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Engineering Evaluation/ Cost Analysis (EE/CA)

Aerovox, Inc. New Bedford, Massachusetts

August 1998



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1. Introduction

BLASLAND, BOUCK & LEE, INC.

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1. Introduction

1.1 General

This report presents the Engineering Evaluation/Cost Analysis (EE/CA) for implementation of a non-time critical removal action to address chemicals of concern at the Aerovox, Inc. (Aerovox) facility (the site) located in New Bedford, Massachusetts. This EE/CA has been prepared by Blasland, Bouck & Lee, Inc. (BBL) at the request of Ropes & Gray, attorneys for Aerovox, and presents an analysis of removal action alternatives for the site.

The United States Environmental Protection Agency (USEPA) has determined that a removal action is appropriate for the Aerovox facility pursuant to Section 106 of the Comprehensive Environmental Response, Compensation, and Liability Act (CERCLA), and that at least six months of planning time exists before on-site removal activities must be initiated. Accordingly, the removal action to be implemented is non-time critical [40 CFR 300.415(b)(4)].

As presented in USEPA's *Guidance on Conducting Non-Time Critical Removal Actions Under CERCLA* (August 1993), non-time critical removal actions may be interim or final actions depending upon the conditions of the site and the specific goals and objectives of the removal action. The National Contingency Plan (NCP) [40 CFR 300.415(e)] provides some examples of removal actions, including measures that limit access; reduce migration and prevent contact through containment or capping; remove materials that contain chemicals of concern; excavate/consolidate source materials; or provide treatment, disposal or incineration.

1.2 Purpose and Scope of this EE/CA

The purpose and scope of this EE/CA is to identify the objectives and goals of the removal action for the Aerovox facility and to analyze the effectiveness, implementability, and cost of appropriate removal action alternatives that satisfy these objectives. This EE/CA also provides a vehicle for public involvement, as it will be made available for public comment in accordance with 40 CFR 300.415(n). Additionally, this EE/CA, along with other documents/information which form the basis for the removal action to be implemented at the Aerovox facility, will be part of the USEPA's Administrative Record File. As detailed in 40 CFR 300.820(a), the Administrative Record File shall be made available for public inspection when the EE/CA is made available for public comment.

1.3 Removal Action Process

The USEPA issued a July 15, 1998 Approval Memorandum (Memorandum) to initiate the EE/CA process. This Memorandum justifies conducting an EE/CA by documenting that the site conditions at the Aerovox facility meet the NCP criteria for initiating a removal action and that the proposed action is non-time critical. A copy of this Memorandum is provided as Attachment 1.

Prior to the start of the non-time critical removal action public comment period, the USEPA will publish a Notice of Availability and a brief description of the EE/CA. This notice will announce the public comment period during which the public has the opportunity to review and comment on the EE/CA and the proposed removal action. A written response to each significant comment received during the public comment period will be produced and included as the Responsiveness Summary in the Action Memorandum. The results of the EE/CA, along with the USEPA's response decision, will be summarized in the Action Memorandum. Once the Action Memorandum and the Responsiveness Summary are prepared, the removal action will be initiated. An Administrative Record File for the removal action will be established and made available for public inspection as specified in the NCP (Sections 300.820 and 300.825). The non-time critical removal action process is presented on Figure 1.

1.4 Report Organization

This EE/CA report is organized as follows:

- Section 2.0 presents the site characterization, including a summary of the site location and physical setting, regional geology, site history, recently completed removal investigation activities, and a streamlined risk evaluation. This section also presents a summary of information regarding the geology/hydrogeology of the site;
- Section 3.0 identifies the potentially applicable or relevant and appropriate requirements (ARARs) associated with a removal action at the site;
- Section 4.0 identifies the scope, goals, and objectives of the removal action;
- Section 5.0 identifies and presents an analysis of removal action alternatives; and
- Section 6.0 presents a comparative analysis of the removal action alternatives and the recommended removal action.

2. Site Characterization

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2. Site Characterization

2.1 General

This section, consistent with USEPA guidance, presents the site characterization information that supports the scope and selection of an appropriate removal action. Accordingly, this section consists of the following subsections:

- Location and Physical Setting;
- Regional Geology;
- Site History;
- Recently Completed Removal Investigation Activities (including a site-specific summary of geology/ hydrogeology information); and
- Streamlined Risk Evaluation.

