using these RVs.

Embankment Instrument Response Values			
Instrument	Typical Measured Elevation Range (2011-2025) (ft, NGVD)	Recommended Threshold Level Elevation (ft, NGVD)	Recommended Limiting Level Elevation (ft, NGVD)
GZ-6	695.4 to 698.2	698.7	701.5
GZ-7	694.4 to 699.7	698.7	701.5

Spillway Instrument Response Values[1]			
Instrument	Typical Measured Elevation Range (2013 to 2025) (ft NGVD)	Recommended Threshold Level Elevation (ft, NGVD)	Recommended Limiting Level Elevation (ft, NGVD)
P-7A	698.2 to 701.9	699.0	702.0
P-7B	692.0 to 696.3	696.3	702.0
IP-8A	696.4 to 700.5	699.0	702.0
P-8B	693.6 to 698.2	698.2	702.0
P-9A	697.6 to 701.4	699.0	702.0
P-9B	695.4 to 699.6	699.0	702.0
P-12B	695.2 to 700.4	698.0	699.9
P-12C	674.0 to 701.3	698.0	699.9
P-14B	690.3 to 695.2	695.2	699.9
P-14C2	697.3 to 700.7	698.0	699.9

1. Spillway RVs should be applied for reservoir levels up to three feet of flow over the spillway (pool level about elevation 719 feet). For greater pool levels, it will be necessary to consult with the dam safety engineer.

For the remaining instrumentation typically read during quarterly maintenance inspections and biennial Phase 1 inspections (i.e., the instruments not shown in the table above), GZA recommends establishing TLs equivalent to the historic measurement range after the ca 2012/2013 dam repairs. The historic value range will be included in future quarterly maintenance inspection forms and are visually depicted by the data included on the instrumentation data plots provided in **Appendix G**.

3.0 ASSESSMENT AND RECOMMENDATIONS

3.1 ASSESSMENT

In general, the overall condition of the Rising Pond Dam during the November 20, 2025, Phase 1 visual inspection was judged to be **SATISFACTORY**. This overall condition rating remains unchanged from the previous Phase 1 inspection conducted in November 2023. Based on the results of this inspection, the Dam is in compliance with MassDCR ODS regulations.

During the November 20, 2025 visual inspection, the Dam was found to have the following unusual conditions of note (several of which had also been noted in the prior Phase 1 inspection and/or in intervening quarterly inspections):

1. Some rust was present on the forebay trash rack. The rust appears similar to that observed during prior inspections and does not appear to be affecting the trash rack integrity based on the July 2015 dive inspection.
2. Vegetation growth was observed in the right stone masonry wall of the forebay.
3. A horizontal joint/crack was observed in the right forebay wall during the July 2025 dive inspection, as summarized in the memorandum provided in **Appendix I** to this Phase 1 report.
4. A crack in the forebay concrete was visible on the upstream face below the abandoned operator. This crack has been observed in prior inspections and appears to be a stable condition.
5. Minor cracking and efflorescence were observed on the downstream face of the gate platform (low-level outlet). This condition has been observed in prior inspections and appears to be a stable condition.
6. A crack was observed in the concrete step to the left of the gate platform. The crack does not affect use of the step.
7. A diagonal crack approximately 0.4 inch wide was observed in the brick wall of the pumphouse near the downstream right corner. This is a new observation made during the 2025 Phase 1 inspection.
8. A low area in the penstock, indicating a possible settled area or "belly," was observed, as it had been during previous inspections and has been investigated since 2023. Repairs are recommended as part of the ongoing penstock investigations.
9. A surficial depression was observed at the former railroad trestle about 130 feet downstream of the gate platform and about 4½-feet left of the penstock springline (sidewall). This triangular, vertical-sided depression measured up to two feet deep and one to two feet in plan dimensions.
10. Floating timber debris was observed upstream of the safety buoys.
11. Timber debris was observed in the energy dissipators at the downstream (right side) toe of the spillway.
12. Minor damage to one of the energy dissipators (fifth from the right) was observed; this damage is not anticipated to impact the overall effectiveness of the energy dissipators.
13. Minor leakage was observed from a four-inch long crack or joint in the right side of the downstream face of the spillway near the sloping ogee concrete and the apron concrete. This is a new observation made during the 2025 dewatered Phase 1 inspection.

14. Areas of minor diffuse leakage were observed from the downstream left masonry training walls.
15. There was a missing gate in the right spillway training wall fence. This opening has been temporarily sealed using plywood.
16. A low area of riprap stones was observed at the downstream toe of slope on right embankment, as it has been in prior inspections, and appears to be a stable condition.
17. Minor sloughing of the riprap downstream of the right training wall. This condition has been observed in prior inspections and appears to be a stable condition.
18. Iron staining was observed along the shoreline at the toe of the right embankment, but no active seepage was observed.
19. Standing water was observed beyond the toe of the right training embankment. This condition is periodically observed during inspections and does not appear to be the result of seepage through the embankment.
20. The protective casing for monitoring well/piezometer GZ-6 was observed to be slightly tilted, as it has been during prior inspections; this condition appears to be stable.
21. Minor buildup of sediment/silt was measured in monitoring wells GZ-2, GZ-5, and GZ-7. However, the current sediment/silt levels is not impacting the water level readings at these monitoring wells.

The locations of these conditions are shown on **Figure 6**.

The following tables presents a comparison of the unusual conditions of note identified during the prior Phase 1 inspection in November 2023 to current conditions and the actions taken to address them.

Unusual Condition Identified in 2023 Phase 1 Inspection	***Resolution or Current Status***
Some rust was present on the forebay trash rack. The rust appears similar to that observed during prior inspections and does not appear to be affecting the trash rack integrity.	No change observed. The trash rack was inspected during the 2025 dive inspection. The condition appears stable with no indication of progressive deterioration and will continue to be monitored.
Minor woody vegetation was observed on the upstream slopes of the left and right embankments.	Minor woody vegetation was not observed during the current inspection; however this is a recurrent condition that will continue to be maintained and monitored.
There were two small cracks in the gate operator casing with accompanying minor grease leaks which do not affect gate operation.	Resolved. A new gate actuator was installed on July 30, 2025. No grease leaks were observed during the 2025 Phase 1 Inspection.
A crack in the forebay concrete was visible on the upstream face below the abandoned operator. This crack has been observed in prior inspections and appears to be a stable condition.	No change observed. The condition is not considered to be affecting the integrity of the structure and will continue to be monitored.
A crack was observed in the concrete step to the left of the gate platform. The crack does not affect use of the step.	No change observed. The condition is not considered to be affecting the integrity of the structure and will continue to be monitored.

Unusual Condition Identified in 2023 Phase 1 Inspection	*Resolution or Current Status*
Logs were observed caught on the safety buoys upstream of the spillway. Two logs were also observed to be caught in the energy dissipators downstream of the spillway – one on the left and one on the right. These logs did not appear to be impacting flow.	Floating timber was observed upstream of the safety buoys, and one log was lodged on the energy dissipators. This is a recurrent condition that will continue to be monitored and logs will be removed if hindering spillway flow capacity.
The three staff gages installed at the Dam are becoming difficult to read due to faded numbers, particularly where water levels typically fluctuate.	Resolved. Since the last Phase 1 inspection in 2023, two new staff gages have been installed – one on the right upstream spillway training wall and one on the left side of the forebay. The third staff gate on the right downstream spillway training wall was repainted since the last Phase 1 inspection.
Previously observed apparent seeps through the left spillway training wall were not noted during this inspection. Photographs indicate that these seeps have been intermittently present over the past decade and do not appear to be affecting dam function.	Not observed during the current inspection, likely due to lowered pool level. As indicated in Section 2.1.3, tracer dye testing indicated the source is a horizontal crack/joint in the right forebay wall. This condition will continue to be monitored and joint/crack filling is recommended for the next dive inspection in 2030.
A low area of riprap stones was observed at the downstream toe of slope on right embankment, as it was in prior inspections, and appears to be a stable condition.	No change observed. This condition appears to be stable and will continue to be monitored.
Monitoring well GZ-6 is slightly tilted, as was previously observed; this condition appears to be stable.	No change observed. The cause of the tilt in the casing is unknown; however, the condition and the tilt of the casing do not appear to be worsening over time, and the tilt has not affected the well or piezometer readings. Repairs to address the tilt are not considered necessary at this time. GZ-6 will continue to be monitored and read, along with the other instruments at the Dam.
Broken / missing Lexan panels and a non-functional gate latch were observed along the top of the right spillway training wall.	Broken/missing panels were replaced, but the fence gate is missing and temporarily secured with plywood. The gate will be replaced during regularly scheduled maintenance activities in 2026.
A low area in the penstock, indicating a possible settled area or “belly,” was observed during previous inspections. An internal penstock inspection was not conducted as part of the November 15, 2023 Phase 1 inspection; however, the “belly” was observed in 2023 during the ongoing penstock investigations.	No change observed. Repairs have been recommended to address this condition, as documented in the 2025 Penstock Investigations End-of-Year Report, submitted to EPA on January 9, 2026.

3.2 RECOMMENDED ACTIVITIES

GZA recommends the activities described below to address the unusual conditions of note listed at the beginning of Section 3.1, in addition to complying with the regular maintenance and repair requirements specified in the applicable OM&M Plan.

Note the investigations into the low area in the penstock are currently ongoing. As of the end of 2025, a series of penstock investigations had been performed. The 2023, 2024, and 2025 Penstock Investigations End-of-Year Reports were submitted to EPA on May 24, 2024, December 19, 2024; and January 9, 2026, respectively. The 2025 End-of-Year Report included a recommendation for repairs to improve the condition of the penstock, a schedule for design and implementation of those repairs, and recommendations for continued monitoring in the meantime.

3.2.1 Studies and Analyses

There are no studies or analyses recommended at this time. The current status of the studies and analyses that were recommended in the 2023 Phase 1 are provided below:

Recommendation	Current Status/Schedule
1. Investigate the low area in the penstock.	Ongoing. The penstock investigations were documented in 2023, 2024, and 2025 Penstock Investigations End-of-Year Reports, submitted to EPA on May 24, 2024, December 19, 2024, and January 9, 2026, respectively. Repairs and ongoing monitoring were recommended in the 2025 Penstock Investigations End-of-Year Report. See Section 3.3 of this report for more detail.
2. Perform an updated dam break analysis in accordance with current MassDCR ODS guidelines.	Completed. An updated dam break analysis was performed and included in the 2024 EAP update for the Dam, as described in Section 2.6 of this report.
3. Establish DBVs for the OWs and VWPs by determining the maximum allowable reading that will meet current MassDCR ODS stability criteria. Threshold and action levels should also be established as early warning signs of a potential dam safety concerns. DBVs and threshold /action levels should be established for critical instrumentation (as determined during the evaluation).	Completed. The existing analysis of record on file with MassDCR ODS (ca 2012) was reviewed and Response Values for OWs and VWPs critical to dam safety have been established, as discussed in Section 2.7.4 of this report.

It is recommended that the Response Values for OWs and VWPs for the instruments critical to dam safety (as identified in Section 2.7.4 of this report) be incorporated into and replace the historic action level shown in the current OM&M Plan. As noted in Section 2.7.4, the revised OM&M Plan will be amended to include these RVs at the time of submission of the first quarterly inspection for 2026.

3.2.2 Monitoring and Maintenance

GZA recommends that the Dam be maintained and monitored in accordance with the applicable OM&M Plan. In addition, the following items should be monitored and maintained:

Recommendation	Current Status/Schedule
1. Continue to monitor the rust on the trash rack.	This condition is monitored during quarterly and biennial inspections, as well as during dive inspections, which are completed every five years. The most recent dive inspection was conducted in July 2025 (see **Appendix I**) and the next one is scheduled for 2030.
2. Remove the vegetation growth observed in the right stone masonry wall of the forebay.	Vegetation will be cleared as part of ongoing vegetation maintenance in 2026.
3. Continue to monitor the cracking and efflorescence below the abandoned operator in the downstream forebay concrete wall (gate platform upstream face), and the downstream face of the gate platform.	This condition is monitored during quarterly and biennial inspections.
4. Continue to monitor the crack at gate platform concrete step for potential future repair if needed.	This condition is monitored during quarterly and biennial inspections.
5. Monitor the diagonal crack in the downstream pumphouse brick wall near the right corner.	This condition will be monitored during future quarterly and biennial inspections.
6. Monitor the surficial depression adjacent to the former railroad trestle.	This condition will be monitored during future quarterly and biennial inspections
7. Remove the floating timber debris upstream of the safety buoys.	The debris will be removed during planned maintenance activities in 2026.
8. Continue to monitor the debris on tailrace, forebay trash racks, area around forebay, spillway, upstream buoys, and energy dissipators for potential removal where needed.	These conditions are monitored during quarterly and biennial inspections and debris will be removed when necessary.

Recommendation	Current Status/Schedule
9. Continue to monitor the minor damage to the energy dissipator (fifth from the right) and the overall effectiveness of the energy dissipators. Conduct repairs if needed.	The energy dissipators are monitored during quarterly and biennial inspections to assess their effectiveness.
10. Monitor the minor leak emanating from a four-inch long crack or joint located below the second from right weephole at the interface between the sloping ogee concrete and the apron concrete and the minor diffuse leakage at the downstream left masonry training wall. (This leak is typically obscured by spillway flow)	This leakage will be monitored during the planned 2029 Phase 1 inspection under dewatered conditions and, if visible, during low-flow or dewatered conditions that occur during other future inspections.
11. Continue to monitor the location of the leakage through the left spillway training wall.	This area is monitored for leakage during quarterly and biennial inspections.
12. Continue to visually monitor the low area of riprap stones at the downstream toe of slope on the right embankment.	This condition is monitored during quarterly and biennial inspections.
13. Monitor the minor sloughing of the riprap downstream of the right training wall.	This condition is monitored during quarterly and biennial inspections.
14. Continue to monitor for seepage (only iron staining observed), if active, at the downstream toe of the right embankment.	This area is monitored for potential seepage during quarterly and biennial inspections.
15. Continue to monitor the slight tilt in well GZ-6.	This condition is monitored during quarterly and biennial inspections.
16. Continue to monitor sediment buildup in observations wells.	Minor sediment buildup was measured in OWs GZ-2, GZ-5, and GZ-7. This condition will continue to be monitored, and if sediment becomes excessive and begins to impact readings, the wells will be flushed.

3.2.3 Minor Repairs

GZA recommends the following minor repairs be performed at the Dam:

Recommendation	Schedule
1. Fill the crack in the forebay platform step.	The joint will be filled in 2026.
2. Repoint the diagonal crack in the brick pumphouse facade.	Repointing will be performed in 2026.
3. Fill the surficial depression adjacent to the former railroad trestle with friable material such as washed peastone.	The surficial depression will be filled during planned maintenance activities in 2026.
4. Replace the missing gate along the top of the right spillway training wall. In the interim, continue securing the gate opening to prevent unauthorized access.	The gate will be replaced during planned maintenance activities in 2026.
5. Fill horizontal joint in right forebay wall (below normal pool and likely source of right spillway training wall leakage).	Joint filling will be performed during next scheduled dive inspection in 2030.

3.3 REMEDIAL MODIFICATIONS

GZA recommends that the following remedial modification be performed at the Dam:.

Recommendation	Current Status/Schedule
1. Construct structural lining within the penstock as described in the 2025 Penstock Investigations End-of-Year Report.	As recommended in the 2025 Penstock Investigations End-of-Year Report, initiate the design for a structural lining system in 2026, with final design and construction of the structural lining system targeted for 2027.

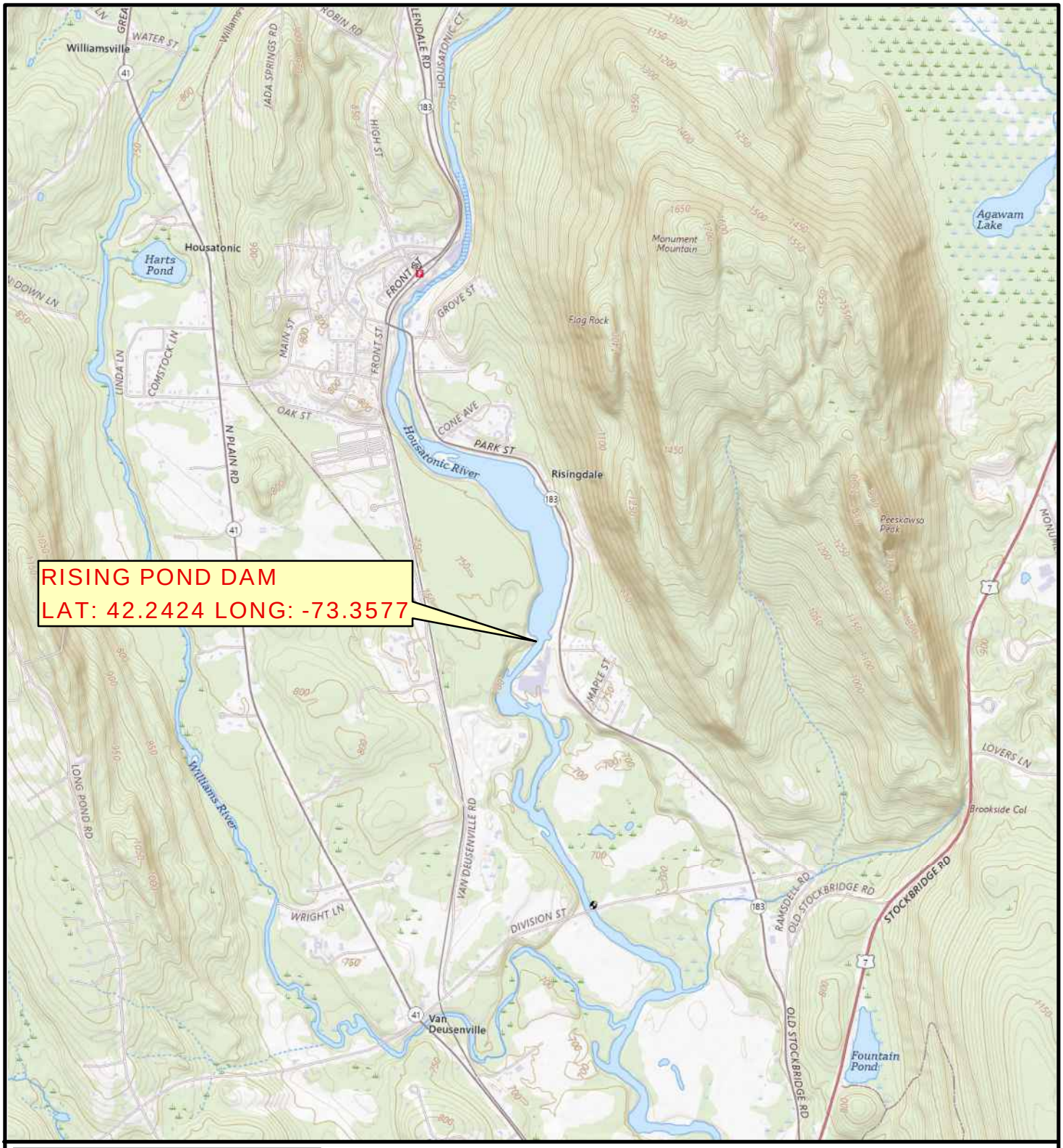
3.4 ALTERNATIVES

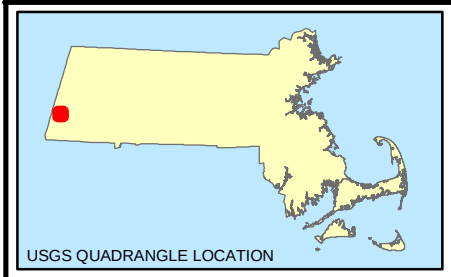
There are no alternatives that need to be considered at this time. Dam removal is not considered a feasible alternative due to the importance impounding existing sediments that are impacted by polychlorinated biphenyls. Maintenance-level work and minor repairs are considered necessary to maintain the safety and functionality of the Dam in the long term.

3.5 OPINION OF PROBABLE CONSTRUCTION COSTS

An opinion of probable construction costs has not been prepared since the above-recommended repairs are minor and the remedial modification design has not been developed.

FIGURES





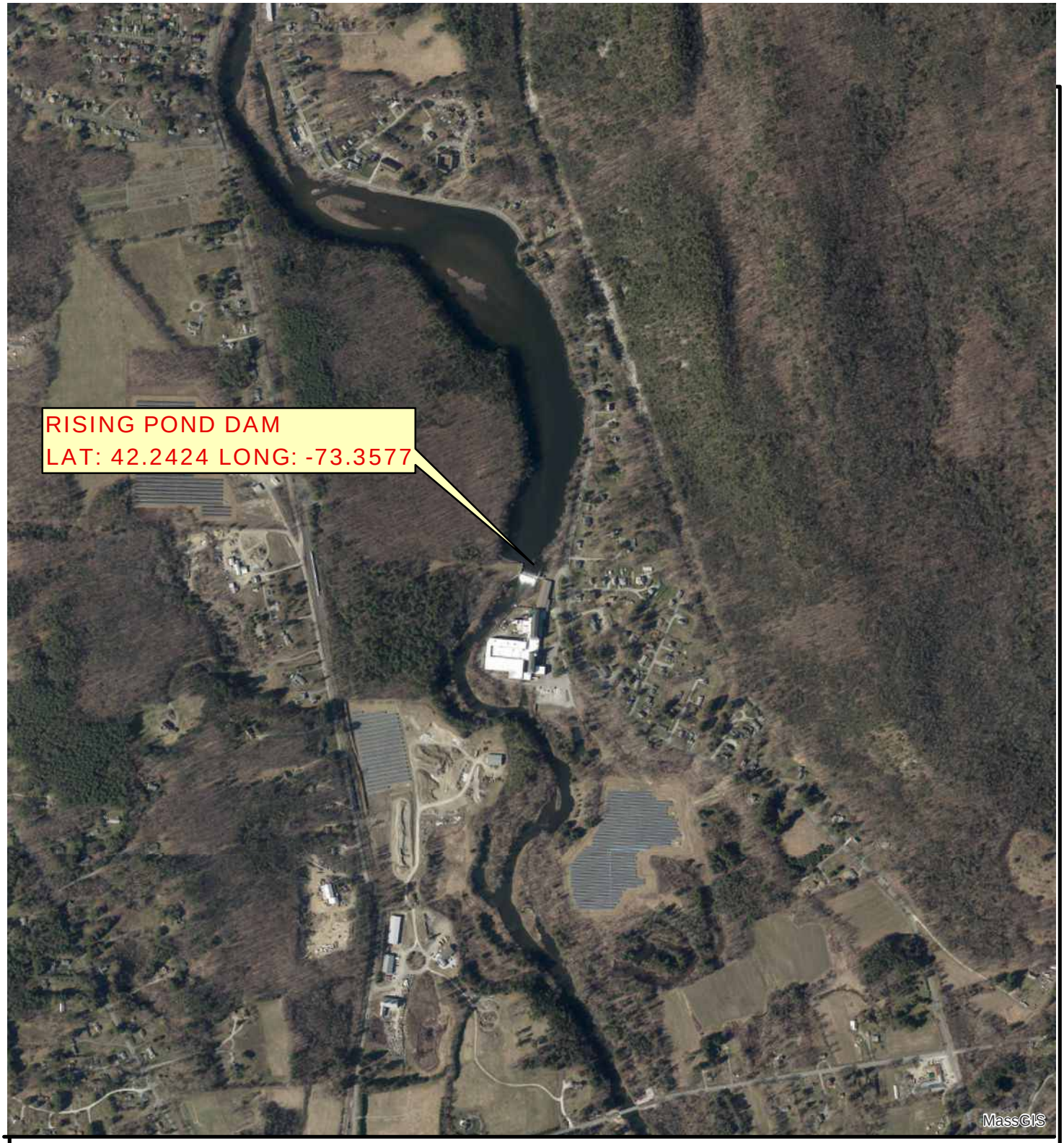
SOURCE : THIS MAP CONTAINS THE USGS NATIONAL TOPOGRAPHIC MAP SERVICE: NATIONAL BOUNDARIES DATASET, 3DEP ELEVATION PROGRAM, GEOGRAPHIC NAMES INFORMATION SYSTREM, NATIONAL HYDROGRAPHY DATASET, NATIONAL LAND COVER DATABASE, NATIONAL STRUCTURES DATASET AND NATIONAL TRANSPORTATION DATASET; USGS GLOBAL ECOSYSTEMS; US CENSUS BUREAU TIGER/LINE DATA; USFS ROAD DATA; NATURAL EARTH DATA; U.S. DEPARTMENT OF STATE HUMANITARIAN INFORMATION UNIT; AND NOAA NATIONAL CENTERS FOR ENVIRONMENTAL INFORMATION, U.S. COASTAL RELIEF MODEL. DATA REFRESHED APRIL, 2023.