Much of the information presented in this section regarding location and physical setting, and site history was obtained from the *Building Demolition Alternative Report* (BBL, April 1998) and the *Soil Sampling Plan* (BBL, April 1998). This section also briefly summarizes previous investigations conducted at the facility including the November 1997 PCB Building Material/Equipment Investigation and the February 1998 soil sampling conducted beneath the concrete floor slab of the manufacturing building. A more detailed discussion of these activities and investigation results is presented in the *Building Demolition Alternative Report*.

This section also presents a description and the results of soil and ground-water sampling conducted at the facility during May 1998, in accordance with requirements set-forth in the *Soil Sampling Plan*, as revised to incorporate comments presented in a May 6, 1998 letter from Ms. Kimberly N. Tisa of the USEPA-Region 1 Office. The information associated with these additional sampling activities has not been previously reported; therefore, a detailed summary of these soil and ground-water sampling activities and analytical results is presented herein (Section 2.5.3).

2.2 Location and Physical Setting

The Aerovox facility is located on an approximately 10 acre parcel at 740 Belleville Avenue in New Bedford, Massachusetts. The location of the site is shown on Figure 2. The facility consists of one three-story building currently used to manufacture capacitors and related products. A parking lot is located south of the manufacturing building. Aerovox and various predecessor companies have occupied the site for over 80 years. During 1995, Aerovox purchased a small parcel located west of the original property (opposite Belleville Avenue) which has been used for additional parking space. The site is located within a highly developed urban/industrial area of New Bedford, Massachusetts. The Acushnet River borders the site to the east. The ground surface at the site slopes gently from the west to the east. The elevation along Belleville Avenue at the West edge of the original property (prior to reaching a seawall constructed along the bank of the Acushnet River) is generally between 4 and 7 feet above MSL.

The Aerovox manufacturing building, shown on Figure 3, encompasses approximately 450,000 square feet and consists of a western section that contains two floors and an eastern section that contains three floors. The exterior walls of the building are brick while the roof is constructed of wood. The first floor in the western section of the

building is estimated to be approximately 6 feet below grade while the first floor in the eastern section of the building is estimated to be approximately $1\frac{1}{2}$ feet below grade. The first floor in both the eastern and western sections of the building is constructed of concrete. Structural components of the building include interior wood columns and steel I-beam floor joists. Wooden floors are present on the second floor of the western section of the building.

2.3 Regional Geology

The site is located in southeastern Massachusetts, near the northern extremity of the Acushnet River estuary, upstream of Apponagansett Bay which opens into the Rhode Island Sound and the Atlantic Ocean. The regional geology is characterized by crystalline bedrock, eroded and contoured by Pleistocene glaciation into a series of low amplitude valleys and ridges. Glaciation is also responsible for the majority of the unconsolidated sediments overlying the bedrock. These glacial deposits range from dense till to highly permeable outwash sand and gravel. A summary of site-specific geology/hydrogeology is presented in Section 2.5.3.2.

2.4 Site History

An investigation of the site was conducted during July and August 1982 pursuant to a Consent Order entered into by Aerovox in May 1982 with the USEPA under Section 106 of CERCLA, 42 U.S.C. 9606. Aerovox also entered into a similar Consent Order with the Massachusetts Department of Environmental Quality Engineering [now known as, and referred to hereafter, as the Massachusetts Department of Environmental Protection (MDEP.)] at the same time. The investigation focused on an unpaved area at the eastern end of the site bordering the Acushnet River and an unpaved strip of land to the north of the manufacturing building. Combined, these areas represent approximately a ½-acre area. The results of the investigation are presented in the *Report of Sampling and Analysis Program at the Aerovox Property, New Bedford, Massachusetts*, prepared by GHR, dated October 7, 1982. The results of the investigation indicated that polychlorinated biphenyls (PCBs) were present in soil at concentrations exceeding 50 parts per million (ppm) and PCBs were also present within the shallow, perched ground-water system at the site.

An evaluation of remedial action alternatives for the Aerovox property was prepared by GHR in accordance with the Consent Orders entered into by Aerovox in May 1982 with the USEPA and the MDEP. The final remedial action alternative selected for the property (as described in an article entitled *On-Site Containment of PCB-Contaminated Soils at Aerovox, Inc., New Bedford, Massachusetts,* prepared by John J. Gushue and Robert S. Cummings) consisted of capping the impacted soil areas (by paving with hydraulic asphalt concrete) and installing a steel sheet pile cutoff wall to serve as a vertical barrier to ground water and tidal flow into and out of the impacted soils. The approximate location of this vertical sheet pile wall is shown on Figure 3. Construction of the final remedial action alternative was started in October 1983 and completed in June 1984. In a letter dated September 21, 1984, the USEPA advised that Aerovox had fully complied with the Consent Order.

An assessment of soil and ground water at and in the vicinity of a former concrete oil containment bunker located south of the manufacturing building boiler room (shown on Figure 3) was conducted during July 1988 by GHR. The assessment was conducted following removal of two 10,000-gallon No. 6 fuel oil storage tanks and one 250-gallon condensate collection tank from the bunker during June and July 1988 by Clean Harbors, Inc. The assessment was conducted pursuant to a request from the MDEP after Aerovox reported that a release of petroleum had occurred at the property. The assessment involved the installation/sampling of soil borings and monitoring wells to determine the extent of petroleum in the vicinity of the former concrete oil containment bunker. An additional assessment of soil and ground water in the vicinity of the former concrete oil containment bunker was conducted during February and March 1989 to provide additional information required by the MDEP.

As required by the MDEP, a short-term measure was implemented at the facility to eliminate (or at a minimum, significantly reduce) the potential for further oil migration by removing the source material from the vicinity of the former concrete oil containment bunker. The short-term measure included the following work: 1) removing petroleum product and water from the concrete oil containment bunker; 2) excavating petroleum-impacted soils for on-site treatment and recycling into an asphalt base course for the parking lot; 3) constructing an oil-water separator to control and recover floating petroleum product; and 4) performing post-construction monitoring of the oil-water separator system to confirm the effectiveness of the short-term measure. Construction activities associated with the short-term measure were completed during November and December 1990. The MDEP determined that no further remedial action was necessary for this matter by a letter dated July 26, 1993.

An inspection of the manufacturing building was conducted by the USEPA during June 1997. As part of that inspection, the USEPA collected wood shaving samples from floor areas inside the manufacturing building and collected oil samples from various oil storage tanks/degreaser operations for PCB analysis. The USEPA data indicated the presence of PCBs in the wood floor samples at concentrations exceeding 50 ppm. PCBs were not detected above laboratory detection limits in the oil samples collected from tanks/equipment at the Aerovox facility. In October 1997, a consultant for Aerovox (East Coast Engineering, Inc.) under USEPA oversight collected wipe samples for PCB analysis. The analytical results indicated the presence of PCBs at concentrations greater than the USEPA-recommended cleanup criteria of 10 micrograms (ug) per 100 square centimeters (cm²) for low- and high-contact interior surfaces as presented in the USEPA PCB Spill Cleanup Policy (40 CFR Part 761.120).

Subsequent to the June 1997 inspection conducted by the USEPA, BBL conducted additional investigation activities to support the USEPA-required removal action at the Aerovox facility. These activities are described in the following section.

2.5 Recently Completed Removal Investigation Activities

The recently completed removal investigation activities completed at the Aerovox facility are as follows:

- PCB Building Material/Equipment Investigation (November 1997);
- Soil Sampling Beneath Concrete Floor Slab (February 1998); and
- Soil and Ground-Water Sampling Activities (May and June 1998).

Presented below is a summary of the November 1997 PCB Building Material/Equipment Investigation and the February 1998 soil sampling conducted beneath the concrete floor slab of the manufacturing building; a more detailed discussion of these activities and investigation results is presented in the *Building Demolition Alternative Report*. Those summaries are followed by a detailed description and the results of soil and ground-water sampling activities conducted at the facility during May 1998, as this information has not been previously reported. A summary of site-specific geology/hydrogeology is also presented in this section.

2.5.1 PCB Building Material/Equipment Investigation

BBL conducted a PCB Building Material/Equipment Investigation in November 1997. The investigation included the additional sampling of building materials/equipment [i.e., full-core building material samples (wood, brick, and concrete), composite scrape samples of dust/dirt from elevated surfaces, wipe samples from non-porous building material surfaces (tile floor, painted walls, steel surfaces), and wipe samples from equipment]. The purpose of the additional sampling of building materials/equipment was to supplement the existing PCB data base, determine the

approximate extent of impacted building materials, develop information regarding the approximate quantities of different building materials, and characterize PCB concentrations on equipment surfaces inside the building.

Table 1 presents the analytical results for each full core sample and each dust/dirt scrape sample along with the sample identification number and building material type (wood, concrete, etc). Table 2 presents the analytical results for each wipe sample collected from non-porous building materials, appurtenances, and equipment inside the building.

The analytical results of full core samples collected during the investigation indicated that PCBs were present at concentrations greater than 50 ppm in samples collected from the following locations:

- The wood floor on the second and third levels of the eastern section of the building;
- The wood floor on the second level in the western section of the building; and
- The concrete floor on the second level in the western section of the building.