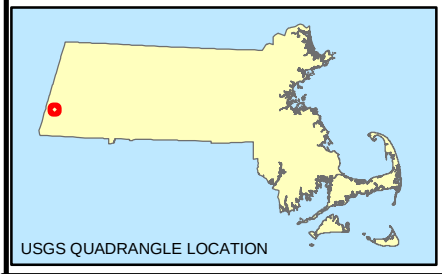
0 1,000 2,000 4,000
SCALE IN FEET

Data Supplied by : esri USGS

N W E S

GZA	PROJ. MGR.: SDK DESIGNED BY: RSG REVIEWED BY: JDA OPERATOR: AJP DATE: 7/22/2024	LOCUS PLAN	JOB NO. 19896.81
		RISING POND DAM (MA00250) GREAT BARRINGTON, MASSACHUSETTS	FIGURE NO. 1





SOURCE: THE 2023 AERIAL IMAGERY MAP SERVICE WAS DISTRIBUTED BY MASSGIS ON MAY 3, 2024. THE FUNDING FOR THIS IMAGERY WAS PROVIDED BY MASSDOT, THE STATE 911 DEPARTMENT AND THE EXECUTIVE OFFICE OF TECHNOLOGY SERVICES AND SECURITY (EOTSS).

Data Supplied by : MASS GIS

0 500 1,000 2,000 3,000

SCALE IN FEET

N W E S

PROJ. MGR.: SDK
DESIGNED BY: RSG
REVIEWED BY: JDA
OPERATOR: AJP

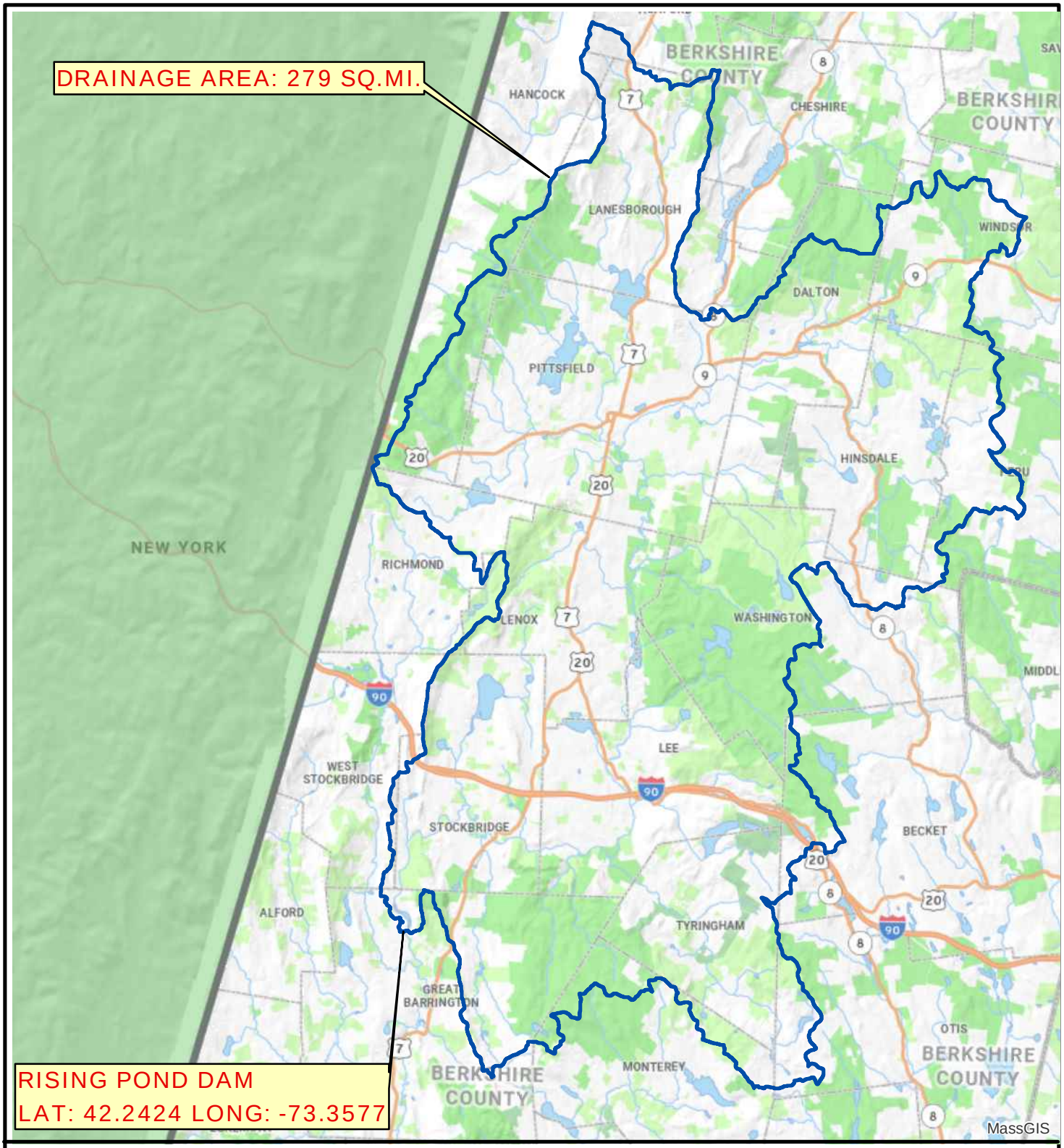
DATE: 12/24/2025

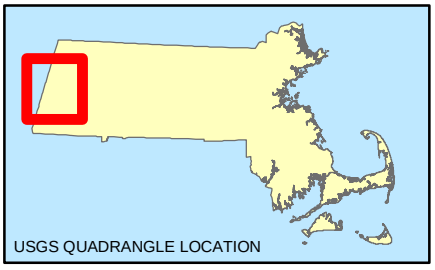
AERIAL PHOTOGRAPH

RISING POND DAM (MA00250)
GREAT BARRINGTON, MASSACHUSETTS

JOB NO. 19896.61

FIGURE NO. 2





SOURCE : THIS MAP CONTAINS THE MASSGIS SCANNED THE USGS TOPOGRAPHIC QUADRANGLES TO CREATE A DIGITAL DATABASE THAT CAN PROVIDE IMAGES OF THE PAPER MAPS, PUBLISHED OCTOBER 14, 2022. THESE IMAGES CAN BE USED AS A BACKDROP FOR PLOTTING VECTOR DATA AND FOR INTERPRETATION AND ANALYSIS.THIS SERVICE USES UNIFORM NATIONALLY RECOGNIZED DATUM AND CARTOGRAPHY STANDARDS AND A VARIETY OF AVAILABLE SOURCES FROM SEVERAL DATA PROVIDERS.

Data Supplied by : MASS GIS, USGS

0 10,000 20,000 40,000 Feet

N W E S

PROJ. MGR.: SDK
DESIGNED BY: RSG
REVIEWED BY: JDA
OPERATOR: RSG

DATE: 12/25/2025

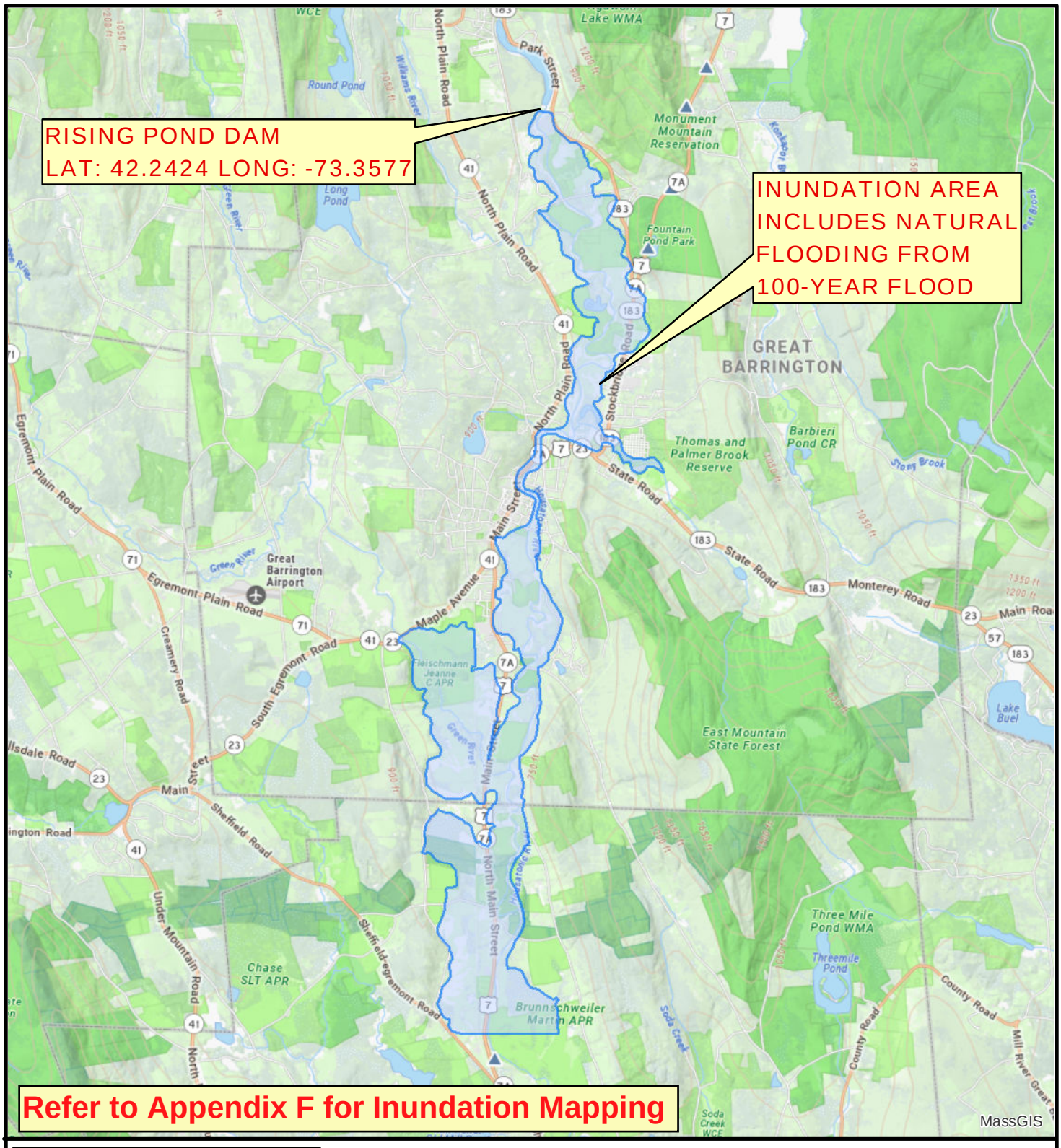
DRAINAGE AREA

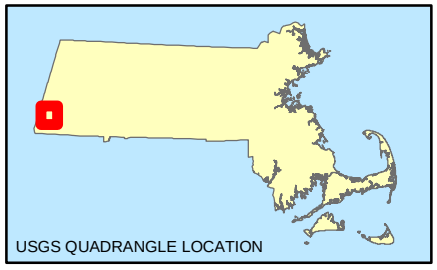
RISING POND DAM (MA00250)
GREAT BARRINGTON, MASSACHUSETTS

JOB NO. 19896.61

FIGURE NO. 3

 \\GZANOR\Jobs\19,000-20,999\19896\19896-50.LAG\3-WORK\06 - Phase 1 Inspections (2-years)\Rising\2023 11 15 Phase I\Figures\GIS\Figure3-DrainageArea.mxd, 7/22/2024, 11:08:20 AM, alexander.perez





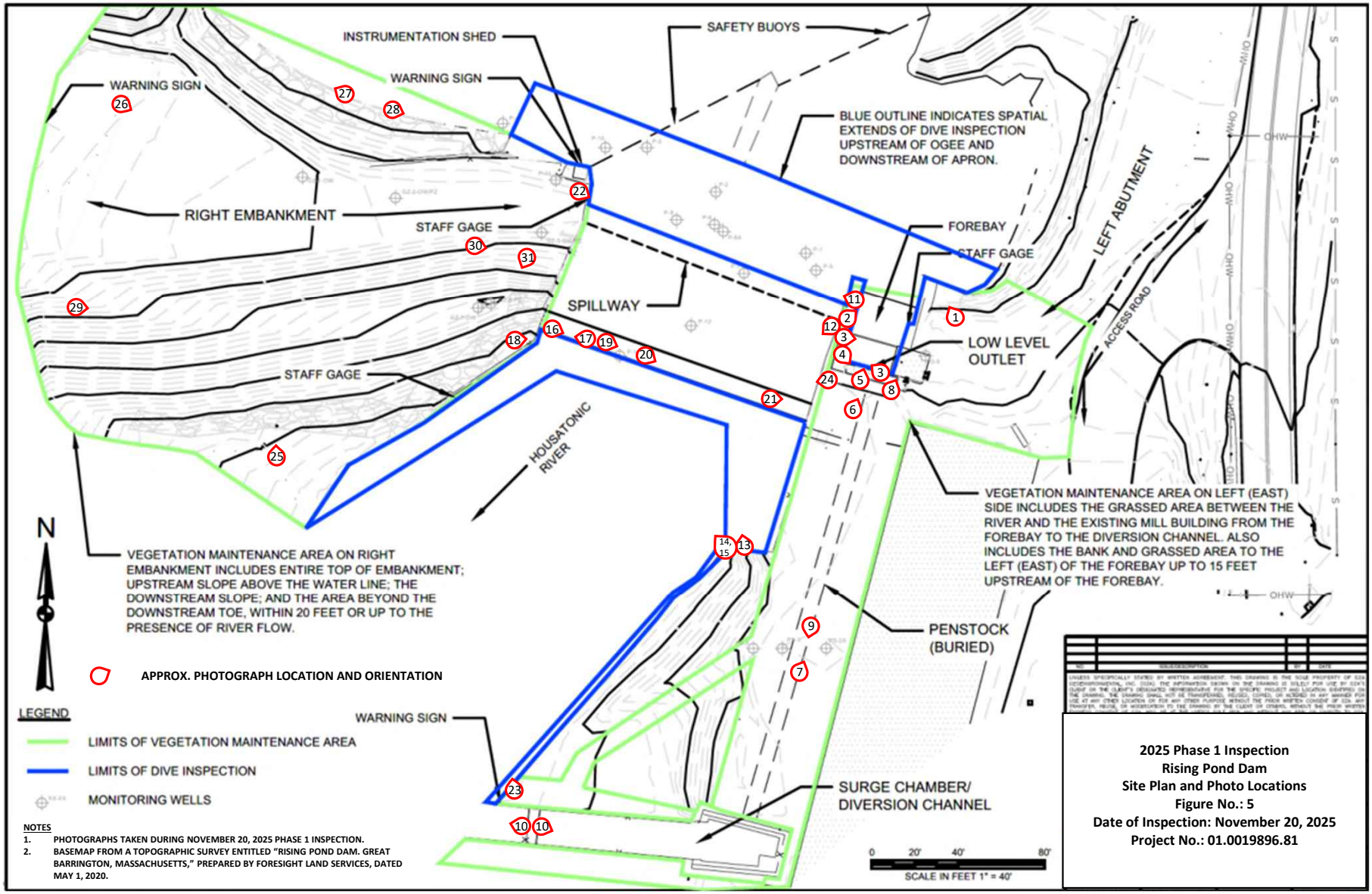
SOURCE : THIS MAP CONTAINS THE MASSGIS SCANNED THE USGS TOPOGRAPHIC QUADRANGLES TO CREATE A DIGITAL DATABASE THAT CAN PROVIDE IMAGES OF THE PAPER MAPS, PUBLISHED OCTOBER 14, 2022. THESE IMAGES CAN BE USED AS A BACKDROP FOR PLOTTING VECTOR DATA AND FOR INTERPRETATION AND ANALYSIS.THIS SERVICE USES UNIFORM NATIONALLY RECOGNIZED DATUM AND CARTOGRAPHY STANDARDS AND A VARIETY OF AVAILABLE SOURCES FROM SEVERAL DATA PROVIDERS.

Data Supplied by : MASSGIS USGS

0 2,500 5,000 10,000 Feet

N W E S

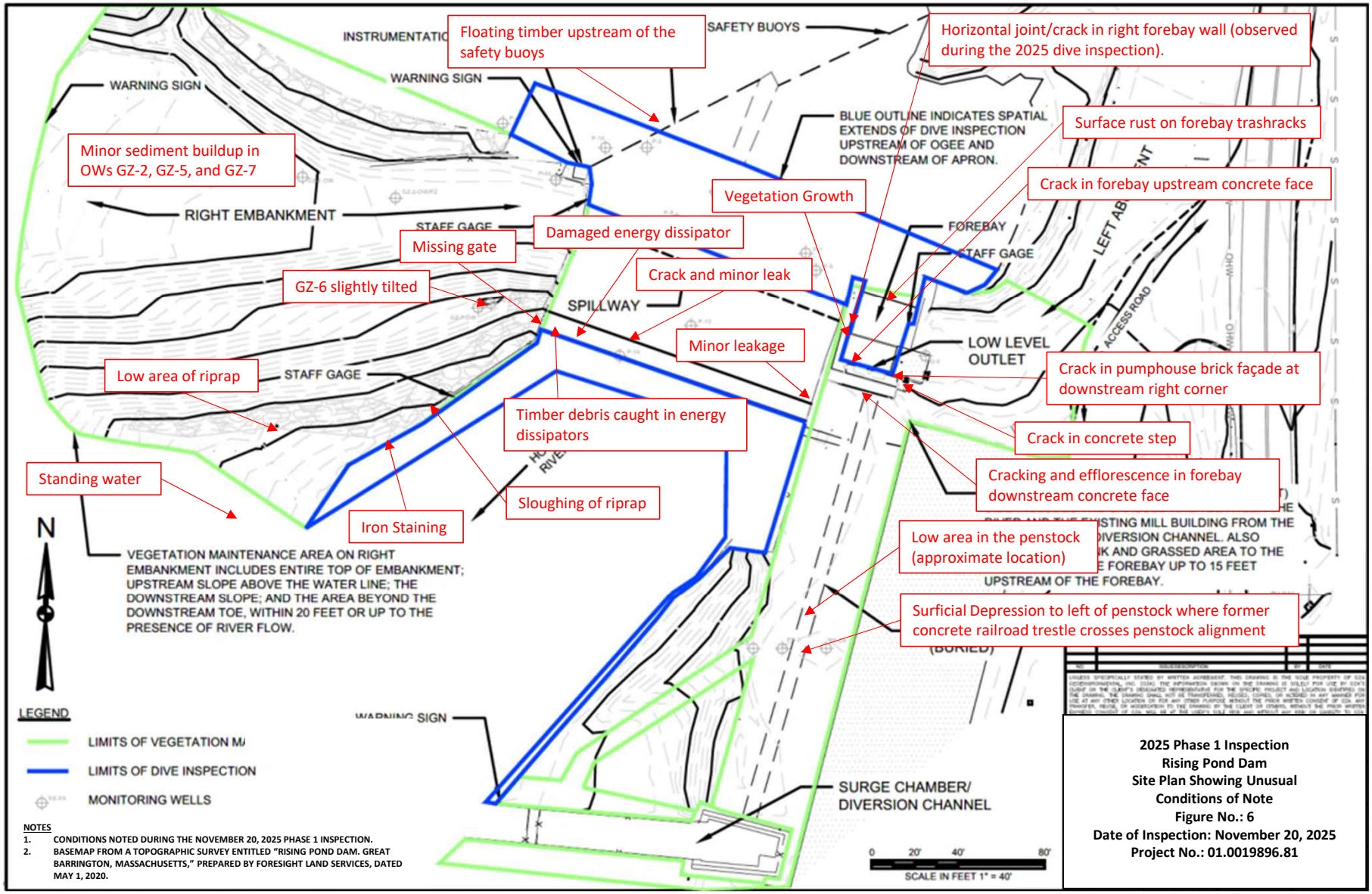
GZA	PROJ. MGR.: SDK DESIGNED BY: RSG REVIEWED BY: JDA OPERATOR: AJP DATE: 12/25/2025	DAM AND DOWNSTREAM AREA	JOB NO. 19896.61
		RISING POND DAM (MA00250) GREAT BARRINGTON, MASSACHUSETTS	FIGURE NO. 4



NOTES

1. PHOTOGRAPHS TAKEN DURING NOVEMBER 20, 2025 PHASE 1 INSPECTION.
2. BASEMAP FROM A TOPOGRAPHIC SURVEY ENTITLED "RISING POND DAM. GREAT BARRINGTON, MASSACHUSETTS," PREPARED BY FORESIGHT LAND SERVICES, DATED MAY 1, 2020.

2025 Phase 1 Inspection
Rising Pond Dam
Site Plan and Photo Locations
Figure No.: 5
Date of Inspection: November 20, 2025
Project No.: 01.0019896.81


Floating timber upstream of the safety buoys
Horizontal joint/crack in right forebay wall (observed during the 2025 dive inspection).
Surface rust on forebay trashracks
Crack in forebay upstream concrete face
Minor sediment buildup in OWs GZ-2, GZ-5, and GZ-7
Vegetation Growth
Damaged energy dissipator
Missing gate
Crack and minor leak
GZ-6 slightly tilted
Minor leakage
Low area of riprap
Timber debris caught in energy dissipators
Crack in pumphouse brick façade at downstream right corner
Crack in concrete step
Standing water
Cracking and efflorescence in forebay downstream concrete face
Sloughing of riprap
Iron Staining
Low area in the penstock (approximate location)
Surficial Depression to left of penstock where former concrete railroad trestle crosses penstock alignment
INSTRUMENTATIC
SAFETY BUOYS
WARNING SIGN
WARNING SIGN
BLUE OUTLINE INDICATES SPATIAL EXTENDS OF DIVE INSPECTION UPSTREAM OF OGEE AND DOWNSTREAM OF APRON.
RIGHT EMBANKMENT
STAFF GAGE
FOREBAY
STAFF GAGE
LEFT AB
SPILLWAY
ACCESS ROAD
LOW LEVEL OUTLET
STAFF GAGE
N
VEGETATION MAINTENANCE AREA ON RIGHT EMBANKMENT INCLUDES ENTIRE TOP OF EMBANKMENT; UPSTREAM SLOPE ABOVE THE WATER LINE; THE DOWNSTREAM SLOPE; AND THE AREA BEYOND THE DOWNSTREAM TOE, WITHIN 20 FEET OR UP TO THE PRESENCE OF RIVER FLOW.
EXISTING MILL BUILDING FROM THE DIVERSION CHANNEL. ALSO K AND GRASSED AREA TO THE E FOREBAY UP TO 15 FEET UPSTREAM OF THE FOREBAY.
(BURIED)
WARNING SIGN
SURGE CHAMBER/ DIVERSION CHANNEL
LEGEND
LIMITS OF VEGETATION MA
LIMITS OF DIVE INSPECTION
MONITORING WELLS
0 20' 40' 80'
SCALE IN FEET 1" = 40'
NOTES
1. CONDITIONS NOTED DURING THE NOVEMBER 20, 2025 PHASE 1 INSPECTION.
2. BASEMAP FROM A TOPOGRAPHIC SURVEY ENTITLED "RISING POND DAM. GREAT BARRINGTON, MASSACHUSETTS," PREPARED BY FORESIGHT LAND SERVICES, DATED MAY 1, 2020.
2025 Phase 1 Inspection
Rising Pond Dam
Site Plan Showing Unusual Conditions of Note
Figure No.: 6
Date of Inspection: November 20, 2025
Project No.: 01.0019896.81

APPENDIX A – LIMITATIONS

DAM ENGINEERING REPORT LIMITATIONS

Use of Report

1. GeoEnvironmental, Inc. (GZA) prepared this report on behalf of, and for the exclusive use of the General Electric Company, (Client) for the stated purpose(s) and location(s) identified in the Report. Use of this report, in whole or in part, at other locations, or for other purposes, may lead to inappropriate conclusions; and we do not accept any responsibility for the consequences of such use(s). Further, reliance by any party not identified in the agreement, for any use, without our prior written permission, shall be at that party's sole risk, and without any liability to GZA.