PCBs were also detected at concentrations greater than 50 ppm in each of the 12 dust and dirt scrape samples. Seventeen of the 18 wipe samples collected from non-porous building materials and appurtenances (electrical conduits and light fixtures) contained PCBs at concentrations greater than the Toxic Substances Control Act (TSCA) PCB Spill Cleanup Policy cleanup level of 10 ug/100 cm² for high- and low-contact surfaces. Ten of the 13 wipe samples collected from the surfaces of equipment at the Aerovox facility contained PCBs at concentrations greater than 10 ug/100 cm².

2.5.2 Soil Sampling Beneath Concrete Floor Slab

BBL conducted soil sampling activities beneath the concrete floor slab of the manufacturing building during February 1998. The purpose of the soil sampling was to characterize PCB concentrations in soil located directly beneath the concrete floor slab inside the building. Fifteen soil samples were collected from beneath the concrete floor slab at a depth of 0 to 2 inches beneath the concrete slab for PCB analysis. In addition, soil samples were collected at a depth of 2 to 6 inches beneath the concrete floor slab at 14 of the 15 soil sampling locations. The soil samples collected from the 2- to 6-inch depth interval were submitted to the laboratory and archived until the PCB analytical results for the samples from the 0- to 2-inch depth interval were determined.

The analytical results of the soil samples indicate that 5 of the 15 soil samples collected from the 0- to 2-inch depth interval contained PCBs at concentrations greater than 50 ppm. The 2- to 6-inch soil samples collected from two of these 5 soil sampling locations (which were initially archived) were analyzed for PCBs. The analytical results indicate that each of these samples also contained PCBs at concentrations greater than 50 ppm. Table 3 presents the analytical results for each soil sample analyzed. The location of each soil sample along with the associated PCB analytical result is shown on Figure 4.

2.5.3 Soil and Ground-Water Sampling Activities

This section presents a description of the investigation activities completed during May 1998 to characterize the soil and ground water that currently exist at the Aerovox facility. These investigation activities were conducted in support of the removal action and included the following:

• Soil Investigation; and

• Ground-Water Investigation.

Detailed descriptions of these soil and ground-water investigation activities and results, and a summary of site-specific geology/hydrogeology are presented below.

2.5.3.1 Soil Investigation

The soil investigation activities were conducted in accordance with the USEPA-approved *Soil Sampling Plan*, as revised to incorporate comments presented in a May 6, 1998 letter from Ms. Kimberly N. Tisa of the USEPA-Region 1 office.

The soil investigation activities consisted of the following:

- Collecting additional soil samples from beneath the floor of the manufacturing building from two sampling locations which exhibited elevated PCB concentrations during previous investigation activities conducted during February 1998; and
- Completing 17 soil borings in order to collect samples to characterize the soil located beneath the parking lot area outside of the manufacturing building.

Soil samples collected as part of the removal investigation activities were handled, labeled, packaged, and shipped in accordance with the protocols outlined in the *Soil Sampling Plan*. Soil samples selected for laboratory analysis were submitted to Galson Laboratories, Inc. (Galson) for laboratory analysis for polychlorinated biphenyls (PCBs) and/or Target Compound List (TCL) volatile organic compounds (VOCs) using the following methods:

Parameter	Analytical Method
PCBs	USEPA SW-846 Method 8082
VOCs	USEPA SW-846 Method 5035/8260

A detailed discussion of the soil investigation activities is presented below.

Soil Investigation Beneath the Concrete Floor Slab

As detailed in the *Building Demolition Alternative Report* and summarized above, 15 soil samples were previously collected from the 0- to 2-inch depth interval beneath the concrete floor slab of the manufacturing building and submitted for laboratory analysis for PCBs. In addition, soil samples were collected from the 2- to 6-inch depth interval beneath the concrete floor slab and submitted for laboratory analysis for PCBs in soil samples collected from the 2- to 6-inch depth interval beneath the concrete floor slab and submitted for laboratory analysis for PCBs from 14 of the 15 sampling locations. The highest concentrations of PCBs in soil samples collected from beneath the concrete floor slab were detected at sampling locations IB-6 and ID-7 (within the pump room, see Figure 4), where samples from the 0-to 2-inch depth interval contained PCBs at concentrations of 18,000 ppm and 14,000 ppm, respectively. Additional soil investigation activities were conducted in order to further characterize the concentrations of PCBs at the maximum feasible depth beneath the concrete floor slab at sampling locations IB-6 and ID-7. A description of these activities is presented below, followed by a discussion of the associated laboratory results.