Standard of Care

2. Our findings and conclusions are based on the work conducted as part of the Scope of Services set forth in the Report and/or proposal, and reflect our professional judgment. These findings and conclusions must be considered not as scientific or engineering certainties, but rather as our professional opinions concerning the limited data gathered during the course of our work. Conditions other than described in this report may be found at the subject location(s).
3. Our services were performed using the degree of skill and care ordinarily exercised by qualified professionals performing the same type of services at the same time, under similar conditions, at the same or a similar property. No warranty, expressed or implied, is made.

General

4. The observations described in this report were made under the conditions stated therein. The conclusions presented were based solely upon the services described therein, and not on scientific tasks or procedures beyond the scope of described services or the time and budgetary constraints imposed by the Client.
5. in preparing this report, GZA relied on certain information provided by the Client, state and local officials, and other parties referenced therein available to GZA at the time of the evaluation. GZA did not attempt to independently verify the accuracy or completeness of all information reviewed or received during the course of this evaluation.
6. Any GZA hydrologic analysis presented herein is for the rainfall volumes and distributions stated herein. For storm conditions other than those analyzed, the response of the site's spillway, impoundment, and drainage network has not been evaluated.
7. Observations were made of the site and of structures on the site as indicated within the report. Where access to portions of the structure or site, or to structures on the site was unavailable or limited, GZA renders no opinion as to the condition of that portion of the site or structure. In particular, it is noted that water levels in the impoundment and elsewhere and/or flow over the spillway may have limited GZA's ability to make observations of underwater portions of the structure. Excessive vegetation, when present, also inhibits observations.
8. In reviewing this Report, it should be realized that the reported condition of the dam is based on observations of field conditions during the course of this study along with data made available to GZA. It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued inspection and care can there be any chance that unsafe conditions be detected.

Compliance with Codes and Regulations

9. We used reasonable care in identifying and interpreting applicable codes and regulations. These codes and regulations are subject to various, and possibly contradictory, interpretations. Compliance with codes and regulations by other parties is beyond our control.
10. This scope of work does not include an assessment of the need for fences, gates, no trespassing signs, swimming or boating barriers, repairs to existing fences and railings and other items which may be needed to minimize trespass and provide greater security for the facility and safety to the public. An evaluation of the project for compliance with OSHA rules and regulations is also excluded.

Additional Services

11. It is recommended that GZA be retained to provide services during any future: site observations, explorations, evaluations, design, implementation activities, construction and/or implementation of remedial measures recommended in this Report. This will allow us the opportunity to: i) observe conditions and compliance with our design concepts and opinions; ii) allow for changes in the event that conditions are other than anticipated; iii) provide modifications to our design; and iv) assess the consequences of changes in technologies and/or regulations.

APPENDIX B – PHOTOGRAPHS

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
1	11/20/2025

Direction Photo Taken:
Upstream

Description:
Upstream impoundment from near the left abutment.

Photo No.	Date:
2	11/20/2025

Direction Photo Taken:
Upstream

Description:
Forebay trashracks. Note minor surficial rusting of the steel.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
3	11/20/2025

Direction Photo Taken:
Left Photo: Right
Right Photo: Left

Description:
(Left Photo): Right forebay wall. Note vegetation growth in the right stone masonry wall.

(Right Photo): Left forebay brick wall. Note, repairs made to the wall in the fall of 2023. Repairs appeared to be intact.

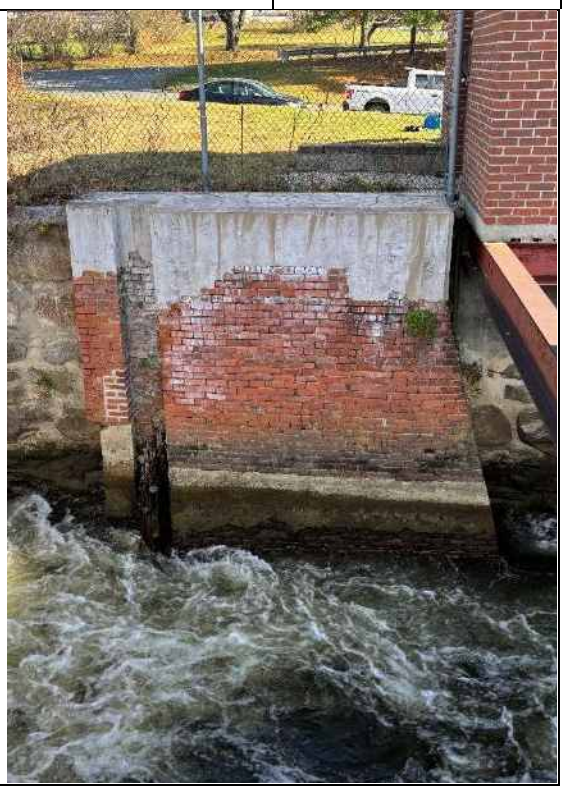


Photo No.	Date:
4	11/20/2025

Direction Photo Taken:
Downstream

Description:
Upstream face of the low-level outlet gate platform concrete structure. Note the minor crack in the concrete face below the abandoned operator (red arrow).

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
5	11/20/2025

Direction Photo Taken:
Upstream

Description:
New gate actuator installed on July 30, 2025.

Photo No.	Date:
6	11/20/2025

Direction Photo Taken:
Upstream

Description:
Downstream face of the low-level outlet gate platform concrete structure. Note minor cracking and efflorescence in the concrete face.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
7	11/20/2025

Direction Photo Taken:
Upstream

Description:
Area above the penstock.

(Inset): Crack in the concrete step to the left of the forebay platform.

(Red Arrow): Diagonal crack in the pumphouse brick wall (see Photo 8).

(Red Circle): Depression in the area above and to the left of the penstock (see Photo 9).

Photo No.	Date:
8	11/20/2025

Direction Photo Taken:
Upstream

Description:
Closeup of the approximately 0.4-inch wide diagonal crack in the lower six courses of the pumphouse downstream brick wall near the right corner.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
9	11/20/2025

Direction Photo Taken:
Downstream

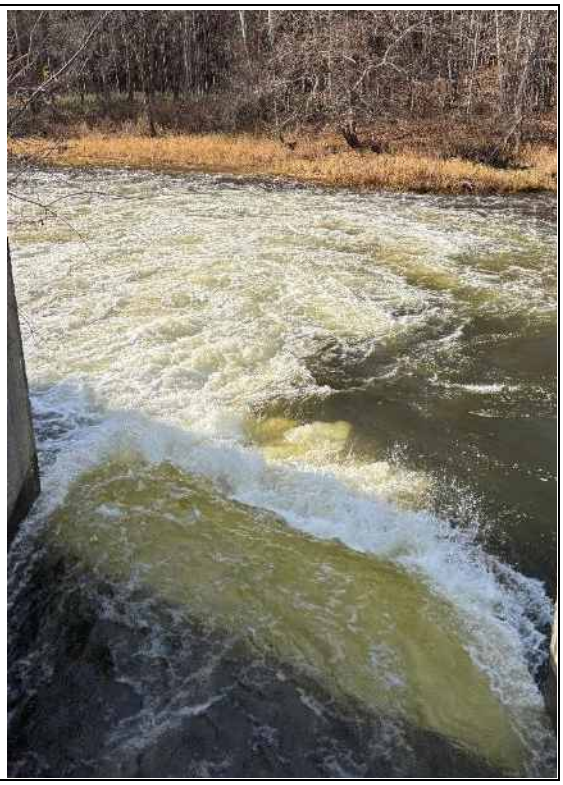
Description:
Closeup of the depression in the area above and to the left of the penstock adjacent to the concrete grade beam structure (former railroad crossing) at about Station 1+30. The depression measured up to two feet deep and one to two feet in plan dimensions.

Photo No.	Date:
10	11/20/2025

Direction Photo Taken:
Left Photo: Left
Right Photo: Right

Description:
(Left Photo): Diversion channel facing the downstream end of the penstock.

(Right Photo): Diversion channel discharge into the Housatonic River downstream of the Dam.



Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
11	11/20/2025

Direction Photo Taken:
Right

Description:
Spillway facing the right embankment.

Note debris in the upstream safety buoys (red arrow).

Photo No.	Date:
12	11/20/2025

Direction Photo Taken:
Downstream

Description:
River channel downstream of the spillway.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
13	11/20/2025

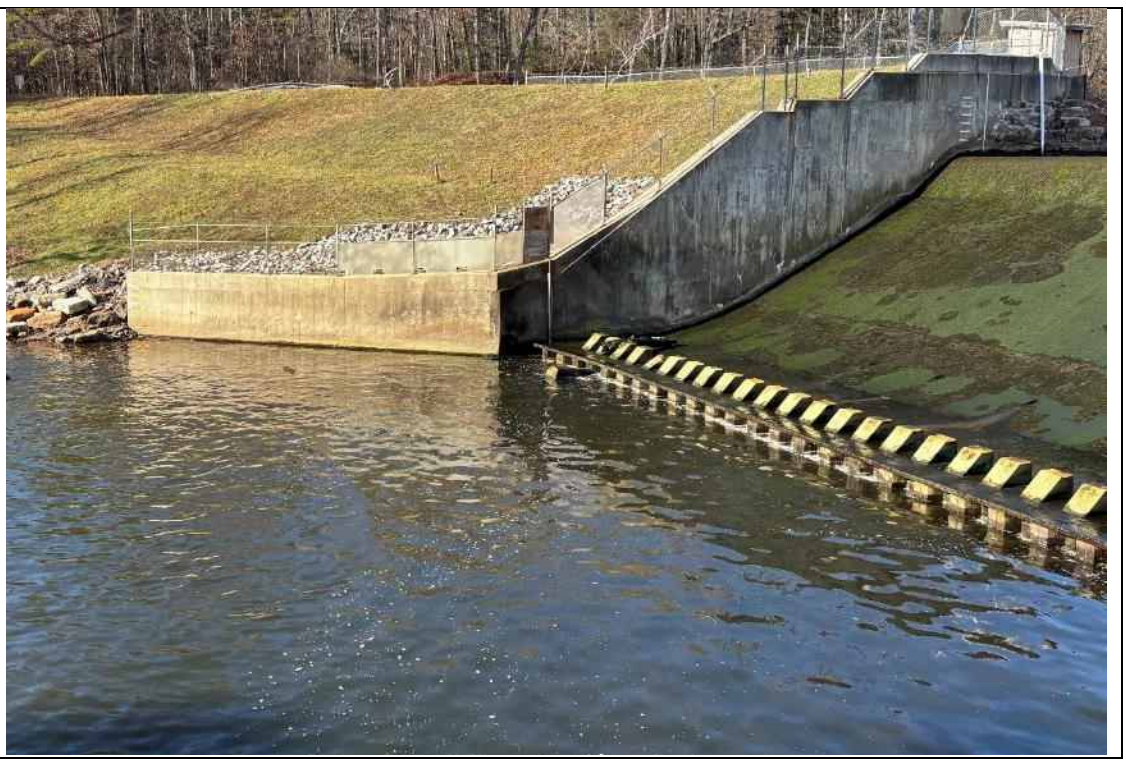
Direction Photo Taken:
Upstream

Description:
Downstream face of the spillway.

Photo No.	Date:
14	11/20/2025

Direction Photo Taken:
Right

Description:
Right downstream spillway training wall.



Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
15	11/20/2025

Direction Photo Taken:
Right

Description:
Right downstream spillway apron.

Note damaged energy dissipator (red circle) and log caught in the energy dissipators (red arrow).

Photo No.	Date:
16	11/20/2025

Direction Photo Taken:
Left

Description:
Closeup of the broken energy dissipator.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
17	11/20/2025

Direction Photo Taken:
Right

Description:
Right downstream spillway training wall.

Note missing fence gate (red circle). Prior to inspection and after inspection, the gate opening was secured with plywood.

Photo No.	Date:
18	11/20/2025

Direction Photo Taken:
Upstream and left

Description:
Downstream face of the spillway.

Note location of horizontal crack/joint in the downstream face of the spillway (red arrow); see Photo 19.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
19	11/20/2025

Direction Photo Taken:
Left

Description:
Closeup of the horizontal crack/joint in the downstream face of the spillway (red arrow). The crack/joint is approximately 4-inch long and located below the second from right weephole at the interface between the sloping ogee concrete and the apron concrete. Minor, clear leakage was observed through the joint, about <½ gpm.

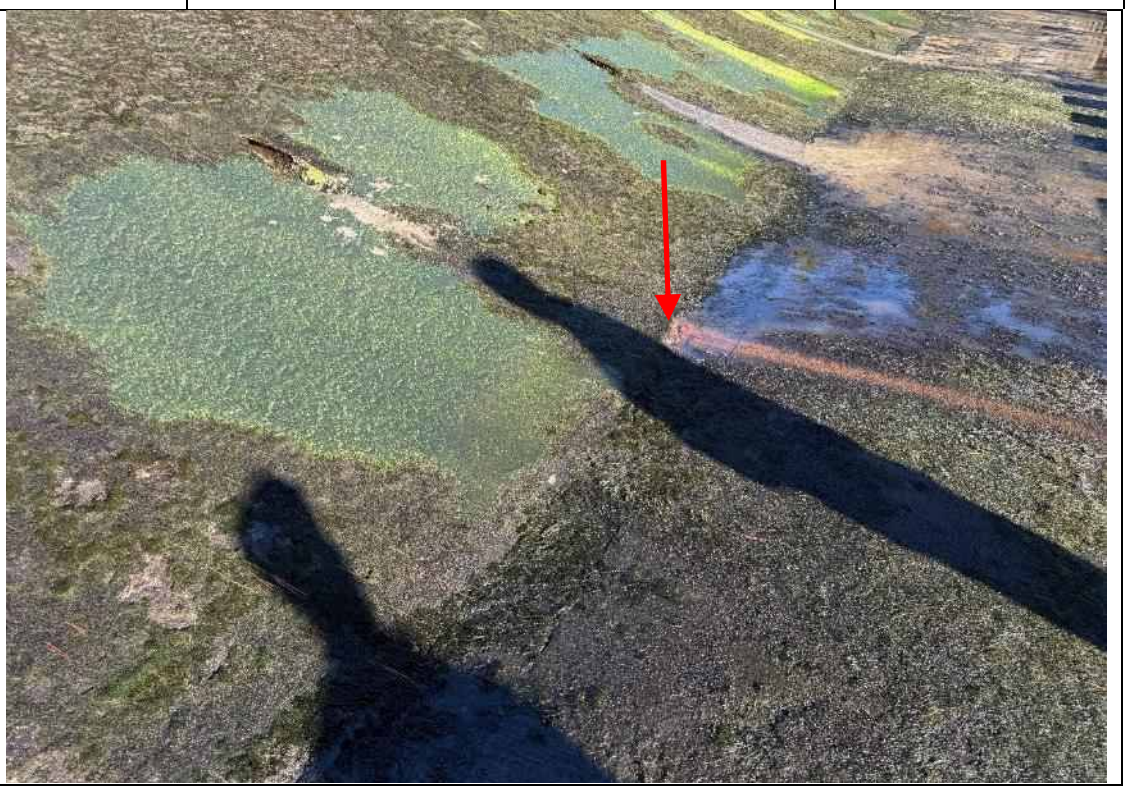


Photo No.	Date:
20	11/20/2025

Direction Photo Taken:
Left

Description:
Spillway downstream apron.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
21	11/20/2025

Direction Photo Taken:
Left

Description:
Minor diffuse leakage was observed emanating from the downstream left masonry training wall with frequency and seepage rate increasing towards the downstream end of the spillway. The leakage was clear with total estimated flow rate less than about 1- to 2-gpm.

Photo No.	Date:
22	11/20/2025

Direction Photo Taken:
Left

Description:
Left abutment and spillway training walls.

The two areas of leakage through the stone masonry left training wall observed during previous site visits were not observed during the dewatered Phase 1 inspection.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
23	11/20/2025

Direction Photo Taken:
Upstream

Description:
Downstream slope of the right embankment.

(Red arrow): minor sloughing of riprap at the end of the right downstream training wall (see Photo 24).

(Red circle): iron staining along the right downstream shoreline.

Photo No.	Date:
24	11/20/2025

Direction Photo Taken:
Downstream/Right

Description:
Toe of the right embankment.
(Red arrow): "Low area" (see Photo 25).
(Red circle): minor sloughing of riprap at the end of the right downstream training wall (see Photo 23).
(White circle): area of standing water.
(White arrow): Tilted piezometer GZ-6 casing.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
25	11/20/2025

Direction Photo Taken:
Upstream

Description:
"Low area" in the downstream riprap near the toe of the right embankment. Its approximate dimensions are ±9 feet (left to right) by ±7½ feet (upstream to downstream) by ±4 inches deep, similar to previous inspections.

Photo No.	Date:
26	11/20/2025

Direction Photo Taken:
Left

Description:
Top of the Right Embankment.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
27	11/20/2025

Direction Photo Taken:
Right

Description:
Upstream slope of the Right Embankment.

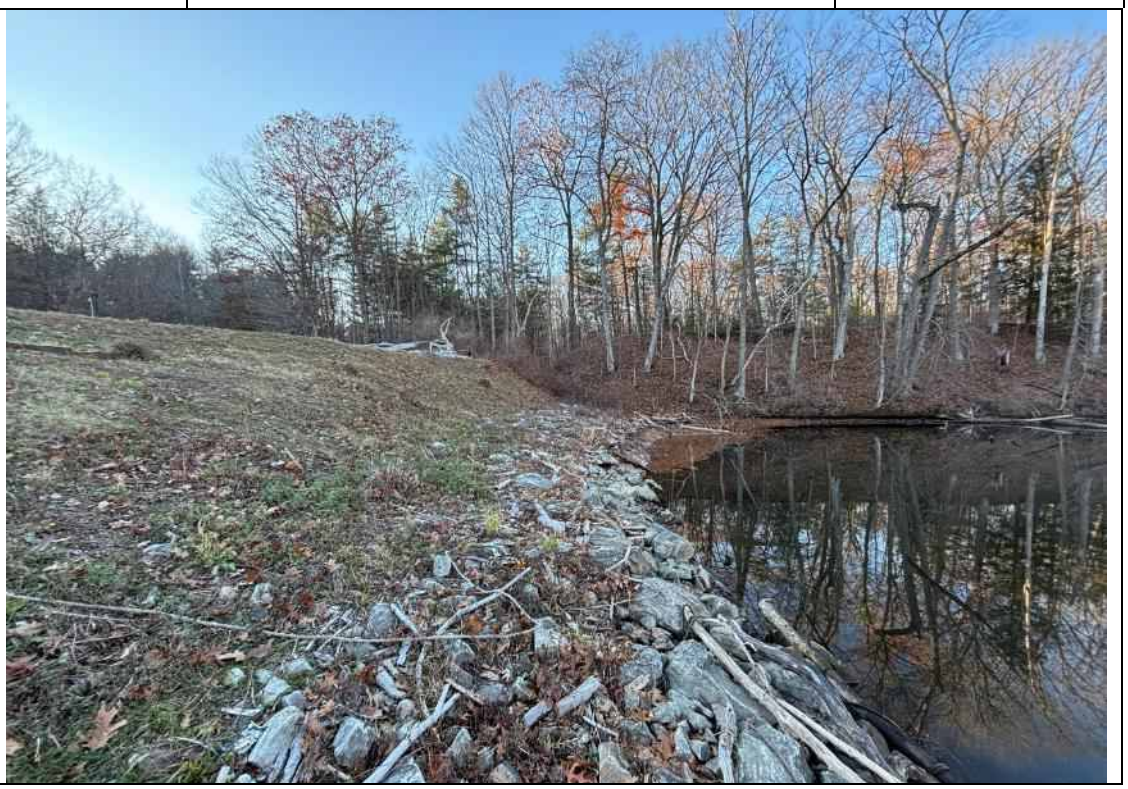


Photo No.	Date:
28	11/20/2025

Direction Photo Taken:
Left

Description:
Upstream slope of the Right Embankment at the sheetpiles.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
29	11/20/2025

Direction Photo Taken:
Left

Description:
Downstream slope of the Right Embankment.

Photo No.	Date:
30	11/20/2025

Direction Photo Taken:
Left.

Description:
Fence along the top of the right spillway training wall.

Note broken gate (plywood blocking temporarily removed to allow access). Plywood re-installed across opening at end of inspection.

Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
31	11/20/2025

Direction Photo Taken:
Downstream

Description:
Riprap protection along the right downstream training wall. Riprap appeared intact.

Photo No.	Date:
-	-

Direction Photo Taken:

Description:

Not Used

APPENDIX C – INSPECTION CHECKLIST

DAM SAFETY INSPECTION CHECKLIST

NAME OF DAM: Rising Pond Dam

STATE ID #: 1-2-113-14

REGISTERED: ☑ YES ☐ NO

NID ID #: MA00250

STATE SIZE CLASSIFICATION: Intermediate

STATE HAZARD CLASSIFICATION: Significant

CHANGE IN HAZARD CLASSIFICATION REQUESTED?: No

DAM LOCATION INFORMATION

CITY/TOWN: Great Barrington

COUNTY: Berkshire

DAM LOCATION: 295 Park St, Housatonic, MA 01236
(street address if known)

ALTERNATE DAM NAME: Rising Paper Company Dam, Rising Dam

USGS QUAD.: Housatonic

LAT.: 42.2424 N

LONG.: 73.3577 W

DRAINAGE BASIN: Housatonic

RIVER: Housatonic River

IMPOUNDMENT NAME(S): Rising Pond

GENERAL DAM INFORMATION

TYPE OF DAM: Earthfill embankment with gravity spillway

OVERALL LENGTH (FT): 670

PURPOSE OF DAM: Impound Rising Pond Reservoir / PCB sediments

NORMAL POOL STORAGE (ACRE-FT): 195 at El. 716.4'

YEAR BUILT: Late 1800s

MAXIMUM POOL STORAGE (ACRE-FT): 710 at El. 726.2'

STRUCTURAL HEIGHT (FT): 38.0

EL. NORMAL POOL (FT): 716.4±

HYDRAULIC HEIGHT (FT): 30

EL. MAXIMUM POOL (FT): 726.2

FOR INTERNAL MADCR USE ONLY

FOLLOW-UP INSPECTION REQUIRED: ☐ YES ☐ NO

CONDITIONAL LETTER: ☐ YES ☐ NO

DAM SAFETY INSPECTION CHECKLIST

NAME OF DAM: Rising Pond Dam

STATE ID #: 1-2-113-14

REGISTERED: ☑ YES ☐ NO

NID ID #: MA00250

STATE SIZE CLASSIFICATION: Intermediate

STATE HAZARD CLASSIFICATION: Significant

CHANGE IN HAZARD CLASSIFICATION REQUESTED?: No

DAM LOCATION INFORMATION

CITY/TOWN: Great Barrington

COUNTY: Berkshire

DAM LOCATION: 295 Park St, Housatonic, MA 01236
(street address if known)

ALTERNATE DAM NAME: Rising Paper Company Dam, Rising Dam

USGS QUAD.: Housatonic

LAT.: 42.2424 N

LONG.: 73.3577 W

DRAINAGE BASIN: Housatonic

RIVER: Housatonic River

IMPOUNDMENT NAME(S): Rising Pond

GENERAL DAM INFORMATION

TYPE OF DAM: Earthfill embankment with gravity spillway

OVERALL LENGTH (FT): 670

PURPOSE OF DAM: Impound Rising Pond Reservoir / PCB sediments

NORMAL POOL STORAGE (ACRE-FT): 195 at El. 716.4'

YEAR BUILT: Late 1800s

MAXIMUM POOL STORAGE (ACRE-FT): 710 at El. 726.2'

STRUCTURAL HEIGHT (FT): 38.0

EL. NORMAL POOL (FT): 716.4±

HYDRAULIC HEIGHT (FT): 30

EL. MAXIMUM POOL (FT): 726.2

FOR INTERNAL MADCR USE ONLY

FOLLOW-UP INSPECTION REQUIRED: ☐ YES ☐ NO

CONDITIONAL LETTER: ☐ YES ☐ NO

NAME OF DAM:	Rising Pond Dam	STATE ID #:	1-2-113-14
INSPECTION DATE:	November 20, 2025	NID ID #:	MA00250

INSPECTION SUMMARY

DATE OF INSPECTION:	November 20, 2025	DATE OF PREVIOUS INSPECTION:	November 15, 2023
TEMPERATURE/WEATHER:	mid-40s °F / Mostly Cloudy	ARMY CORPS PHASE I:	☑ YES ☐ NO If YES, date September 1979
CONSULTANT:	GZA GeoEnvironmental, Inc.	PREVIOUS DCR PHASE I:	☑ YES ☐ NO If YES, date 11/15/2023
BENCHMARK/DATUM:	NGVD29		
OVERALL PHYSICAL CONDITION OF DAM:	SATISFACTORY	DATE OF LAST REHABILITATION:	Early 1990s major modifications
SPILLWAY CAPACITY:	>100% SDF w/ no actions by Caretaker		
EL. POOL DURING INSP.:	715.3±	EL. TAILWATER DURING INSP.:	693.8±

PERSONS PRESENT AT INSPECTION

NAME	TITLE/POSITION	REPRESENTING
Jonathan D. Andrews, P.E.	Principal-in-Charge	GZA GeoEnvironmental, Inc.
Mengxuan Zhao	Assistant Project Manager	GZA GeoEnvironmental, Inc.
Leslie DeCristofaro	Engineer I	GZA GeoEnvironmental, Inc.
Tom Czelusniak	Regulator	HDR, Inc. (representing EPA)
Steve Garrity	Owner's Contractor	LB Corporation

EVALUATION INFORMATION

		Click on box to select E-code
E1)	TYPE OF DESIGN	4
E2)	LEVEL OF MAINTENANCE	5
E3)	EMERGENCY ACTION PLAN	5
E4)	EMBANKMENT SEEPAGE	4
E5)	EMBANKMENT CONDITION	4
E6)	CONCRETE CONDITION	4
E7)	LOW-LEVEL OUTLET CAPACITY	5
E8)	LOW-LEVEL OUTLET CONDITION	4
E9)	SPILLWAY DESIGN FLOOD CAPACITY	5
E10)	OVERALL PHYSICAL CONDITION	4
E11)	ESTIMATED REPAIR COST	Not applicable
	ROADWAY OVER CREST	NO
	BRIDGE NEAR DAM	NO

NAME OF INSPECTING ENGINEER: Jonathan D. Andrews, P.E. SIGNATURE: 

NAME OF DAM:	Rising Pond Dam	STATE ID #:	1-2-113-14
INSPECTION DATE:	November 20, 2025	NID ID #:	MA00250

OWNER:		CARETAKER:	
ORGANIZATION	General Electric Company	ORGANIZATION	General Electric Company
NAME/TITLE	Kevin Mooney, Senior Project Ma	NAME/TITLE	Kevin Mooney, Senior Project Manager
STREET	1 Plastics Avenue	STREET	1 Plastics Avenue
TOWN, STATE, ZIP	Pittsfield, MA 01201	TOWN, STATE, ZIP	Pittsfield, MA 01201
PHONE	413-448-6610	PHONE	413-448-6610
EMERGENCY PH. #	413-441-4619	EMERGENCY PH. #	413-441-4919
FAX	-	FAX	-
EMAIL	kevin.mooney@geaerospace.c	EMAIL	kevin.mooney@geaerospace.com
OWNER TYPE	Private		

PRIMARY SPILLWAY TYPE	Ogee overflow weir		
SPILLWAY LENGTH (FT)	130	SPILLWAY CAPACITY (CFS)	17,093 at El. 726.2' (500-year flood)
AUXILIARY SPILLWAY TYPE	Not applicable	AUX. SPILLWAY CAPACITY (CFS)	Not applicable
NUMBER OF OUTLETS	One	OUTLET(S) CAPACITY (CFS)	±3,300 at El. 726.2'
TYPE OF OUTLETS	14-ft diameter penstock	TOTAL DISCHARGE CAPACITY (CFS)	±20,000
DRAINAGE AREA (SQ MI)	279	SPILLWAY DESIGN FLOOD (PERIOD/CFS)	100-year / 11,700

HAS DAM BEEN BREACHED OR OVERTOPPED ☐ YES ☑ NO IF YES, PROVIDE DATE(S) Not applicable

FISH LADDER (LIST TYPE IF PRESENT) Not applicable

DOES CREST SUPPORT PUBLIC ROAD? ☐ YES ☑ NO IF YES, ROAD NAME: Not applicable

PUBLIC BRIDGE WITHIN 50' OF DAM? ☐ YES ☑ NO IF YES, ROAD/BRIDGE NAME: Not applicable

MHD BRIDGE NO. (IF APPLICABLE) Not applicable

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 NID ID #: MA00250

EMBANKMENT (CREST)

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
CREST	1. SURFACE TYPE	Grassed.	X		
	2. SURFACE CRACKING	None observed.	X		
	3. SINKHOLES, ANIMAL BURROWS	None observed.	X		
	4. VERTICAL ALIGNMENT (DEPRESSIONS)	No unusual movement, depressions, or misalignment observed.	X		
	5. HORIZONTAL ALIGNMENT	Irregular, wide at right abutment.	X		
	6. RUTS AND/OR PUDDLES	None observed.	X		
	7. GRASS COVER CONDITION	Full coverage. Mowed grass currently dormant.	X		
	8. WOODY VEGETATION (TREES/BRUSH)	None observed.	X		
	9. ABUTMENT CONTACT	No signs of crest settlement, movement, etc.	X		

ADDITIONAL COMMENTS:

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 NID ID #: MA00250

EMBANKMENT (D/S SLOPE)

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
D/S SLOPE	1. WET AREAS (NO FLOW)	None observed.	X		
	2. SEEPAGE	None observed.	X		
	3. SLIDE, SLOUGH, SCARP	Minor sloughing of the riprap downstream of the right training wall at the toe…		X	
	4. EMB.-ABUTMENT CONTACT	No signs of settlement, movement, etc.	X		
	5. SINKHOLE/ANIMAL BURROWS	Low area in downstream riprap. Appeared similar to previous inspections.		X	
	6. EROSION	None observed.	X		
	7. UNUSUAL MOVEMENT	None observed.	X		
	8. GRASS COVER CONDITION	Full coverage. Mowed grass (currently dormant) with minor weedy/woody vegetation.	X		
	9. WOODY VEGETATION (TREES/BRUSH)	None observed.	X		
	10. EVIDENCE OF SEEPAGE ALONG PENSTOCK	None observed.	X		
	11. TOE AND 30' BEYOND (SEEP, BOIL, ETC)	Standing water downstream of the right embankment toe.	X		
	12. CONTACT WITH CONRETE FOREBAY	Good condition; no settlement, erosion, seepage, or other unusual conditions noted.	X		

ADDITIONAL COMMENTS: 3. of the right embankment, and iron staining along the shoreline downstream of the right embankment was observed. These conditions were similar to previous quarterly inspections.

11. Standing water has been observed to approach the toe of the right embankment during periods of high tailwater. This water does not appear to be a result of seepage through the embankment.

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 NID ID #: MA00250

EMBANKMENT (U/S SLOPE)

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
U/S SLOPE	1. SLIDE, SLOUGH, SCARP	None observed.	X		
	2. SLOPE PROTECTION TYPE AND COND.	Riprap/sheeting/soil slope on right side; sheetpile and soil slope on left.	X		
	3. SINKHOLE/ANIMAL BURROWS	None observed.	X		
	4. EMB.-ABUTMENT CONTACT	Gully at upstream right contact, possible man-made artifact from original construction.	X		
	5. EROSION	None observed.	X		
	6. UNUSUAL MOVEMENT	No observed unusual movement or misalignment observed.	X		
	7. GRASS COVER CONDITION	Full coverage. Dormant mowed grass with minor weeds.	X		
	8. WOODY VEGETATION (TREES/BRUSH)	Minor overgrown vegetation on the upstream slope of the right and left embankments.	X		
	9. CONTACT WITH FOREBAY WALLS	Good condition; no settlement, erosion, seepage, or other unusual conditions noted.	X		

ADDITIONAL COMMENTS: 4. Gully does not appear to have the potential to hydraulically connect across the Dam during the SDF.

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 NID ID #: MA00250

INSTRUMENTATION

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
INSTR.	1. PIEZOMETERS	VWPs are read at the right embankment instrumentation shed and at instrument locations.		X	
	2. OBSERVATION WELLS	OWs installed in the right and left embankments.		X	
	3. STAFF GAGE AND RECORDER	Since the last Phase 1 inspection in 2023, two new staff gages have been installed: …	X		
	4. WEIRS	None.	X		
	5. INCLINOMETERS	None.	X		
	6. SURVEY MONUMENTS	Metal plate with an "X" on the upstream end of the right training wall.	X		
	7. DRAINS	None.	X		
	8. FREQUENCY OF READINGS	Piezometers and observation wells are read quarterly.		X	
	9. LOCATION OF READINGS	Taken by GZA during quarterly and Phase I visual inspections.		X	

ADDITIONAL COMMENTS: Note: See Appendix G for instrumentation data and plots.
2. Minor buildup of sediment/silt was measured in monitoring wells GZ-2, GZ-5, and GZ-7. However, the current sediment/silt lev
is not impacting the water level readings at these monitoring wells.
3. (1) on the right upstream spillway training wall, and (2) on the left side of the forebay. The third staff gate on the right
downstream spillway training wall was repainted since the last Phase 1 inspection.

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 NID ID #: MA00250

DOWNSTREAM AREA

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
D/S AREA	1. ABUTMENT LEAKAGE	None observed.	X		
	2. FOUNDATION SEEPAGE	None observed.	X		
	3. SLIDE, SLOUGH, SCARP	None observed.	X		
	4. WEIRS	None.	X		
	5. DRAINAGE SYSTEM	None.	X		
	6. INSTRUMENTATION	USGS gage #01197500 on Division St. Bridge ±1 mile downstream.	X		
	7. VEGETATION WITHIN 15 FT	None. Wooded area further downstream.	X		
	8. ACCESSIBILITY	Access to left side off Route 183 (Park Street); Access to right side off Van Deusenville Road	X		
	9. DOWNSTREAM HAZARD DESCRIPTION	Homes, businesses, and secondary roads.	X		
	10. DATE OF LAST EAP UPDATE	12/22/2025	X		

ADDITIONAL COMMENTS: See "Downstream Slope".

NAME OF DAM: Rising Pond Dam | STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 | NID ID #: MA00250

MISCELLANEOUS

AREA INSPECTED	CONDITION	OBSERVATIONS
MISC.	1. RESERVOIR DEPTH (AVG)	Approximately 4-to-5-feet at normal pool.
	2. RESERVOIR SHORELINE	Wooded, grass, moderate slopes.
	3. RESERVOIR SLOPES	About 6-feet-high; wooded above, natural soil below.
	4. ACCESS ROADS	Left side - Rte. 183 at Hazen Paper Mill (295 Park Street); Right side - locked railroad gate off Van Deusenville Road across from AmeriGas facility (69 Van Deusenville Road).
	5. SECURITY DEVICES	Left side access through locked chain link fence. Right side locked railway gate/chainlink fence.
	6. WATER PUBLIC HAZARDS & PROTECTION	Buoys installed upstream of the dam.
	7. LAND-SIDE PUBLIC HAZARDS & PROTECTION	Warning signs installed.
	8. VANDALISM OR TRESPASS	☐ YES ☑ NO WHAT: n/a
	9. AVAILABILITY OF PLANS	☑ YES ☐ NO DATE: 1991-2012 rehab (GZA)
	10. AVAILABILITY OF DESIGN CALCS	☑ YES ☐ NO DATE: 1991-2012 rehab (GZA)
	11. AVAILABILITY OF EAP/LAST UPDATE	☑ YES ☐ NO DATE: 12/22/2025
	12. AVAILABILITY OF O&M MANUAL	☑ YES ☐ NO DATE: 12/22/2025
	13. CARETAKER/OWNER AVAILABLE	☑ YES ☐ NO DATE: 11/20/2025
	14. CONFINED SPACE ENTRY REQUIRED	☐ YES ☑ NO PURPOSE: Not applicable

ADDITIONAL COMMENTS: 4. Right side - railroad gate off Van Deusenville Road across the street from AmeriGas facility (69 Van Deusenville Road).
5. Vehicular access to right side is limited by locked railway gate and locked chainlink fence. Pedestrian access possible.

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 NID ID #: MA00250

PRIMARY SPILLWAY

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
SPILLWAY	1. SPILLWAY TYPE	Rock-filled timber crib with concrete facing, sheetpile walls below u/s & d/s concrete	X		
	2. WEIR TYPE	Ogee-shaped.	X		
	3. SPILLWAY CONDITION	Overall adequate. A minor leak was observed emanating from an approximately 4-inch long.		X	
	4. TRAINING WALLS	Concrete right side. Mortared stone masonry left side.		X	X
	5. SPILLWAY CONTROLS AND CONDITION	No controls - overflow weir.	X		
	6. UNUSUAL MOVEMENT	None observed.	X		
	7. APPROACH AREA	Housatonic River; former railroad pier upstream; clear; buoys installed upstream of the dam.	X		
	8. DISCHARGE AREA	Housatonic River, clear.	X		
	9. DEBRIS	Floating timber debris was observed upstream of the safety buoys. A log was caught…		X	X
	10. WEEPHOLES	Weepholes cleared by LB using metal rod. Sounding of each weephole indicated no…		X	
	11. ACCESS/FENCING	Missing gate in right training wall fence. Temporarily secured with plywood.			X

ADDITIONAL COMMENTS: NOTE: Pool temporarily lowered below spillway crest to allow dewatered inspection.

3. crack or joint located below the second from right weephole at the interface between the sloping ogee concrete and the apron concrete. The crack or joint was too narrow to be probed with a folding ruler. Leakage flow was clear with an estimated flow rate less than about ½ gallon per minute (gpm). The fifth energy dissipator from the right training wall was damaged with a portion of the upper half missing and with exposed steel reinforcing.

4. Minor diffuse leakage were observed emanating from the downstream left masonry training wall with frequency and seepage rate increasing towards the downstream end of the spillway. The leakage was clear with total estimated flow rate less than about 1- to 2-gpm.

9. in the energy dissipators on the right downstream spillway apron.

10. material or debris buildup and upstream end felt firm.

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

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AUXILIARY SPILLWAY (N/A)

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
SPILLWAY	1. SPILLWAY TYPE				
	2. WEIR TYPE				
	3. SPILLWAY CONDITION				
	4. TRAINING WALLS				
	5. SPILLWAY CONTROLS AND CONDITION				
	6. UNUSUAL MOVEMENT				
	7. APPROACH AREA				
	8. DISCHARGE AREA				
	9. DEBRIS				

N/A

ADDITIONAL COMMENTS:

NAME OF DAM: Rising Pond Dam | STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 | NID ID #: MA00250

OUTLET WORKS

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
OUTLET WORKS	1. TYPE	Penstock and tailrace outlet with flip bucket.	X		
	2. INTAKE STRUCTURE	Minor cracking and efflorescence on the upstream and downstream faces of the…		X	
	3. TRASHRACK	Minor surficial rusting.		X	
	4. PRIMARY CLOSURE	Sluice gate replaced in 2008; rehabbed in 2021. Operated by the caretaker prior to…	X		
	5. SECONDARY CLOSURE	None.	X		
	6. CONDUIT	14-ft-diameter, ~220-ft-long buried steel penstock; 90-deg. right turn into a concrete diversion channel which discharges into the Housatonic downstream of the Dam.		X	X
	7. OUTLET STRUCTURE/HEADWALL	Sheetpile wall downstream.	X		
	8. EROSION ALONG TOE OF DAM	None observed.	X		
	9. SEEPAGE/LEAKAGE	Internal visual inspection of the penstock not performed. No indications of significant seepage/leakage observed.	X		
	10. DEBRIS/BLOCKAGE	None observed.	X		
	11. UNUSUAL MOVEMENT	During the November 20, 2025 Phase 1 inspection, GZA's inspection team observed…			X
	12. DOWNSTREAM AREA	Housatonic River.	X		
	13. PENSTOCK BOUNDARY AREA	Fence and diversion channel crossing appear to be in good condition.	X		
	14. MISCELLANEOUS	A crack was observed in the concrete step to the left of the forebay platform; similar to...		X	X
	15. BLOW-OFF/DRAIN VALVE AREA	Inoperable. No leakage observed at outlet to spillway.	X		

ADDITIONAL COMMENTS:

2. low-level outlet gate platform concrete structure were observed, along with a crack on the upstream face (downstream forebay wall) below the abandoned operator. These conditions appear similar to those observed in previous inspections.

4. and during the inspection (dewatered inspection). A new gate actuator was installed on July 30, 2025. The previously-reported grease leaks were not observed. The new actuator digital readout indicated the gate was 21 percent open (about 30-inches) during the inspection.

6. A low area in the penstock was observed during a visual inspection conducted by GZA in October 2021, indicating a possible settled area or "belly". As a result, penstock investigations began in 2022 / 2023 to determine the potential cause of the possible settled area and to determine whether there is active movement or settlement of the penstock. See Phase 1 report for more detail.

9. No turbid discharge observed at discharge channel.

11. a depression in the area above and to the left of the penstock adjacent to the concrete grade beam structure (former railroad crossing) at about Station 1+30. The depression was located about 4½ feet left of the penstock springline (sidewall). The depression measured up to two feet deep and one to two feet in plan dimensions.

14. previous inspections. An approximately 0.4-inch wide diagonal crack was observed in the lower six courses of the pumphouse downstream brick wall near the right corner.

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 NID ID #: MA00250

CONCRETE/MASONRY DAMS (CREST)

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
CREST	TYPE	N/A			
	SURFACE CONDITIONS				
	CONDITIONS OF JOINTS				
	UNUSUAL MOVEMENT				
	HORIZONTAL ALIGNMENT				
	VERTICAL ALIGNMENT				

ADDITIONAL COMMENTS:

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 NID ID #: MA00250

CONCRETE/MASONRY DAMS (DOWNSTREAM FACE)

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
D/S FACE	TYPE	N/A			
	SURFACE CONDITIONS				
	CONDITIONS OF JOINTS				
	UNUSUAL MOVEMENT				
	ABUTMENT CONTACT				
	LEAKAGE				

ADDITIONAL COMMENTS:

NAME OF DAM: Rising Pond Dam STATE ID #: 1-2-113-14

INSPECTION DATE: November 20, 2025 NID ID #: MA00250

CONCRETE/MASONRY DAMS (UPSTREAM FACE)

AREA INSPECTED	CONDITION	OBSERVATIONS	NO ACTION	MONITOR	REPAIR
U/S FACE	TYPE				
	SURFACE CONDITIONS				
	CONDITIONS OF JOINTS				
	UNUSUAL MOVEMENT				
	ABUTMENT CONTACTS				

N/A

ADDITIONAL COMMENTS:

APPENDIX D – PREVIOUS REPORTS & REFERENCES

PREVIOUS REPORTS AND REFERENCES

The following is a list of reports that were located during the file review or were referenced in previous reports.

1. 2025 Penstock Investigations End-of-Year Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, January 9, 2026.
2. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, August 12, 2025.
3. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, May 20, 2025.
4. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, March 13, 2025.
5. 2024 Penstock Investigations End-of-Year Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, December 19, 2025.
6. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, December 18, 2024.
7. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, September 27, 2024.
8. Rising Pond Dam Phase 1 Inspection/Evaluation Report (2023) prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, February 9, 2024, revised July 23, 2024.
9. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, June 20, 2024.
10. 2023 Penstock Investigations End-of-Year Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, May 24, 2024.
11. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, March 27, 2024.
12. Rising Pond Dam Post-Storm Inspection Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, December 21, 2023.
13. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, August 17, 2023.
14. 2023 Penstock Investigations Mid-Year Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, August 16, 2023.
15. Rising Pond Dam Post-Storm Inspection Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, July 17, 2023.
16. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, May 30, 2023.
17. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, February 2, 2023.

18. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, November 21, 2022.
19. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, September 6, 2022.
20. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, June 27, 2022.
21. Rising Pond Dam Quarterly Inspection/Evaluation Report prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, March 4, 2022.
22. Rising Pond Dam Phase 1 Inspection/Evaluation Report (2021) prepared for General Electric Company, Pittsfield, MA by GZA GeoEnvironmental, Inc., Norwood, MA, November 2021, including Summary of Penstock Gate Inspection/Rehabilitation and Left Embankment/Forebay Dam Raising Activities, dated February 16, 2022.
23. Rising Pond Dam Quarterly Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., May 19, 2021.
24. Rising Pond Dam Quarterly Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., March 10, 2021.
25. Rising Pond Dam Quarterly Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., November 10, 2020.
26. Rising Pond Dam Quarterly Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., August 7, 2020.
27. Rising Pond Dam Quarterly Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., May 8, 2020.
28. Rising Pond Dam Quarterly Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., February 12, 2020.
29. Rising Pond Dam Phase 1 Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., November 5, 2019.
30. Operation, Monitoring, and Maintenance Plan – Rising Pond Dam MA 00250, GZA GeoEnvironmental, Inc., August 2019
31. Rising Pond Dam Phase 1 Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., December 19, 2016.
32. Rising Pond Dam Phase 1 Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., November 12, 2014.
33. "Right Embankment Sinkhole Observations and Test Pit Exploration Letter," GZA GeoEnvironmental, Inc., September 16, 2009.
34. Rising Pond Dam Emergency Action Plan, GZA GeoEnvironmental, Inc. 2008.
35. Rising Pond Dam 2007 Structural Integrity Assessment and Inspection/Evaluation Report, Montgomery Watson Harza (MWH), November 15, 2007.
36. "Spillway Sliding Stability Analysis and Review Letter," GZA GeoEnvironmental, Inc., April 13, 2006.
37. Rising Pond Dam Inspection Report, Montgomery Watson Harza (MWH), October 29, 2002.
38. Rising Pond Dam Inspection / Evaluation Report, GZA GeoEnvironmental, Inc., December 1, 2005.
39. Rising Pond Dam Operations and Maintenance Manual, GZA GeoEnvironmental, Inc. 2008.
40. Rising Paper Co. Dam, Phase 1 Inspection Report, Department of the Army - New England Division Corps of Engineers, September 1979.
41. Massachusetts Geographic Information System (MASSGIS), *Geographic Information System database* - http://www.mass.gov/mgis/.

42. Rising Pond Dam Phase 1 Inspection/Evaluation Report, GZA GeoEnvironmental, Inc., November 23, 2009.
43. Phase I Engineering Evaluation and Preliminary Design Report, Rising Pond Dam GZA GeoEnvironmental, Inc., July 13, 2012.

The following references were utilized during the preparation of this report and the development of the recommendations presented herein.

1. Commonwealth of Massachusetts Regulations, 302 CMR 10.00 – Dam Safety, Effective 2/10/17.
2. Federal Emergency Management Agency (FEMA), *Federal Guidelines for Dam Safety: Emergency Action Planning for Dam Owners (FEMA64)*, revised in July 2013, which updates and replaces the earlier April 2004 version.
3. Federal Energy Regulatory Commission's (FERC), *Engineering Guidelines for The Evaluation of Hydropower Projects*, December 2021.
4. U.S. Department of the Interior, Bureau of Reclamation, *Design of Small Dams, Water Resources Technical Publication*, Washington, D.C., 3rd Edition 1987.

APPENDIX E – DEFINITIONS

COMMON DAM SAFETY DEFINITIONS

For a comprehensive list of dam engineering terminology and definitions refer to 302 CMR10.00 Dam Safety, or other reference published by FERC, Dept. of the Interior Bureau of Reclamation, or FEMA. Please note should discrepancies between definitions exist, those definitions included within 302 CMR 10.00 govern for dams located within the Commonwealth of Massachusetts.

Orientation

Upstream – The side of the dam that borders the impoundment.

Downstream – The high side of the dam, the side opposite the upstream side.

Right – The area to the right when looking in the downstream direction.

Left – The left when looking in the downstream direction.

Dam Components

Dam – Any artificial barrier, including appurtenant works, which impounds or diverts water.

Embankment – The fill material, usually earth or rock, placed with sloping sides, such that it forms a permanent barrier that impounds water.

Crest – The top of the dam, usually provides a road or path across the dam.

Abutment – That part of a valley side against which a dam is constructed. An artificial abutment is sometimes constructed as a concrete gravity section, to take the thrust of an arch dam where there is no suitable natural abutment.

Appurtenant Works – Structures, either in dams or separate therefrom, including but not be limited to, spillways; reservoirs and their rims; low-level outlet works; and water conduits including tunnels, pipelines, or penstocks, either through the dams or their abutments.

Spillway – A structure over or through which water flows are discharged. If the flow is controlled by gates or boards, it is a controlled spillway; if the fixed elevation of the spillway crest controls the level of the impoundment, it is an uncontrolled spillway.

Size Classification
(as listed in Commonwealth of Massachusetts, 302 CMR 10.00 Dam Safety)

Large – Structure with a height greater than 40 feet or a storage capacity greater than 1,000 acre-feet.

Intermediate – Structure with a height between 15 and 40 feet or a storage capacity of 50 to 1,000 acre-feet.

Small – Structure with a height between 6 and 15 feet and a storage capacity of 15 to 50 acre-feet.

Non-Jurisdictional – Structure less than 6 feet in height or having a storage capacity of less than 15 acre-feet.

Hazard Classification
(as listed in Commonwealth of Massachusetts, 302 CMR 10.00 *Dam Safety*)

High Hazard (Class I) – Dams located where failure will likely cause loss of life and serious damage to home(s), industrial or commercial facilities, important public utilities, main highway(s) or railroad(s).

Significant Hazard (Class II) – Dams located where failure may cause loss of life and damage to home(s), industrial or commercial facilities, secondary highway(s) or railroad(s), or cause the interruption of the use or service of relatively important facilities.

Low Hazard (Class III) – Dams located where failure may cause minimal property damage to others. Loss of life is not expected.

General

Acre-foot – A unit of volumetric measure that would cover one acre to a depth of one foot. It is equal to 43,560 cubic feet. One million U.S. gallons = 3.068 acre feet.

AHPS – Advanced Hydrologic Prediction Service – a website showing Housatonic River flows and river stage at United States Geological Survey Gage No. 01197500 at Division Street in Great Barrington.

Dam safety engineer – A Professional Engineer experienced in dam safety and registered in Massachusetts.

EAP – Emergency Action Plan – A predetermined (and properly documented) plan of action to be taken to reduce the potential for property damage and/or loss of life in an area affected by an impending dam failure.

Height of dam (structural height) – The vertical distance from the lowest portion of the natural ground, including any stream channel, along the downstream toe of the dam to the lowest point on the crest of the dam.

Hydraulic height – The height to which water rises behind a dam and the difference between the lowest point in the original streambed at the axis of the dam and the maximum controllable water surface.

Maximum storage capacity – The volume of water contained in the impoundment at maximum water storage elevation.

Maximum water storage elevation – The maximum elevation of water surface which can be contained by the dam without overtopping the embankment section.

Normal pool – The elevation of the impoundment during normal operating conditions.

Normal storage capacity – The volume of water contained in the impoundment at normal water storage elevation.

OM&M Plan – Operation, Monitoring, and Maintenance Plan.

Spillway Design Flood (SDF) – The flood used in the design of a dam and its appurtenant works particularly for sizing the spillway and outlet works, and for determining maximum temporary storage and height of dam requirements.

Condition Rating

Unsafe – Major structural*, operational, and maintenance deficiencies exist under normal operating conditions.

Poor – Significant structural*, operation and maintenance deficiencies are clearly recognized for normal loading conditions.

Fair – Significant operational and maintenance deficiencies, no structural deficiencies. Potential deficiencies exist under unusual loading conditions that may realistically occur. Can be used when uncertainties exist as to critical parameters.

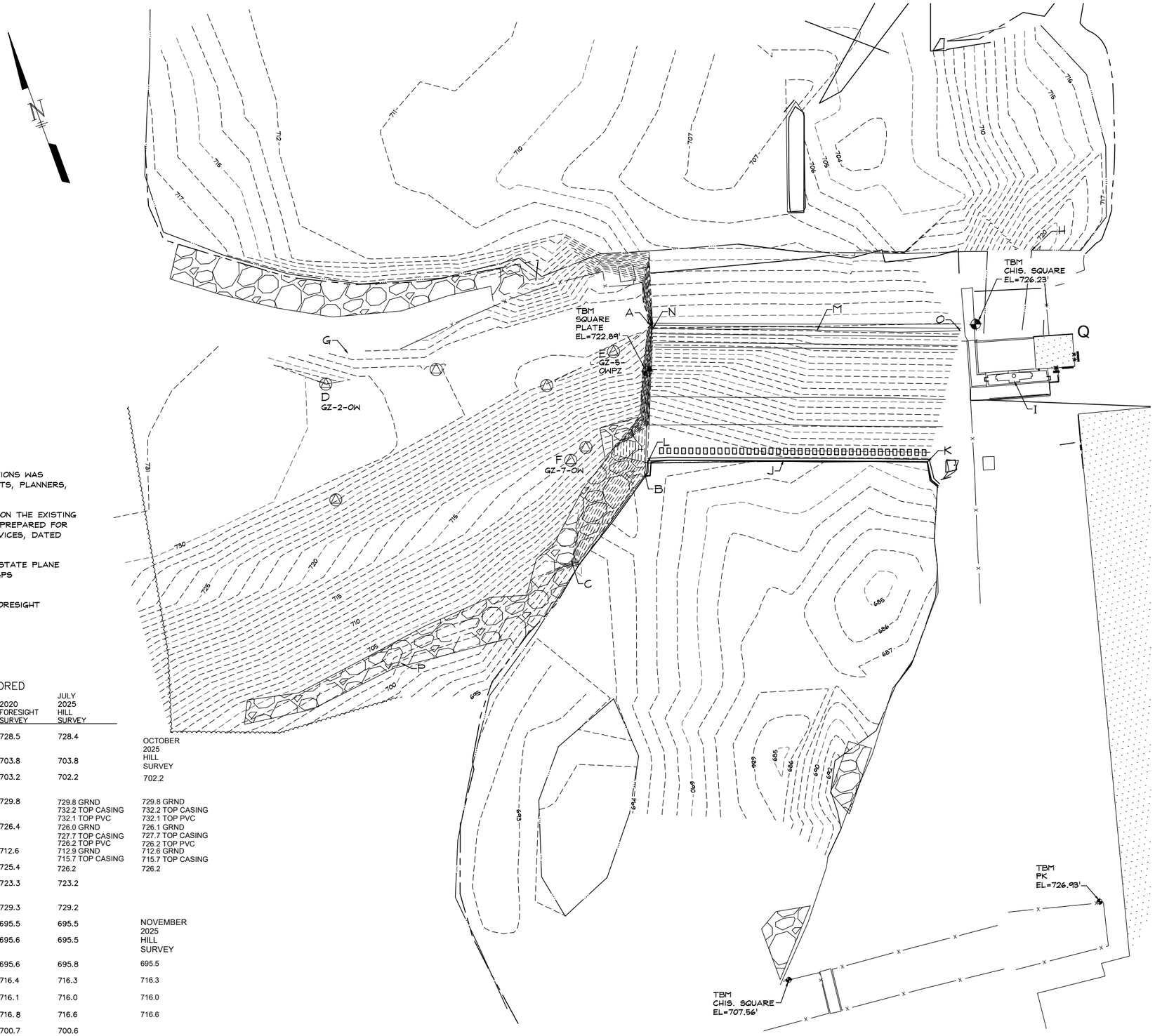
Satisfactory – Minor operational and maintenance issues. Infrequent hydrologic events could result in deficiencies.

Good – No existing or potential deficiencies recognized. Safe performance is expected under all loading including SDF.

* Structural deficiencies include but are not limited to the following:

- Excessive uncontrolled seepage (e.g., upwelling of water, evidence of fines movement, flowing water, erosion, etc.).
- Missing riprap with resulting erosion of slope.
- Sinkholes, particularly behind retaining walls and above outlet pipes, possibly indicating loss of soil due to piping, rather than animal burrows.
- Excessive vegetation and tree growth, particularly if it obscures features of the dam and the dam cannot be fully inspected.
- Deterioration of concrete structures (e.g., exposed rebar, tilted walls, large cracks with or without seepage, excessive spalling, etc.).
- Inoperable outlets (gates and valves that have not been operated for many years or are broken).

APPENDIX F1 – 2025 TOPOGRAPHIC AND BATHYMETRIC SURVEY



LEGEND

- BENCH MARK
- MONITORING WELL
- EDGE OF RIVER
- 1014 / 1015 — EXISTING CONTOUR
- RIP RAP

GENERAL PLAN NOTES:

1. THE BATHYMETRIC SURVEY AND SURVEY OF KEY ELEVATIONS WAS CONDUCTED IN JULY 2025 BY HILL-ENGINEERS, ARCHITECTS, PLANNERS, INC.
2. CONTOURS AND ELEVATIONS SHOWN HEREON ARE BASED ON THE EXISTING BENCHMARKS SHOWN ON A TOPOGRAPHIC SURVEY PLAN, PREPARED FOR GZA GEOENVIRONMENTAL, INC., BY FORESIGHT LAND SERVICES, DATED OCTOBER 7, 2020.
3. THE HORIZONTAL DATUM IS BASED ON MASSACHUSETTS STATE PLANE COORDINATE SYSTEM NAD83, WHICH WAS OBTAINED BY GPS OBSERVATION.
4. GROUND TOPOGRAPHY AND DETAIL ARE PER THE 2020 FORESIGHT SURVEY.

KEY ELEVATIONS TO BE MONITORED

POINT	LOCATION	2020 FORESIGHT SURVEY	JULY 2025 HILL SURVEY	OCTOBER 2025 HILL SURVEY
A	RIGHT SIDE TOP OF SPILLWAY TRAINING WALL AT TOP OF DAM	728.5	728.4	
B	RIGHT SIDE SPILLWAY TRAINING WALL AT ANGLE	703.8	703.8	
C	RIGHT SIDE TOP OF SPILLWAY TRAINING WALL AT BOTTOM OF SLOPE	703.2	702.2	702.2
D	GZ-2-OW TOP OF CASING	729.8	729.8 GRND 732.2 TOP CASING 732.1 TOP PVC	729.8 GRND 732.2 TOP CASING 732.1 TOP PVC
E	GZ-5-OW/PZ TOP OF CASING	726.4	726.0 GRND 727.7 TOP CASING 726.2 TOP PVC	726.1 GRND 727.7 TOP CASING 726.2 TOP PVC
F	GZ-7-OW TOP OF CASING	712.6	712.9 GRND 715.7 TOP CASING	712.6 GRND 715.7 TOP CASING
G	RIGHT END OF RIGHT SIDE SHEETPILE WALL	725.4	726.2	726.2
H	UPSTREAM, RIGHT CORNER OF LEFT SIDE FOREBAY SHEETPILE WALL	723.3	723.2	
I	CENTER GATE MOUNT	729.3	729.2	
J	CENTERLINE DOWNSTREAM END OF CONCRETE SPILLWAY	695.5	695.5	NOVEMBER 2025 HILL SURVEY
K	LEFT SIDE CORNER OF DOWNSTREAM END OF CONCRETE SPILLWAY	695.6	695.5	
L	RIGHT END DOWNSTREAM END OF CONCRETE SPILLWAY	695.6	695.8	695.5
M	CENTERLINE SPILLWAY CREST	716.4	716.3	716.3
* N	RIGHT END SPILLWAY CREST (AT WALL)	716.1	716.0	716.0
* O	LEFT END SPILLWAY CREST (AT WALL)	716.8	716.6	716.6
P	REBAR MONITORING POINT AT LOW AREA	700.7	700.6	
Q	MONITORING WELL D-9		726.9 TOP CASING AT GRND 726.8 TOP PVC 1 726.7 TOP PVC 2	

* THE 2020 FORESIGHT SURVEY RESULTS THAT HAVE BEEN PREVIOUSLY REPORTED TRANSPOSED THE ELEVATIONS FOR POINTS N AND O. THE CORRECT ELEVATIONS ARE NOW SHOWN.

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50 Depot Street
Dalton, MA 01226
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Adams, MA 01220
(413) 743-0013
www.hillengineers.com

PRELIMINARY

NOT TO BE USED FOR CONSTRUCTION

REV.	DESCRIPTION	DR'N	CK'D.	DATE
A.	ISSUED FOR REVIEW AND COMMENT			8/13/25

Supplemental surveys performed in October and November of 2025. Results of all 2025 surveys shown herein, including the table in the lower left corner of this drawing.

GZA GEOENVIRONMENTAL, INC.
249 VANDERBILT AVENUE
NORWOOD, MA 02062

PROJECT DESCRIPTION: RISING POND LENOX, MA

DRAWING TITLE: SITE PLAN

DRAWN BY	JPM
DATE DRAWN	8/13/25
SCALE	1" = 20'
APV'D BY	

CAD CODE: SRV-2625-002-WOODS-CS.DWG

GRAPHIC SCALE: 0 20 40

PROJECT NUMBER: SRV-2625-002

DRAWING NUMBER	REV.
CX101	A

2025 Bathymetric Survey Data - Rising Pond Dam

10200,42.1430,-73.2127,707.56,
10201,42.1430,-73.2127,726.93,PK BY OTHERS
10205,42.1432,-73.2127,726.23,BM CH SQ ON CNC
10206,42.1432,-73.2127,726.28,FFE CHECK 7-9-25
10207,42.1432,-73.2127,726.23,CHECK BM 7-9-25
10208,42.1433,-73.2129,722.89,BM 6''SQ.PLATE+
10400,42.1433,-73.2126,723.23,LOC.PT-H
10401,42.1433,-73.2128,721.54,T-STN.1.5'TO COR
10402,42.1433,-73.2127,723.38,T-STN.PIER
10403,42.1433,-73.2127,723.55,T-STN.PIER
10404,42.1433,-73.2128,722.22,T-STN.1.5'TO COR
10405,42.1433,-73.2129,711.5,CNC.SPILL@BEND
10406,42.1433,-73.2129,699.55,CNC.SPILL CRW
10407,42.1432,-73.2129,695.78,CNC.SPILL CRW
10408,42.1435,-73.2130,712.98,EW 7-9-25
10409,42.1434,-73.2131,713.15,EW 7-9-25
10410,42.1434,-73.2131,713.02,EW 7-9-25
10411,42.1434,-73.2130,712.93,EW 7-9-25
10412,42.1431,-73.2128,692.78,EW ON RUBBLE
10413,42.1431,-73.2128,692.68,EW ON RUBBLE
10414,42.1432,-73.2127,700.95,T-EC+SHEET RW
10415,42.1432,-73.2127,700.75,T-EC+SHEET
10416,42.1432,-73.2127,700.39,T-EC+SHEET
10417,42.1431,-73.2128,700.24,T-EC+SHEET+
10418,42.1431,-73.2128,714.57,T-EC+SHEET RW2
10419,42.1431,-73.2128,714.63,T-EC+SHEET RW2
10420,42.1431,-73.2128,714.93,T-EC+SHEET RW2
10421,42.1430,-73.2129,715.14,T-EC+SHEET RW2
10422,42.1430,-73.2129,711.37,T-EC+SHEET RW2
10423,42.1430,-73.2129,711.21,T-EC+SHEET RW2+
10424,42.1431,-73.2127,714.48,T-EC2@FACE SRW
10425,42.1433,-73.2127,716.94,EW COR.CRW
10426,42.1433,-73.2126,716.84,EW ON RR
10701,42.1429,-73.2130,702.02,IPS1
10703,42.1431,-73.2127,721.66,PTS3
10704,42.1430,-73.2127,726.35,PKF HDWALL
10705,42.1429,-73.2130,702.14,IP
10706,42.1431,-73.2127,721.87,PTS-3
10707,42.1430,-73.2129,706.46,IP
10708,42.1430,-73.2127,726.36,CH SQ
10709,42.1430,-73.2127,726.29,PK NAIL
11000,42.1434,-73.2127,704.36,G-BED
11001,42.1433,-73.2127,705.67,G-BED COR.CRW
11002,42.1433,-73.2127,704.24,G-BED
11003,42.1433,-73.2127,704.41,G-BED
11004,42.1433,-73.2127,706.94,G-BED
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11010,42.1433,-73.2127,709.22,T-EC +-
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11015,42.1433,-73.2128,712.47,OS CNC.
11016,42.1433,-73.2128,709.35,EC FLSH
11017,42.1433,-73.2128,712.53,OS CNC.
11018,42.1433,-73.2128,708.63,EC FLSH
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11122,42.1433,-73.2130,732.22,TOC
11123,42.1433,-73.2130,732.09,T-PVC
11124,42.1433,-73.2130,728.1,MW GZ-2 G
11125,42.1433,-73.2130,730.35,TOC
11126,42.1433,-73.2130,730.29,T-PVC
11127,42.1433,-73.2129,725.77,MW G
11128,42.1433,-73.2129,729.67,TOC
11129,42.1433,-73.2129,729.63,T-PVC 1''
11130,42.1433,-73.2129,729.67,T-PVC 3/4''
11131,42.1432,-73.2129,712.65,MW GZ-6 PZ G
11132,42.1432,-73.2129,715.04,TOC
11133,42.1432,-73.2129,715.01,T-PVC
11134,42.1432,-73.2129,712.89,MW GZ-7 OW G
11135,42.1432,-73.2129,715.69,T-CAP
11136,42.1433,-73.2126,716.61,MAHW EW 7-10-25
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11138,42.1433,-73.2126,716.7,MAHW EW 7-10-25
11139,42.1433,-73.2126,716.7,MAHW EW 7-10-25
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11143,42.1433,-73.2126,723.52,T-COR.RR
11144,42.1432,-73.2126,723.65,T-RR B-SRW
11145,42.1432,-73.2126,723.22,T-RR B-SRW
11146,42.1432,-73.2126,724.21,T-RR B-SRW
11147,42.1432,-73.2126,725.88,T-SRW COR
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11161,42.1432,-73.2126,722.12,BUSH 10D
11162,42.1432,-73.2126,722.61,GL 1'SQ+B
11163,42.1432,-73.2126,722.39,CL.SIGN 2.5'W+B
11164,42.1432,-73.2126,722.18,CL.SIGN 2.5'W
11165,42.1432,-73.2126,722.46,G
11166,42.1432,-73.2126,722.54,G
11167,42.1432,-73.2126,722.8,EC 3'W DOOR
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11171,42.1432,-73.2126,722.16,B
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11176,42.1432,-73.2126,725.18,OS
11177,42.1432,-73.2126,726.7,T2
11178,42.1432,-73.2126,726.75,T2
11179,42.1432,-73.2126,726.21,T2+BD
11180,42.1432,-73.2126,726.68,BD
11181,42.1432,-73.2126,726.88,TOC 1'D FLSH
11182,42.1432,-73.2126,726.8,T-PVC D-9-DEEP
11183,42.1432,-73.2126,726.66,T-PVC D-9SHALLOW
11184,42.1432,-73.2126,726.34,B-CRW
11185,42.1432,-73.2126,726.24,B-CRW
11186,42.1432,-73.2126,726.18,B-CRW EG
11187,42.1432,-73.2126,726.13,B-CRW EG
11188,42.1432,-73.2127,725.99,EG CLF END
11189,42.1432,-73.2126,725.9,CLF
11190,42.1432,-73.2126,725.9,CLF END
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11195,42.1432,-73.2127,726.05,EG EC
11196,42.1432,-73.2127,725.99,EG EC
11197,42.1432,-73.2127,729.18,LOC.PT-I T-MET.
11198,42.1432,-73.2127,728.94,CNC.ELEV
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11306,42.1432,-73.2127,716.63,LOC.PT-O
11307,42.1433,-73.2129,725.96,MW+G
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11309,42.1433,-73.2129,726.18,T PVC
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12001,42.1433,-73.2130,729.8,MW GZ-1-OW G
12002,42.1433,-73.2130,732.21,TOC
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12010,42.1432,-73.2129,712.88,MW GZ-7-OW/PZ G
12011,42.1432,-73.2129,715.6,TOC
12012,42.1432,-73.2129,715.4,T-PVC
12013,42.1432,-73.2129,702.2,T-CL.CRW END
13000,42.1432,-73.2127,716.64,LOC.PT-O AGAIN
13001,42.1433,-73.2128,716.32,LOC.PT-M AGAIN
13002,42.1433,-73.2129,716.03,0.0' STAFF GAUGE
13003,42.1433,-73.2129,716.03,LOC.PT-N AGAIN
13004,42.1432,-73.2129,695.54,LOC.PT-L AGAIN

APPENDIX F2 – GATE ACTUATOR MANUFACTURER INSTALLATION STATEMENT

Rotork Site Services Statement of Work

Job Details

Project Number :	FM4299	**Customer :**	GZA GEOENVIROMENTAL INC
Site Name :	GZA GEOENVIROMENTAL INC	**Customer Ref :**	37892 / CUS152947-00-1
Site Address :	295 PARK ST HOUSATONIC 01236 United States of America	**Contact Name :**	Seth Krause
		Contact Phone :	717.579.2500
		Contact Email :	seth.krause@gza.com

Job Summary

Description: SERIAL NO.: B393870101
ACTUATOR DESCRIPTION: IQ91 FA25B4 WT 575-3-60 WD18796-01
The baseplate of the existing Rotork actuator became cracked and began leaking fluid. The actuator will be replaced (separate contractor) with a new Electric actuator IQ91 FA25 B4 [IP68] 575/3/60 230 RPM. Rotork will verify the actuator was installed properly, set limits, and verify the operability of the gate using the new actuator.
Site Contact: Kevin Mooney (413-441-4619)

Summary: -Commission one actuator.

Future Considerations:

Asset Summary

Serial Number	Tag Number	Model	Incident Description	Status
B393870101		IQ3 PRO	-Commission/start up one actuator on gate valve.	Completed

Actions Taken on Assets

Serial Number	Action Taken
B393870101	-Commission/Start up actuator. -Inspected wiring good. -Found "Contactor Release Relay" (RR Relay) wired but not energized. -Installed wire jumpers on Terminal Bung between terminals 4&5 to 45&46 to enable actuator to operate open/close. -Setup actuator open/close limits. Gate was authorized to open up to four feet of fourteen feet total. The full close limit was set with gate on full close position (zero turn). The full open limit was calculated from about 968 Centre Column turns to raise stem four feet, this value (968) was multiplied by three to allow actuator move full open up to twelve feet (total 2,900 turns). This gate stem raises up to fourteen feet however was used twelve feet as safety measure. -Open/close torque adjustment was left in 40%. Valve torque demand was less than 15% during all operation. -Performed operational tests open/close up to four feet satisfactorily. Actuator LCD display shows about 33.3% open when gate stem at four feet from full close. -Downloaded data log to file. -Job complete.

Parts Used

Serial Number	Part Number	Part Description	Quantity
---	---	---	0.0

Hours Worked and Travel

Resource	Start Date/Time	End Date/Time	Hours Worked	Hours Travelled	Distance Travelled	Booking Status	Overnight Stay
Renato Nelli	10/26/25 2:30 PM	10/27/25 8:00 PM	5.00	12.50		Completed	Yes

Customer Signature: …… Signed By: Seth Krause

Rotork Site Services

US Service 833-970-8675 **US Service** US.service@rotork.com

675 Mile Crossing Blvd., Rochester, NY 14624

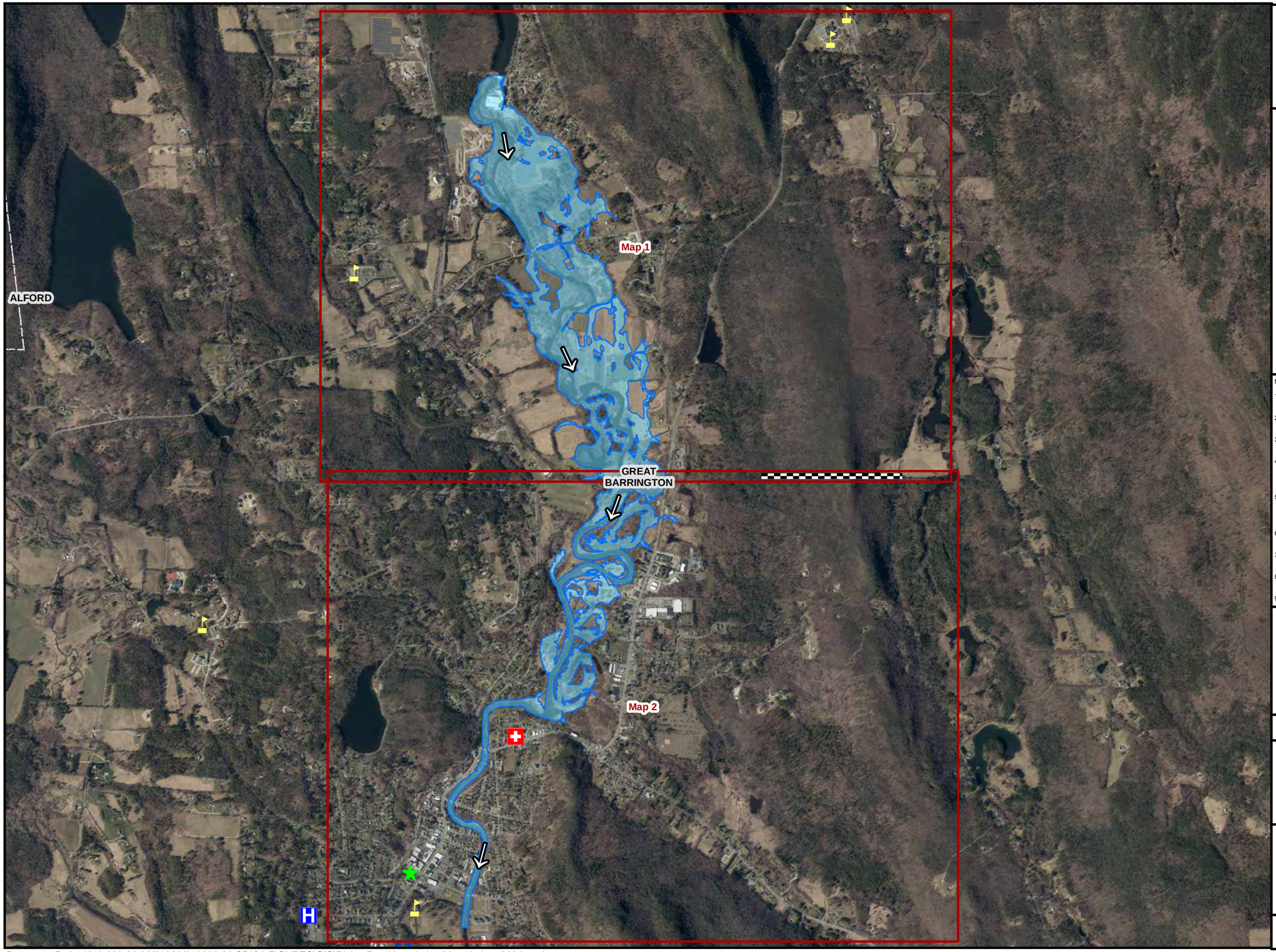
5607 W. Douglas Ave, Milwaukee, WI, 53218

1811 Brittmoore Rd, STE 100, Houston, TX 77043

4735 N Mingo Rd, Tulsa, OK 74117

Please tell us how we are doing.

APPENDIX F3 – INUNDATION MAPS (BASED ON GZA'S 2024 DAM BREAK ANALYSIS)



NORTH

Page Size: 11 by 17 inches
1 inch = 2,000 feet

0 1,000 2,000 4,000 Feet

LEGEND

- Flow Direction
- Matchlines
- Town Halls
- Fire Stations
- Hospitals
- Police Stations
- Schools
- Municipal Boundaries
- Inundation Area for Dam Failure (Significant Hazard Breach)

NOTES:

1. THE INUNDATION AREA SHOWN IS APPROXIMATE AND SHOULD BE USED AS A GUIDELINE FOR ESTABLISHING EVACUATION ZONES.
2. ACTUAL INUNDATION AREA WILL DEPEND ON ACTUAL FAILURE CONDITIONS AND MAY DIFFER FROM THIS MAP.
3. INUNDATION AREA WAS CALCULATED BY SIMULATING DAM FAILURE WITH THE HEC-RAS 6.5 COMPUTER SOFTWARE IN MAY 2024.
4. INUNDATION AREA REFLECTS FAILURE OF THE DAM'S EMBANKMENT, OCCURRING WHEN THE RESERVOIR IS AT THE TOP OF EMBANKMENT (727.0 FT, NAVD88) AND THE DOWNSTREAM RIVERS HAVE NORMAL BASEFLOWS REPRESENTED IN THE LIDAR.
5. THE INUNDATION AREAS SHOWN ON THIS MAP REFLECT EVENTS OF AN EXTREMELY REMOTE NATURE. THESE RESULTS ARE NOT IN ANY WAY INTENDED TO REFLECT UPON THE INTEGRITY OF RISING POND DAM.
6. ELEVATION REFERENCE DATUM USED IN THE MODEL WAS THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
7. AERIAL PHOTO OBTAINED FROM MASSGIS (DATE OF IMAGERY IS SPRING 2019).
8. SCHOOLS LOCATIONS FROM MA DEP (JULY 2020). SCHOOLS INCLUDE PRE-K THROUGH HIGH SCHOOL
9. HOSPITALS LOCATIONS FROM THE DPH, OEMS (DEC. 2018)

INUNDATION LIMITS FOR RISING POND DAM (MA00250)

GREAT BARRINGTON, MASSACHUSETTS

INDEX MAP

Prepared For:

GENERAL ELECTRIC COMPANY

1 Plastics Ave.
Pittsfield, MA 01201

Prepared By:

GZA GeoEnvironmental, Inc.
249 Vanderbilt Avenue
Norwood, MA 02062
Phone: (781) 278-3700 Fax: (781) 278-5701

Proj. Mgr.:	JDA	Dwg. Date:	11/26/2024
Designed By:	RSG		
Reviewed By:	SK	Job No.:	01.0019896.80
Operator:	RSG		

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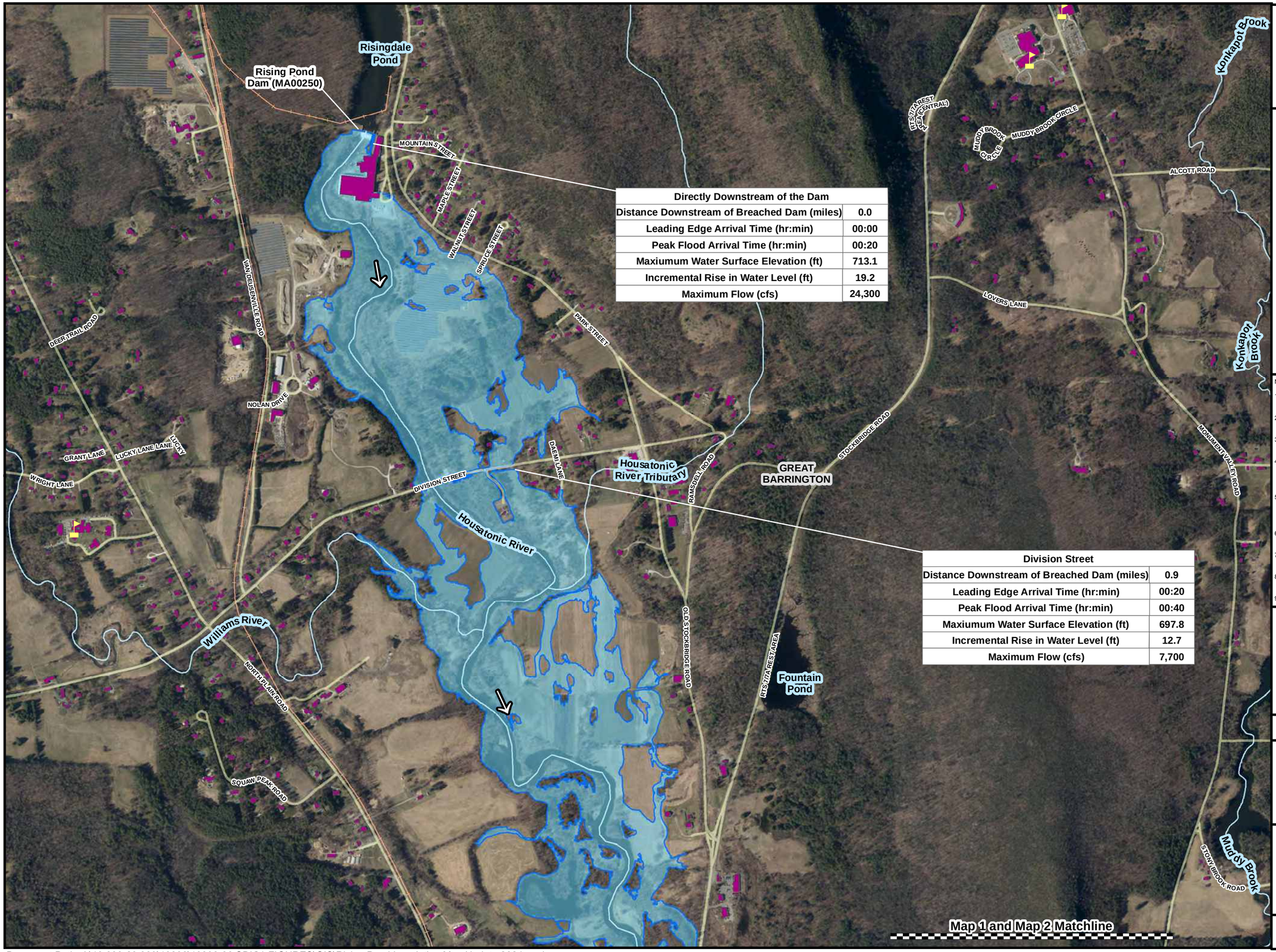

Directly Downstream of the Dam	
Distance Downstream of Breached Dam (miles)	0.0
Leading Edge Arrival Time (hr:min)	00:00
Peak Flood Arrival Time (hr:min)	00:20
Maxiumum Water Surface Elevation (ft)	713.1
Incremental Rise in Water Level (ft)	19.2
Maximum Flow (cfs)	24,300

Division Street	
Distance Downstream of Breached Dam (miles)	0.9
Leading Edge Arrival Time (hr:min)	00:20
Peak Flood Arrival Time (hr:min)	00:40
Maxiumum Water Surface Elevation (ft)	697.8
Incremental Rise in Water Level (ft)	12.7
Maximum Flow (cfs)	7,700

NORTH

Page Size: 11 by 17 inches
1 inch = 1,000 feet

0 500 1,000 2,000 Feet

LEGEND

- Flow Direction
- Matchlines
- Town Halls
- Fire Stations
- Hospitals
- Police Stations
- Schools
- Roadways
- Rivers & Streams
- Municipal Boundaries
- Buildings
- Railroads
- Inundation Area for Dam Failure (Significant Hazard Breach)

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8. SCHOOLS LOCATIONS FROM MA DEP (JULY 2020). SCHOOLS INCLUDE PRE-K THROUGH HIGH SCHOOL
9. HOSPITALS LOCATIONS FROM THE DPH, OEMS (DEC. 2018)

INUNDATION LIMITS FOR RISING POND DAM (MA00250)

GREAT BARRINGTON, MASSACHUSETTS

INUNDATION MAP 1 OF 2

Prepared For:

GENERAL ELECTRIC COMPANY

1 Plastics Ave.
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Prepared By:

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Proj. Mgr.:	JDA	Dwg. Date:	11/26/2024
Designed By:	RSG		
Reviewed By:	SK	Job No.:	01.0019896.80
Operator:	RSG		

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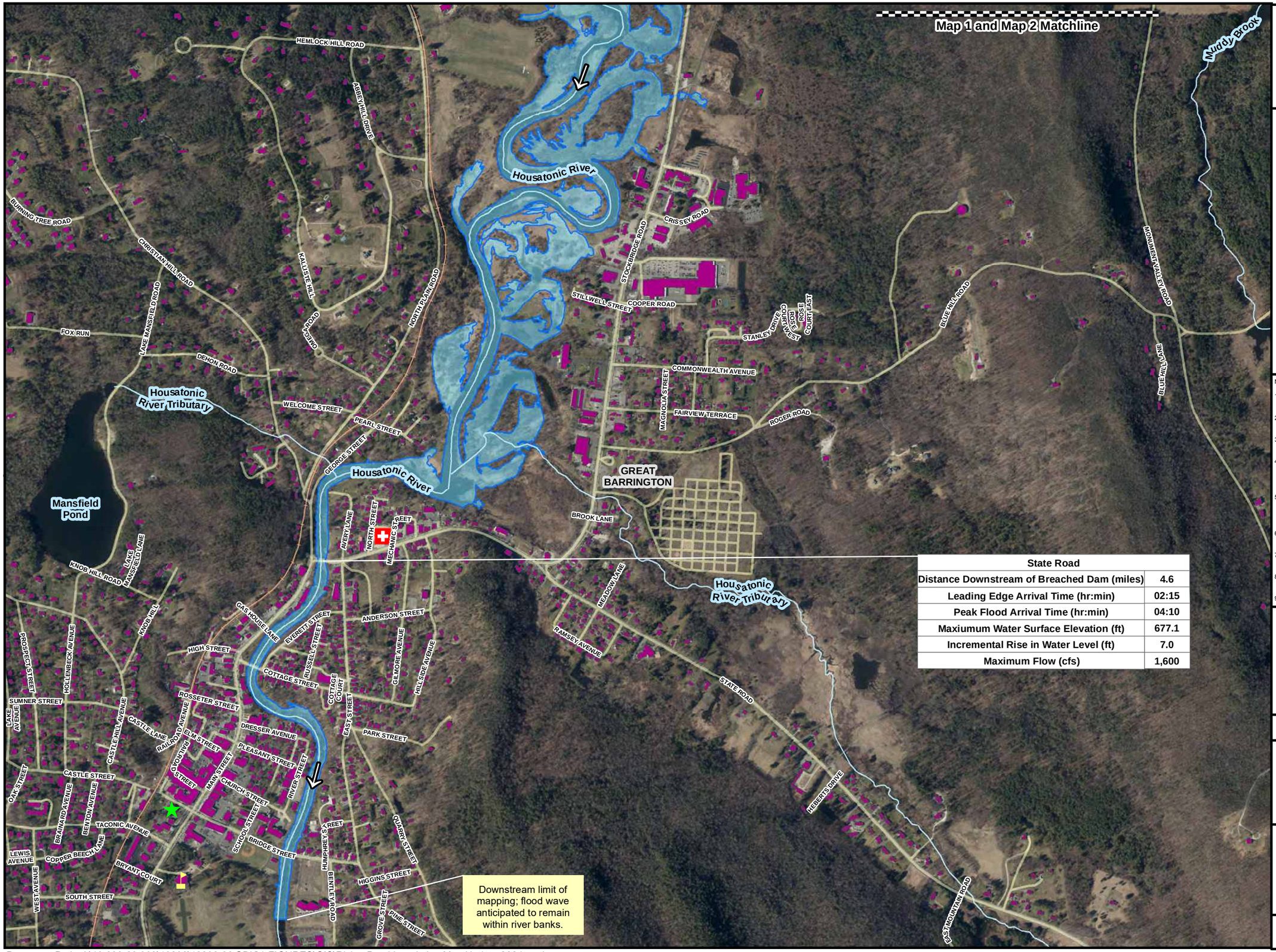

State Road	
Distance Downstream of Breached Dam (miles)	4.6
Leading Edge Arrival Time (hr:min)	02:15
Peak Flood Arrival Time (hr:min)	04:10
Maxiumum Water Surface Elevation (ft)	677.1
Incremental Rise in Water Level (ft)	7.0
Maximum Flow (cfs)	1,600

NORTH

Page Size: 11 by 17 inches
1 inch = 1,000 feet

0 500 1,000 2,000 Feet

LEGEND

- Flow Direction
- Matchlines
- Town Halls
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INUNDATION LIMITS FOR RISING POND DAM (MA00250)

GREAT BARRINGTON, MASSACHUSETTS

INUNDATION MAP 2 OF 2

Prepared For:

GENERAL ELECTRIC COMPANY

1 Plastics Ave.
Pittsfield, MA 01201

Prepared By:

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Phone: (781) 278-3700 Fax: (781) 278-5701

Proj. Mgr.:	JDA	Dwg. Date:	11/26/2024
Designed By:	RSG		
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Operator:	RSG		

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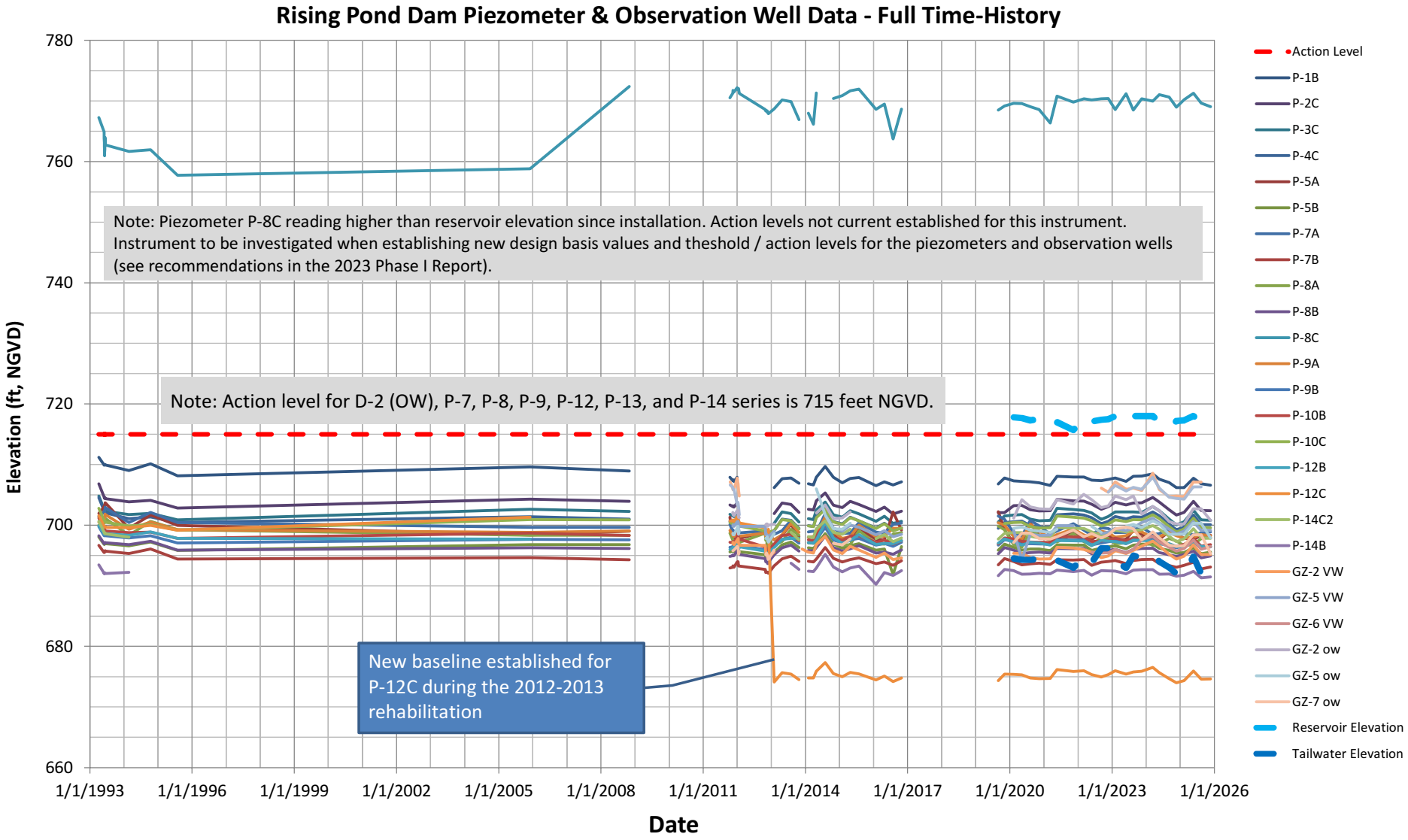
APPENDIX G – INSTRUMENTATION DATA AND WATER LEVEL RECORDS



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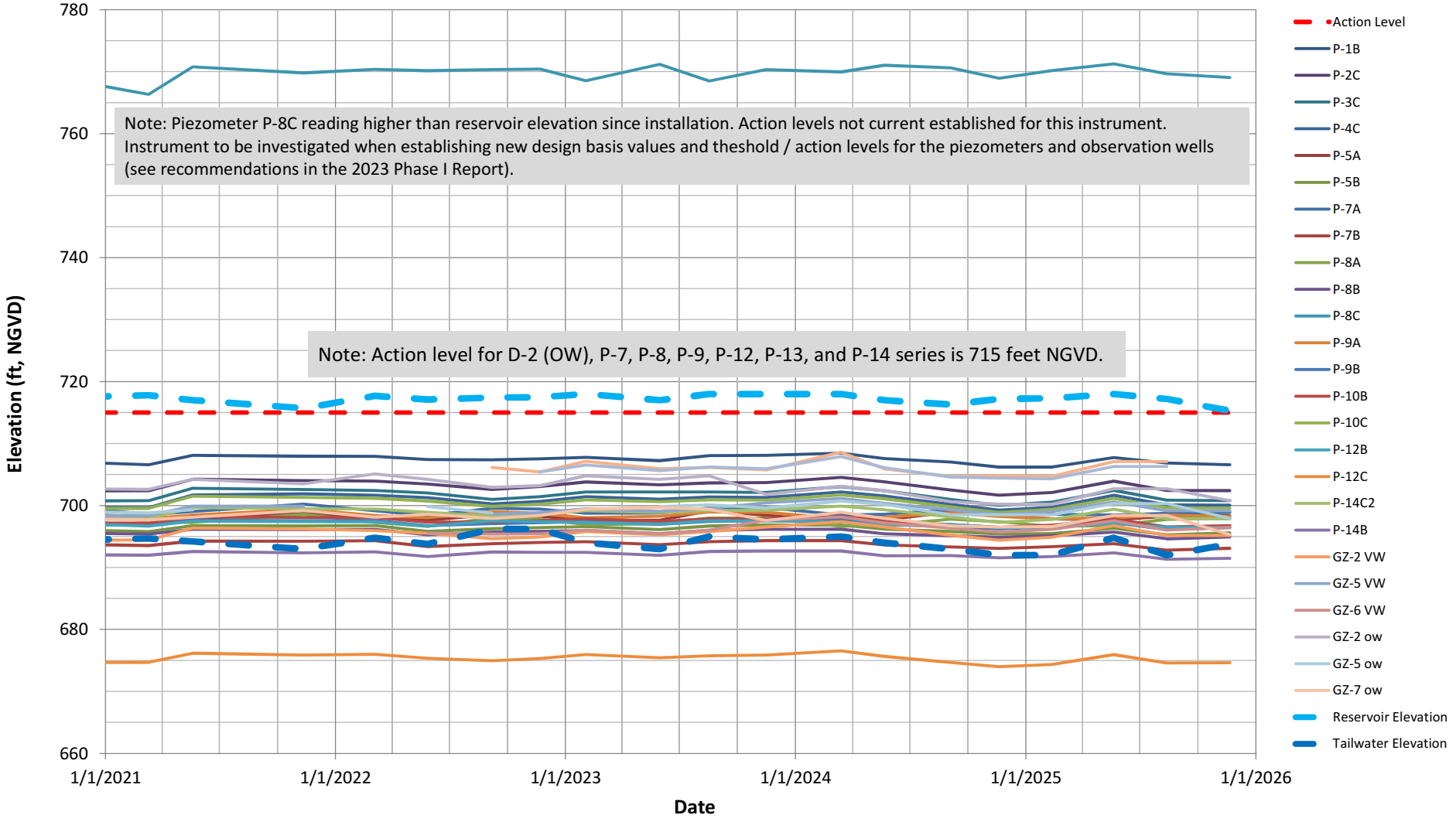
Rising Pond Dam Piezometer & Observation Well Data - 5-Year Plot



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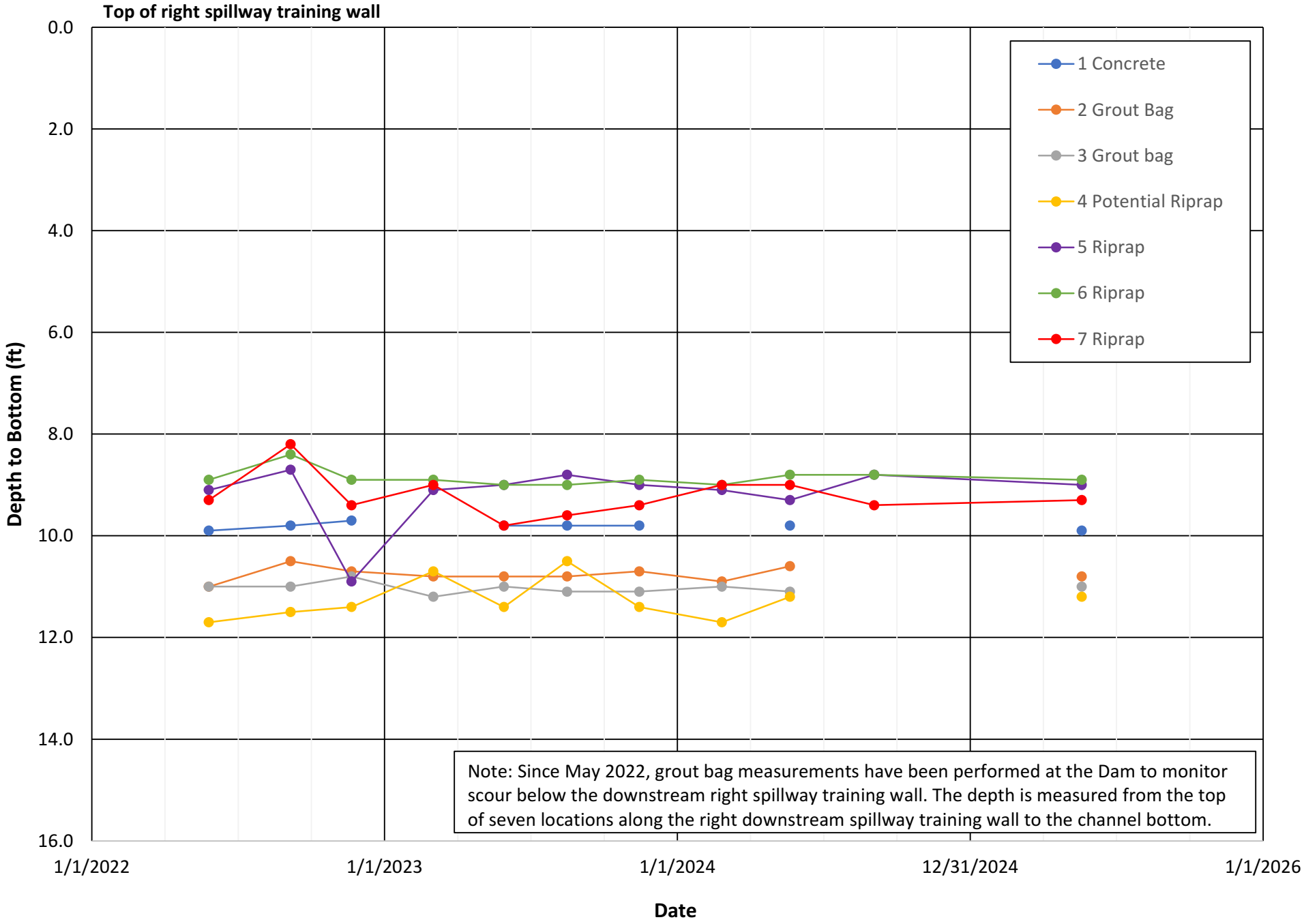
Rising Pond Dam Grout Bag Measurements



APPENDIX H – MAINTENANCE TRACKING TABLE

Rising Pond Dam – Maintenance Tracking Table – Dated February 18, 2026 (based on Phase 1 inspection conducted on November 20, 2025)			
Condition Observed Requiring Monitoring or Maintenance/Repair	**When Observed**	**Proposed Response**	**Status**
1. Some rust was present on forebay trash rack.	Since 2019 Phase 1 inspection.	Monitor and, if necessary, re-paint the forebay trash rack.	The trash rack was inspected during the 2025 dive inspection. The condition appears stable with no indication of progressive deterioration. The trash racks will condition to be monitored during future inspections.
2. Vegetation growth was observed in the right stone masonry wall of the forebay.	Since the third quarterly inspection in 2025.	Remove vegetation from masonry training wall.	Vegetation will be cleared as part of ongoing vegetation maintenance in 2026.
3. Horizontal joint/crack observed in the right forebay wall.	2025 dive inspection	Fill the joint	The joint will be filled during the next scheduled dive inspection in 2030.
4. A crack in the forebay concrete was observed on the upstream face below the abandoned operator; and minor cracking and efflorescence were observed on the downstream face of the gate platform.	Since 2019 Phase 1 inspection.	Continue to monitor this cracking/efflorescence.	This cracking/efflorescence is regularly monitored during quarterly and biennial inspections.
5. A crack was observed in the concrete step to the left of the gate platform.	Since the November 2023 Phase 1 inspection.	Fill the crack.	The crack will be filled during planned maintenance in 2026.
6. A diagonal crack was observed in the lower six courses of the pumphouse downstream brick wall near the lower right corner.	2025 Phase 1 inspection	Repoint the crack.	This crack will be repointed during planned maintenance in 2026.

Rising Pond Dam – Maintenance Tracking Table – Dated February 18, 2026 (based on Phase 1 inspection conducted on November 20, 2025)			
Condition Observed Requiring Monitoring or Maintenance/Repair	**When Observed**	**Proposed Response**	**Status**
7. Small cracks with a minor grease leak through them were observed in the gate operator.	Since the 2021 Phase 1 inspection.	None	A new gate actuator was installed on July 30, 2025. No grease leaks were observed during the 2025 Phase I Inspection.
8. A low area or "belly" was observed in the penstock about 110 feet upstream of the end of the pipe.	First observed during the October 22, 2021 penstock inspection.	Continue to investigate and monitor.	The latest report on the investigations and evaluations of the penstock was submitted to EPA on January 9, 2026 and included recommendations for repairs.
9. Surficial depression to the left of the penstock at about Sta. 1+30 measuring up to 2-ft deep and 1-to-2 feet wide.	2025 Phase 1 inspection	Fill in depression and continue to monitor the area	This depression will be backfilled using crushed peastone (or similar friable materials) during planned maintenance activities in 2026.
10. Minor debris accumulation was observed in the diversion channel.	2025 second quarterly inspection.	Monitor and remove if impeding flow.	Debris was not present during the 2025 Phase I inspection. Debris accumulation will continue to be monitored during quarterly and biennial inspections and removed if determined to be impeding flow.
11. Floating timber debris was observed upstream of the safety buoys.	Since the 2024 fourth quarterly inspection.	Remove the debris.	The debris was not impeding flow over the spillway. Debris was not removed prior to the Phase 1 inspection in November 2025. Debris will be removed during the 2026 routine maintenance activities, subject to weather and flow conditions.

Rising Pond Dam – Maintenance Tracking Table – Dated February 18, 2026 (based on Phase 1 inspection conducted on November 20, 2025)			
Condition Observed Requiring Monitoring or Maintenance/Repair	**When Observed**	**Proposed Response**	**Status**
12. Timber debris was observed in the energy dissipators at downstream toe of the spillway.	One log near the right downstream spillway training wall was observed during the 2025 Phase 1.	Continue to monitor and remove the timber if impeding flow.	One log was observed to be caught in the energy dissipators near the right downstream spillway training wall during the 2025 Phase I inspection (low flow). Once spillway flow resumed, the log did not appear to be impeding flow. The energy dissipators will continue to be monitored for the accumulation of logs
13. Minor damage to the energy dissipators.	Since the third quarterly inspection of 2024.	Monitor effectiveness of energy dissipators and repair as needed.	One of the energy dissipators (fifth from the right) was observed to be partially broken (top half of the dissipator was missing) during the Phase I inspection. There are 37 energy dissipators at the toe of the spillway. The energy dissipators function as a unit, and it is not expected that damage to one dissipator will significantly impact that overall function.
14. Minor leak emanating from a four-inch long crack or joint located below the second from right weephole at the interface between the sloping ogee concrete and the apron concrete.	2025 Phase 1 inspection	Continue to monitor during dewatered inspections.	The minor leak and crack or joint will be monitored during inspections under dewatered or low-flow conditions.
15. Areas of minor diffuse leakage observed emanating from the downstream left masonry training wall.	2025 Phase 1 inspection	Continue to monitor	The leakage will be monitored during future quarterly and biennial inspections.

Rising Pond Dam – Maintenance Tracking Table – Dated February 18, 2026 (based on Phase 1 inspection conducted on November 20, 2025)			
Condition Observed Requiring Monitoring or Maintenance/Repair	**When Observed**	**Proposed Response**	**Status**
16. Two leaks observed through the left spillway training wall.	Areas of minor clear leakage have been observed near the base of the left training wall since the 2021 Phase 1 inspection and may have been intermittently present earlier.	Continue to monitor.	The leakage was not observed during the Phase I inspection (likely due to the lowered pool level). This area will continue to be monitored during future inspections.
17. Missing gate in the right spillway training wall fence.	The gate was missing during the 2025 second and third quarterly inspections.	Replace the gate.	The gate opening has been temporarily secured with plywood to deter access. The missing gate will be replaced during regular maintenance activities in 2026.
18. A low area in the riprap is present at the downstream toe of the right embankment.	Since 2019 Phase 1 inspection.	Continue to monitor this area.	The location of this low area was marked and surveyed in February 2020. The condition will continue to be monitored during biennial and quarterly inspections.
19. Minor sloughing of the riprap downstream of the right training wall was observed.	Since the first quarterly inspection of 2024.	Continue to monitor.	This condition will continue to be monitored during quarterly and biennial inspections.
20. Minor isolated areas of taller vegetative growth were observed on the downstream slope, and a small area of reedy vegetation and minor debris were observed in the riprap at the toe of the right embankment.	Vegetation and debris have been observed since the third quarterly inspection of 2024, with minor isolated areas of taller vegetative growth noted during the 2025 second and third quarterly inspections.	Continue to remove vegetation as necessary.	Vegetation was cleared prior to the 2025 Phase 1 inspection. Vegetation management will continue to be performed during regularly scheduled maintenance activities in 2026.

Rising Pond Dam – Maintenance Tracking Table – Dated February 18, 2026 (based on Phase 1 inspection conducted on November 20, 2025)			
Condition Observed Requiring Monitoring or Maintenance/Repair	**When Observed**	**Proposed Response**	**Status**
21. Iron staining was observed along the shoreline at the toe of the right embankment (but no active seepage observed).	Since the first quarterly inspection of 2024.	Continue to monitor.	This condition will continue to be monitored for seepage during quarterly and biennial inspections.
22. Standing water beyond the toe of the right training embankment.	Periodic condition, possibly due to fluctuations in tailwater and/or surface runoff.	Continue to monitor	The condition is well beyond the downstream of the toe of the dam and does not appear to be a result of seepage through the embankment. Signs of seepage through the embankment will continue to be monitored during quarterly and biennial inspections.
23. Protective casing for monitoring well/piezometer GZ-6 was observed to be slightly tilted.	Since the 2019 Phase 1 inspection.	Continue to monitor.	The cause of the tilt in the casing is unknown; however, the condition and the tilt of the casing do not appear to be worsening over time, and the tilt has not affected the in the well or piezometer readings. The well/piezometer will continue to be measured.
24. Nested piezometer D-9 was found during 2021 left embankment raising activities. It was previously excavated and exposed, and a handhole installed over it in 2022. It still needs to be surveyed.	Since the second quarterly inspection in 2022.	The handhole installed over this piezometer and the casing elevations need to be surveyed.	Completed; the handhole and casing elevations were surveyed during the 2025 topographic/ bathymetric survey conducted in July 2025, with supplemental surveys in October and November 2025.

Rising Pond Dam – Maintenance Tracking Table – Dated February 18, 2026 (based on Phase 1 inspection conducted on November 20, 2025)			
Condition Observed Requiring Monitoring or Maintenance/Repair	**When Observed**	**Proposed Response**	**Status**
25. Minor buildup of sediment/silt was measured in monitoring wells GZ-2, GZ-5, and GZ-7. However, the current sediment/silt levels are not impacting the water level readings at these monitoring wells.	Observed in this and other monitoring wells since the second quarterly inspection in 2022.	Continue to monitor.	Sediment/silt buildup will continue to be monitored during quarterly and biennial inspections.
Note: Gray-shaded cells indicate that a listed condition had been addressed or was not present during the current inspection.			

APPENDIX I - 2025 UNDERWATER DIVE INSPECTION SUMMARY

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M E M O R A N D U M

To: File

From: Seth D. Krause, P.E. (GZA)
Jonathan Andrews, P.E. (GZA)

Date: February 18, 2026

File No.: 01.0019896.81

Re: 2025 Underwater Dive Inspection Summary
Rising Pond Dam (MA00250)
Great Barrington, Massachusetts

GZA GeoEnvironmental (GZA) has prepared this memorandum to describe the dive inspection performed by Seaway Diving and Salvage Co. (Seaway), a subcontractor to GZA, at the Rising Pond Dam in Great Barrington, Massachusetts, on behalf of the General Electric Company (GE). The information provided in this memorandum is subject to the Limitations provided in **Attachment A**.

The dive inspection was conducted on July 8 and 9, 2025. It was conducted under the Operation, Monitoring, and Maintenance (OM&M) Plan for Rising Pond Dam submitted and approved by the U.S. Environmental Protection Agency (EPA) in August 2019 and amended in 2020. It should be noted that a further revised OM&M Plan for Rising Pond Dam was submitted to EPA on December 19, 2024. That further revised OM&M Plan was under EPA review in the summer of 2025 and was later conditionally approved by EPA in a letter dated October 21, 2025, which required GE to revise that plan again. GE submitted a further revision of that plan addressing EPA's conditions on December 22, 2025, that revised plan was approved by EPA on February 12, 2026. In the meantime, since this dive inspection was conducted prior to receiving EPA's conditional approval on October 21, 2025, it was conducted in accordance with the 2019 OM&M Plan as amended, and this dive inspection memorandum was prepared in accordance with that plan.

The OM&M Plan states that a dive inspection will be conducted, at a minimum, every five years and when deemed warranted by a dam safety engineer. The objective of the dive inspection was to observe and document submerged portions of the spillway, forebay, and adjacent structures that cannot be seen from above the water surface.

To accomplish the stated objective, Seaway performed an underwater tactile, physical inspection and a qualitative visual assessment through a manned dive survey. The results of this inspection will be used to assist in evaluating the integrity of inspected structures. These results will also be maintained in GE's files to provide a history of observed conditions at the Dam.

BACKGROUND

Rising Pond Dam is a run-of-the-river structure located on the Housatonic River, consisting of left- and right-side earthen embankments, a 127-foot-long central ogee-shaped spillway, upstream and downstream concrete aprons, and outlet works to the left of the spillway. The spillway is a concrete-faced timber crib structure with a steel crest plate. The spillway training walls are a combination of concrete, mortared stone masonry, and steel sheetpiles.

The low-level outlet is located directly to the left of the spillway. The low-level outlet works consist of a mortared stone masonry forebay with a steel trash rack, a concrete gate platform with sluice gate, and a 14-foot-diameter steel penstock that transitions to an open diversion channel discharging into the Housatonic River approximately 230 feet downstream of the dam. The upstream invert elevation (El.) of the penstock is reported to be 699 feet.[1] A fire-protection pumphouse that services the Rising Paper Mill is located to the left of and adjacent to the forebay on the left (east) embankment. The pumphouse is unrelated to dam operations. A wide embankment/fill area is present on the left side of the spillway and outlet structures. The upstream slopes are surfaced with a combination of grass and riprap.

METHODOLOGY

The inspection of the underwater portions of the spillway, forebay, and adjacent structures, was conducted by Seaway on July 8 and 9, 2025 with GZA personnel on-site to oversee the inspection. The underwater inspection of the forebay was conducted on July 8, 2025 under normal water surface elevations (pool elevation about 717 feet). The underwater inspection upstream and downstream of the spillway and adjacent structures was conducted on July 9, 2025 under lowered pool conditions (pool elevation about 712 feet; approximately 4 feet below the spillway crest).

The three-person dive team included the diver, the site supervisor, and a dive tender. The dive inspection was performed using supplied air and a live video feed which was viewable by GZA in real time. Video recordings with audio narration were provided by Seaway as part of the inspection deliverable package.

GE's Contractor, LB Corporation (LB), opened the low-level outlet gate on July 9, 2025 to temporarily lower the reservoir level by routing river flows through the penstock to expose the upstream apron and downstream face and toe of the spillway. At the conclusion of the dive inspections, the gate was sequentially lowered to maintain flow downstream. Once the impoundment elevation was restored to normal operating levels (about El. 716 feet) and overtopped the uncontrolled spillway, the gate was fully closed.

Housatonic River flow at the downstream United States Geological Survey (USGS) Division Street River gage (USGS Gage 01197500) was reported to be approximately 67 to 380 cubic feet per second (cfs)[2] during the July 8 and 9 inspections (prior to and after gate operation).

Photographs, including screenshots of video frames ("still images") from the dive inspection, are provided in **Attachment B**. Areas inspected and photograph locations are shown on **Figure 1**.

[1] All elevations in this report are presented in National Geodetic Vertical Datum of 1929, NGVD29.

[2] USGS Division Street River Gage 01197500. Housatonic River Near Great Barrington, MA - USGS Water Data for the Nation

INSPECTION TERMINOLOGY AND DEFINITIONS

In accordance with Section 3.0 of the OM&M Plan, the dive inspection was performed to document underwater conditions, including, but not limited to, potential cracks or voids, erosion and undermining, structural separations at material boundaries, and/or concrete of questionable or deteriorated quality. Based on the inspection, the conditions were assessed using the following terms:

- Sound – Intact, stable, free from deficiency (unless otherwise noted).
- Separated – Concrete present, but not in contact with adjoining material (e.g. sheetpile or stone masonry).
- Missing – Void in the area of a joint, base, or riprap.

The conditions of concrete surfaces were typically evaluated using the following terms:

- Spall – Generally circular or oval depression in the concrete surface.
- Crack – Horizontal, vertical or transverse gap in the concrete or masonry.

FIELD INVESTIGATIONS AND OBSERVATION SUMMARY

As previously noted, Seaway conducted the dive inspection upstream and downstream of the Rising Pond Dam spillway on July 8 and 9, 2025. Seth Krause (in both days) and Nicolette Schluter (on July 8, 2025) representing GZA were on site to observe the dive inspection. Tom Czelusniak (on both days) and Alex Carli-Dorsey (on July 8, 2025) were on site representing EPA. This inspection was intended to provide information regarding the overall condition and integrity of the upstream spillway and training walls, forebay, slide gate, sheetpile, and riprap underwater areas.

The overall conditions and integrity of the upstream spillway and training walls, forebay, slide gate, sheetpile, and riprap underwater areas were similar to those observed during the last prior dive inspection, conducted in 2020. No progressive deterioration or undermining of the underwater structures was observed which would impact the overall condition and integrity the structures.

Below is a general summary of the conditions observed during the 2025 dive inspection. It is noted that conditions of note observed during the dive inspection were quantified, where possible, including lengths, widths, and depths of concrete cracking, undermining, and deterioration. The divers attempted to measure and quantify the deterioration, corrosion and pitting that was observed at the steel structures; however, the divers were unable accurately quantify the observed steel conditions due to poor visibility. Where a condition of note could not be measured, the condition was described qualitatively.

<u>SETUP AND LOGISTICS</u>

On July 8, 2025, the Seaway team set up its land-side support equipment near the left abutment to access the forebay. On July 9, 2025, the Seaway team set up on the downstream slope of the right embankment to access the upstream face of the spillway, the right embankment sheetpiles, and the areas downstream of the spillway, including the spillway toe, downstream training walls, and area downstream of the diversion channel. Visual assessments of the underwater areas were performed by a Seaway diver with an audio and video helmet camera communication. Seaway observed the video feed and communicated with the diver from the shore-based trailer,

with observation by GZA. The limits of the dive inspection, including the left side to a distance 20 feet beyond the diversion channel discharge, are shown on **Figure 1**.

TRASHRACK, SLIDE GATE, SHEETPILES, AND FOREBAY WALLS

The low-level outlet works to the left of the spillway consists of a mortared stone masonry forebay with a steel trash rack and a concrete-walled gate chamber with slide gate that controls flow to the 14-foot-diameter steel penstock that discharges downstream. The slide gate was fully closed and locked out in the closed position during the dive inspection within the forebay.

The trash rack, slide gate, sheetpiles and forebay wall were inspected on July 8, 2025 under typical water levels (about El. 717 feet). Overall, the upstream face of the trash rack and its corresponding components appeared to be in sound condition. Components appeared to be in place with no breakage, significant wear or deterioration, or cracks. Mild rust was observed in a few locations. A steel plate was observed on the upstream side of the trash rack, generally covering the lower half of the trash rack. Sediment (about two to three inches thick) and logs were encountered on the forebay floor and along the base of the steel trash rack. The upstream steel plate, measured during the 2020 dive inspection, is about seven feet tall and spans the width of the trash rack. During the 2025 inspection, the upstream plate hindered observation of the lower portion of the trash rack on the upstream side. Horizontal steel members span the forebay and support the trash rack. The lower support member was covered by a thin layer of sediment. Two steel cables are connected to the steel plate by shackles, likely for plate removal as needed. The cables generally appeared to be sound with slight wear. -The downstream face of the trash rack was also inspected during the 2025 dive inspection from within the forebay. The downstream face / bottom half of the trash rack was in similar condition to that of the upstream face / upper half. Mild rust and marine growth were observed on the downstream side of the trash rack. Sediment at the bottom of the trash rack hindered observation of the bottom few inches.

Overall, the upstream side of the penstock slide gate and visible gate sealing surfaces were observed to be in sound condition. It is noted that the slide gate was rehabilitated in August 2021 (after the 2020 dive inspection conducted), as discussed in a February 16, 2022 GZA memorandum titled “Summary of Penstock Gate Inspection/Rehabilitation and Left Embankment/Forebay Dam Raising Activities.” One of the primary objectives of the gate rehabilitation was to reduce gate leakage through the gate seals surfaces (sides and bottom). During the 2025 dive inspection, the divers confirmed that the gate was well-sealed with little to no positive flow (apparent leakage) around the perimeter of the gate.

No deficiencies were observed in the forebay sheetpiles. The river bottom upstream of the sheetpile appeared to consist of soft sediment. No undermining, erosion, or scouring was observed; but some rust, pits, and rust blisters of the sheetpiles were observed near the steel trash rack. The interface between the sheetpile and concrete near the trash rack generally showed sound contact, but the few inches of mud hindered continuous observations.

The forebay masonry walls (downstream of the trash rack) were generally observed to be sound with isolated instances of mortar loss or deterioration. There were a few missing bricks observed in the left brick forebay wall. The missing bricks and instances of mortar loss/deterioration were not continuous, and the masonry and brick forebay walls were generally intact and stable. Additional above-water visual observations were made on July 9, 2025, when the pool was lowered. These observations confirmed some of the observations made by the diver on the previous day.

An approximately 16-foot long by ⅛-inch wide (vertical dimension) horizontal joint or crack (joint/crack) was observed at the concrete-to-masonry interface on the right forebay wall. The joint was located about three to four feet below the water surface at the time of the inspection (about El. 713 to El. 714 feet). Using a folding ruler, the joint/crack was probed horizontally (rightwards towards the spillway) at up to seven inches deep. The divers noted positive flow into the joint/crack during the inspection. The divers introduced water-soluble, biodegradable green tracing dye into the joint/crack, which subsequently was sucked into the joint/crack. GZA and EPA observed discharge of dye at two points on the spillway side of right forebay masonry wall (e.g., left downstream spillway training wall), estimated to be about 13 feet below the top of the forebay concrete walkway (about El. 713 feet). Discharge was estimated to be about one to two gallons per minute (gpm). These observed dye discharge points are at two locations on the left spillway training wall where leakage discharge has regularly been observed during past quarterly and Phase 1 inspections. The left training wall leakage at these locations was not observed on July 9, 2025 under the lowered water levels (water level about El. 712 feet), which is about a foot or two below the horizontal joint/crack.

SPILLWAY CREST, UPSTREAM APRON, LEFT UPSTREAM TRAINING WALL, AND RIGHT EMBANKMENT SHEETPILES

The spillway crest, upstream apron, and right embankment sheetpiles were inspected on July 9, 2025 under lowered water levels (about El. 712 feet). The spillway crest and much of the upstream apron and the sheetpiles were visible above the water level at the time of the inspection. Diver inspection of these structures was supplemented by visual observation from the forebay structure and right embankment.

Overall, the spillway crest, upstream apron, and sheetpiles appeared sound with no apparent misalignment, settlement, erosion, undermining, or deterioration observed. Marine growth was observed on the spillway crest.

The upstream concrete apron was in contact with the upstream, left, and right sheet piles. No undercutting was noted along the interfaces between concrete, sheetpiling, and river bottom. The diver noted that the sheetpiling extended above the concrete apron by about six inches near the left side of the spillway and was buried by sediment near the right side of the spillway. A few logs were present on the upstream apron near the right side of the spillway. The river bottom upstream of the apron base ranged from muddy to rocky and muddy with wood, leaves, and other organic matter. No scoured areas were observed in the river bottom.

Deficiencies were not observed on the left upstream training wall (dividing wall between spillway and forebay). The interfaces between the concrete/stone masonry wall, sheetpile, and concrete were observed to be in sound condition, with no signs of separation. Joints were observed to be free of erosion and scouring. Some loose rock debris was encountered on top of the sheet piling.

Overall, the right embankment sheetpiles appeared intact and stable. Minor surface rusting, corrosion, and discoloration of the sheetpiles was observed. The riprap armoring on the right embankment upstream slope beyond (to right of) the sheetpiles was observed to be intact with no signs of sloughing or scouring.

SPILLWAY DOWNSTREAM APRON AND TOE

The spillway downstream apron and toe were inspected on July 9, 2025 under lowered water levels (pool about El. 712 feet). The ogee-shaped spillway is concrete-faced with a steel crest plate, upstream concrete apron and steel sheet piles, and downstream concrete apron with concrete energy dissipaters and steel sheet piles. Riprap is present downstream of the spillway apron and energy dissipators. The riprap coverage is in contact with adjacent concrete and steel sheetpiling. The entire spillway downstream apron and much of the toe were visible above the tailwater level at the time of the inspection. Inspection of these structures was completed via visual

observation from the downstream apron, as well as by the diver in the water at the toe of the spillway. Water depths at the toe of the spillway during the dive ranged from about one to two feet.

Overall, the spillway downstream apron and toe appeared sound with no apparent misalignment, settlement, erosion, undermining, or significant deterioration observed.

Marine growth was observed on the downstream apron of the spillway. A broken energy dissipator was observed near the right downstream end of the spillway. This broken energy dissipator was first observed during the third quarterly inspection in 2024. There are 37 energy dissipators at the toe of the spillway, and all other energy dissipators were observed to be intact. Since the energy dissipators function as a unit, it is not expected that damage to one dissipator will significantly impact their overall function. The overall effectiveness of the energy dissipators was subsequently reviewed during the November 2025 biennial Phase 1 inspection, as discussed in GZA's February 2026 report on that inspection. A few logs were observed at the right downstream end of the spillway apron.

A minor horizontal crack or joint measuring about four inches long was observed on the right downstream face of the spillway at the transition to the spillway apron. The crack had clear leakage of less than about one gpm. There are 11 weepholes in the downstream face of the spillway. The third weephole from the right was leaking less than one gpm of clear water at the time of the inspection. Additional discussion about these leaks is provided in the report on the 2025 Phase 1 inspection.

Steel sheetpiling conditions along the toe of the downstream apron appeared sound, with no signs of undermining, missing material, or corrosion. Riprap was observed along the sheet piles. No separation between the concrete apron, steel sheetpiles, and downstream riprap was observed. Riprap coverage along the toe of the dam was continuous and appeared to be in sound condition.

DOWNSTREAM TRAINING WALLS

The downstream training walls were inspected on July 9, 2025 under lowered water levels (about El. 712 feet). The two downstream training walls are a combination of concrete, mortared stone masonry, and steel sheetpiles. Water depths along the right downstream training wall (concrete) were up to about five inches deep, allowing for clear visual observation of the base of the training wall. The water depths along the left downstream training wall (stone masonry/sheet pile) were about two to four feet deep.

Overall, the downstream training walls appeared sound with no apparent misalignment, settlement, erosion, undermining, or significant corrosion or deterioration observed. There was some deterioration of the right downstream training wall concrete below the c.a. 2012 training wall repairs.

Surficial conditions of the training walls generally appeared free of cracks or spalls in the concrete or stone masonry, although some minor pitting and rust were found in a limited area near the intersection of the spillway and the left training wall. No separations were observed at the interface joints between the spillway sheetpiles or at the concrete/stone masonry portions of each training wall.

Material at the base of both spillway training walls consisted of riprap that slowly transitioned to natural river rock, sand, and construction debris as the diver moved downstream. Smaller rocks were encountered closer to the transition point between riprap and natural river rock; and wooden sticks, logs, and pieces of reinforcing bar were encountered mixed in with riprap or river rock at various locations along both walls. The training walls and riprap appeared to be in generally sound condition.

As part of the 2011 through 2013 rehabilitation work at Rising Pond Dam, voids under the downstream portion of the right training wall had been filled with diver-placed grout bags. During the July 2025 dive inspection, the grout bags were intact and stable along the right training wall. Construction debris such as rebar embedded in concrete pieces were observed intermittently at the base of the training wall.

The left training/sheetpile wall was inspected along its length from the toe of the spillway to about 20 feet beyond the diversion channel discharge. The riprap along the length of this training/sheetpile wall was observed to be intact. No undermining or scour at the base along this length of sheetpile was observed.

OVERALL EVALUATION AND RECOMMENDATIONS

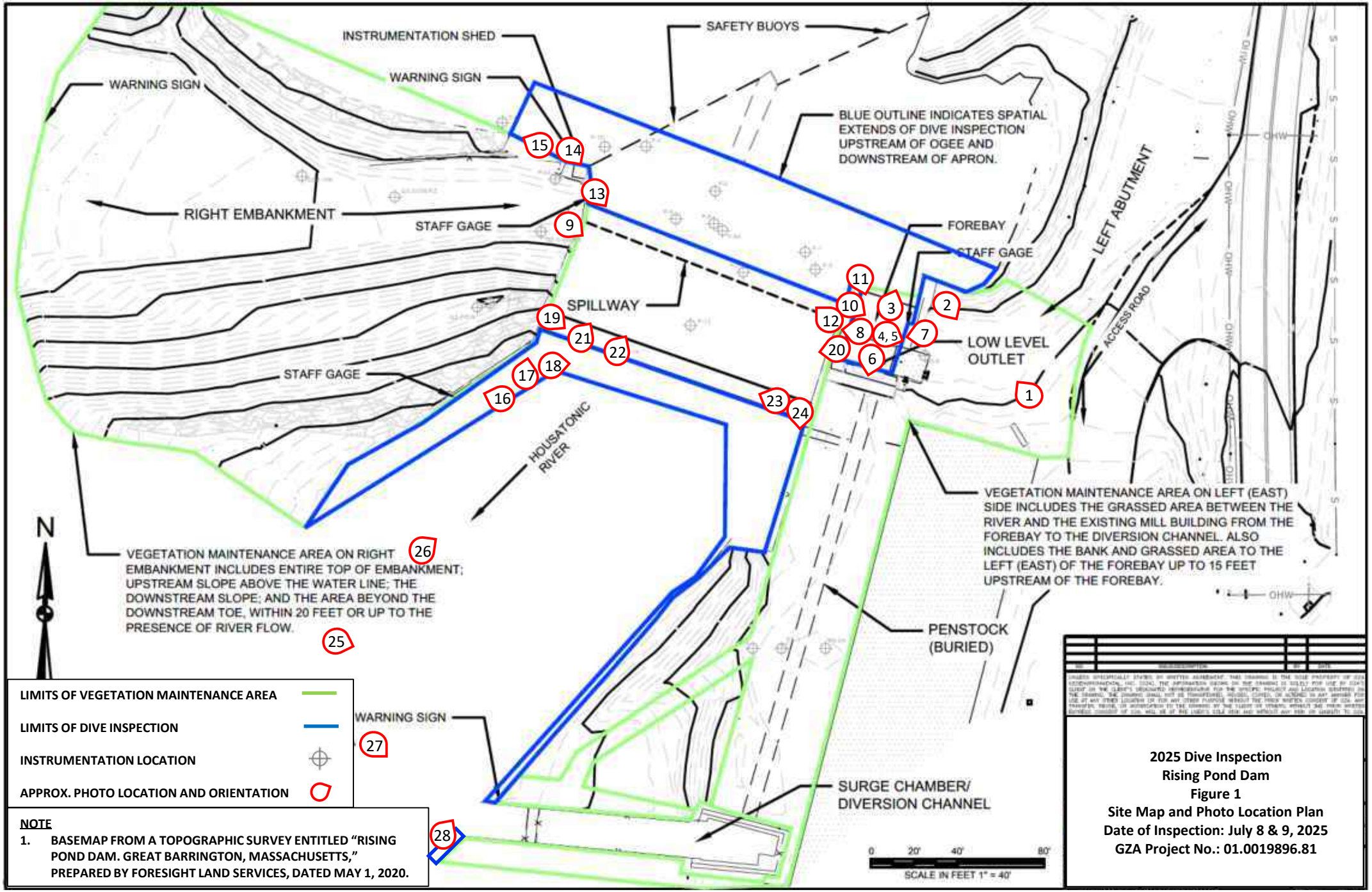
The overall conditions and integrity of the inspected areas, including the spillway, training walls and aprons, forebay, slide gate, sheet pile, and riprap observed during the July 8 and 9, 2025 dive inspection, were similar to those observed during the last prior dive inspection in 2020. No progressive deterioration or undermining of the underwater structures was observed which would impact the overall condition and integrity the structures. We recommend that the dive inspection be repeated in five years (i.e., in 2030) as specified in the OM&M Plan.

Based on the dye testing, it is apparent that the source of the leakage through the right forebay masonry wall is the continuous horizontal joint/crack that was observed during this dive inspection. We recommended repair of the joint/crack in the right forebay wall during the next dive inspection, scheduled for 2030.

Please feel free to contact us if you have any questions or comments regarding the content of this memorandum.

Attachments: Figure 1 - Dive Inspection Summary and Still Image Location Plan
Attachment A: Limitations
Attachment B: Inspection Photographs

FIGURE



NOTE

1. BASEMAP FROM A TOPOGRAPHIC SURVEY ENTITLED "RISING POND DAM. GREAT BARRINGTON, MASSACHUSETTS," PREPARED BY FORESIGHT LAND SERVICES, DATED MAY 1, 2020.

2025 Dive Inspection
Rising Pond Dam
Figure 1
Site Map and Photo Location Plan
Date of Inspection: July 8 & 9, 2025
GZA Project No.: 01.0019896.81

ATTACHMENT A
Limitations

DAM ENGINEERING REPORT LIMITATIONS

Use of Report

1. GeoEnvironmental, Inc. (GZA) prepared this report on behalf of, and for the exclusive use of the General Electric Company, (Client) for the stated purpose(s) and location(s) identified in the Report. Use of this report, in whole or in part, at other locations, or for other purposes, may lead to inappropriate conclusions; and we do not accept any responsibility for the consequences of such use(s). Further, reliance by any party not identified in the agreement, for any use, without our prior written permission, shall be at that party's sole risk, and without any liability to GZA.

Standard of Care

2. Our findings and conclusions are based on the work conducted as part of the Scope of Services set forth in the Report and/or proposal, and reflect our professional judgment. These findings and conclusions must be considered not as scientific or engineering certainties, but rather as our professional opinions concerning the limited data gathered during the course of our work. Conditions other than described in this report may be found at the subject location(s).
3. Our services were performed using the degree of skill and care ordinarily exercised by qualified professionals performing the same type of services at the same time, under similar conditions, at the same or a similar property. No warranty, expressed or implied, is made.

General

4. The observations described in this report were made under the conditions stated therein. The conclusions presented were based solely upon the services described therein, and not on scientific tasks or procedures beyond the scope of described services or the time and budgetary constraints imposed by the Client.
5. in preparing this report, GZA relied on certain information provided by the Client, state and local officials, and other parties referenced therein available to GZA at the time of the evaluation. GZA did not attempt to independently verify the accuracy or completeness of all information reviewed or received during the course of this evaluation.
6. Any GZA hydrologic analysis presented herein is for the rainfall volumes and distributions stated herein. For storm conditions other than those analyzed, the response of the site's spillway, impoundment, and drainage network has not been evaluated.
7. Observations were made of the site and of structures on the site as indicated within the report. Where access to portions of the structure or site, or to structures on the site was unavailable or limited, GZA renders no opinion as to the condition of that portion of the site or structure. In particular, it is noted that water levels in the impoundment and elsewhere and/or flow over the spillway may have limited GZA's ability to make observations of underwater portions of the structure. Excessive vegetation, when present, also inhibits observations.
8. In reviewing this Report, it should be realized that the reported condition of the dam is based on observations of field conditions during the course of this study along with data made available to GZA. It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued inspection and care can there be any chance that unsafe conditions be detected.

Compliance with Codes and Regulations

9. We used reasonable care in identifying and interpreting applicable codes and regulations. These codes and regulations are subject to various, and possibly contradictory, interpretations. Compliance with codes and regulations by other parties is beyond our control.
10. This scope of work does not include an assessment of the need for fences, gates, no trespassing signs, swimming or boating barriers, repairs to existing fences and railings and other items which may be needed to minimize trespass and provide greater security for the facility and safety to the public. An evaluation of the project for compliance with OSHA rules and regulations is also excluded.

Additional Services

11. It is recommended that GZA be retained to provide services during any future: site observations, explorations, evaluations, design, implementation activities, construction and/or implementation of remedial measures recommended in this Report. This will allow us the opportunity to: i) observe conditions and compliance with our design concepts and opinions; ii) allow for changes in the event that conditions are other than anticipated; iii) provide modifications to our design; and iv) assess the consequences of changes in technologies and/or regulations.

ATTACHMENT B
Inspection Photographs

2025 Dive Inspection Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
1	7/8/2025

Direction Photo Taken:
Upstream and right

Description:
Dive inspection setup near the left abutment.

Photo No.	Date:
2	7/8/2025

Direction Photo Taken:
Left

Description:
Dive inspection setup near the left abutment.

2025 Dive Inspection Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
3	7/8/2025

Direction Photo Taken:
Upstream

Description:
Typical condition of the trash rack in the forebay. Note marine growth and minor surficial rusting.

Photo No.	Date:
4	7/8/2025

Direction Photo Taken:
Left

Description:
Missing brick in the left forebay wall at about 8 to 10 feet below the water surface.

2025 Dive Inspection Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
5	7/8/2025

Direction Photo Taken:
Left

Description:
Fire pump intake near the base of the forebay. Intake feeds Pumphouse for Mill fire protection system.

Photo No.	Date:
6	7/8/2025

Direction Photo Taken:
Downstream

Description:
Upstream face of the gate at the invert, about 19-ft depth. Note minimal- to no-gate leakage.

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
7	7/8/2025

Direction Photo Taken:
Right

Description:
Location of the horizontal joint/crack in right forebay wall at the masonry to concrete interface ~3-feet below the water surface. The joint is about 16-feet long, ⅛-inch wide and was probed up to about 7-inches deep. Dye inserted underwater at approximate location of diver (red arrow). See Photos 8 & 9.

Photo No.	Date:
8	7/8/2025

Direction Photo Taken:
Right

Description:
Horizontal joint in right forebay wall at the masonry to concrete interface. Dye introduced was sucked into the joint/crack and discharged on the other side of the wall (left spillway training wall).

See Photo 9 for discharge location of dye.

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
9	7/8/2025

Direction Photo Taken:
Left

Description:
Location of dye leaking through the left spillway training wall.

See Photos 7 and 8 for location where dye was introduced in the right forebay wall.

Photo No.	Date:
10	7/9/2025

Direction Photo Taken:
Left

Description:
Left forebay wall, lower water elevation. Deterioration of the exposed brick below the normal water level (red arrow).

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Client Name:	Site Location:	Project No.
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Photo No.	Date:
11	7/9/2025

Direction Photo Taken:
Downstream

Description:
Upstream face of the low-level outlet, lower water elevation.

Photo No.	Date:
12	7/9/2025

Direction Photo Taken:
Right

Description:
Upstream apron, crest, and downstream face of the spillway. Note lowered pool level.

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Client Name:	Site Location:	Project No.
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Photo No.	Date:
13	7/9/2025

Direction Photo Taken:
Left

Description:
Upstream apron and spillway crest with lowered pool level

Photo No.	Date:
14	7/9/2025

Direction Photo Taken:
Downstream and left

Description:
Upstream face of the sheetpiles at the right embankment/ instrumentation shed with lowered pool level. Minor surface rusting and corrosion of the sheetpiles.

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Client Name:	Site Location:	Project No.
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Photo No.	Date:
15	7/9/2025

Direction Photo Taken:
Right

Description:
Upstream slope of the right embankment with lowered pool level.

Photo No.	Date:
16	7/9/2025

Direction Photo Taken:
Right

Description:
Right downstream training wall with lowered pool level.

Note good contact at the base of the wall; no scoured or undermined areas observed.

2025 Dive Inspection
Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
17	7/9/2025

Direction Photo Taken:
Upstream

Description:
Right downstream training wall at the toe of the spillway with lowered pool level.

Note some deterioration of concrete below more recent (ca 2012) wall repairs; good contact at wall base; no undermined areas observed; ca 2012 grout bags visible and in-place (photo right)

Photo No.	Date:
18	7/9/2025

Direction Photo Taken:
Upstream

Description:
Downstream toe and face of the spillway. Contact along the toe of the spillway intact, no undermining or deterioration of the sheetpiles observed. Note grout bags visible (photo left), along with downstream riprap (photo right and foreground)

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
19	7/9/2025

Direction Photo Taken:
Left

Description:
Alignment along the downstream toe of the spillway.

Red circle is the approximate exit point of the tracer dye that was introduced to the horizontal joint/crack within the forebay (see Photos 8 and 9).

Photo No.	Date:
20	7/9/2025

Direction Photo Taken:
Downstream and right

Description:
Downstream toe of the spillway, lower water elevation.

Note broken energy dissipator (red arrow) and minor leakage (red circle).

See Photos 21 and 22 for more detail.

2025 Dive Inspection Photographic Log

Client Name:	Site Location:	Project No.
General Electric Company	Rising Pond Dam (MA00250), Great Barrington, MA	01.0019896.81

Photo No.	Date:
21	7/9/2025

Direction Photo Taken:
Upstream

Description:
Broken energy dissipator (5th from right) at the toe of the spillway.

Photo No.	Date:
22	7/9/2025

Direction Photo Taken:
Upstream

Description:
Minor horizontal joint or crack and leakage at the slope transition at the downstream toe of the spillway. Crack was about 4 inches long and leakage was clear and less than 1 gpm.

2025 Dive Inspection Photographic Log

Client Name:	Site Location:	Project No.
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Photo No.	Date:
23	7/9/2025

Direction Photo Taken:
Right

Description:
Downstream face of the spillway.

Photo No.	Date:
24	7/9/2025

Direction Photo Taken:
Downstream

Description:
Upstream end of the left bank sheetpile wall / downstream left end of the spillway.

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Client Name:	Site Location:	Project No.
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Photo No.	Date:
25	7/9/2025

Direction Photo Taken:
Left

Description:
Left bank sheetpile wall downstream of dam.

Diver walked the length of the sheetpiles (red arrows). Contact along the length of the sheetpiles was intact, no undermining, scour or sheetpile deterioration was observed.

Photo No.	Date:
26	7/9/2025

Direction Photo Taken:
Upstream

Description:
Downstream face of the spillway and right training wall. Diver walked the length of the training wall and spillway apron (red arrows). Contact along the length of the structures was intact, no undermining, scour or significant deterioration was observed.

2025 Dive Inspection Photographic Log

Client Name:	Site Location:	Project No.
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Photo No.	Date:
27	7/9/2025

Direction Photo Taken:
Left

Description:
Left bank sheetpile wall at the diversion channel outlet.

Diver observed the riprap downstream of the diversion channel (red circle). Riprap appeared intact; no scour or displaced riprap observed.

Photo No.	Date:
28	7/9/2025

Direction Photo Taken:
Upstream

Description:
Downstream face of the dam, the left bank sheetpile wall, and diversion channel discharge. Diver walked the sheetpiles downstream of diversion channel outlet (red arrows). Contact along the length of the sheetpiles was intact, no undermining, scour or sheetpile deterioration was observed.