Soil Located Beneath the Concrete Floor Slab Sampling Activities

Prior to collecting additional soil samples at soil sampling locations IB-6 and ID-7 (shown on Figure 5), a jackhammer and "Hilti" hammer drill equipped with a pulverizing bit were utilized to remove approximately 4to 5-inches of cement/bentonite grout which was placed over the sampling locations following the previous investigation activities within the manufacturing building conducted during February 1998. Soil samples were collected using a 1¼-inch outer diameter steel casing (e.g. direct push sampling method) equipped with a dedicated polyethylene liner which was retracted from the outer casing at 4-foot intervals in order to retrieve the soil samples. The sampling device was manually driven into the soil using a pneumatic hammer device. The outer steel casing of the sampling device was decontaminated between sampling locations. Due to the presence of compact soil at both soil boring locations (IB-6 and ID-7), refusal of the sampling device was reached at two feet below ground surface for soil sampling location ID-7.

At sampling location IB-6, soil samples were collected from depths of 0.5- to 1-foot and 1- to 2-feet. The soil sample collected from the 0.5- to 1-foot depth interval was placed in a jar and archived for future laboratory analysis, if considered necessary. The soil sample collected from the 1- to 2-foot depth interval was submitted to Galson for laboratory analysis for PCBs using USEPA SW-846 Method 8082. No ground water was encountered while conducting sampling activities at soil boring location IB-6.

At sampling location ID-7, soil samples were collected from depths of 1- to 2-feet, and 3- to 4-feet. No soil sample was retrieved from the 2- to 3-foot depth after the sampling tube liner was destroyed during sampling activities. A soil sample was collected from the 3- to 4-foot depth interval using a 4-foot long inner sampling tube and pushing the tube from the 3- to 4-foot depth. The sample collected from this depth was submitted to Galson for laboratory analysis for PCBs using USEPA SW-846 Method 8082. The soil sample collected from the 1- to 2-foot depth interval was placed in a jar and archived for future laboratory analysis, if considered necessary. Following coring activities, a shovel was used to remove soil to a depth of approximately 1.4 feet below the concrete floor surface. Based on the presence of a noticeable odor, a grab sample was collected at the direction of the USEPA and submitted to Galson for laboratory analysis for TCL VOCs using USEPA SW-846 Method 8260. Because this VOC grab sample was not part of the original scope, it was collected in a glass sampling jar which was not equipped with a teflon lined cap or a septum. Ground water was encountered at sampling location ID-7 at a depth of three feet below ground surface.

Excess soil removed during sampling activities was replaced and a cement/bentonite grout was placed in the sampling locations to restore the floor to the original grade. Detailed field notes describing the activities conducted during the additional investigation of the soil located beneath the floor of the manufacturing building are included as Attachment 2.

Soil Located Beneath the Concrete Floor Slab Sampling Results

Analytical results obtained for the laboratory analysis of soil samples collected from beneath the concrete floor slab within the manufacturing building for PCBs and TCL VOCs are presented below. The discussion includes a comparison of the analytical results obtained from the laboratory analysis of the soil samples with MDEP Soil Category S-3 & GW-3 Standards presented in the Massachusetts Contingency Plan (MCP), 310 CMR 40.0000, effective October 31, 1997.

<u>PCBs</u>

Analytical results obtained for the laboratory analysis of soil samples collected from beneath the concrete floor slab of the manufacturing building for PCBs are listed in Table 4 and shown on Figure 6. Total PCBs were detected in soil samples IB-6 (1-2') and ID-7 (3-4') at concentrations of 4,100 and 2,000 ppm, respectively. Both of these concentrations exceed the MDEP Soil Category S-3 & GW-3 Standards of 2 ppm for PCBs presented in MCP 310 CMR 40,0000.

<u>VOCs</u>

Analytical results obtained for the laboratory analysis of the subsurface soil sample collected from ID-7 for TCL VOCs are listed in Table 5 and shown on Figure 7. Analytical results obtained for the analysis of the soil sample for TCL VOCs are summarized below.

Detected Constituent	Detected Concentration (ppm)	MDEP S-3 & GW-3 Soil Standard (ppm)
Trichloroethylene	30	500
Tetrachloroethylene	1.2	100
1,2,3-Trichlorobenzene	0.7	-
1,2,4-Trichlorobenzene	1.5	800
40.0000,	& GW-3 Standards were obtaine P Soil Category S-3 & GW-3 St tent.	White and a start

The results indicate that the soil sample collected from ID-7 does not contain TCL VOCs at concentrations which exceed the MDEP Soil Category S-3 & GW-3 Standards presented in MCP 310 CMR 40.0000.

Soil Samples Beneath the Parking Lot

A discussion of the activities conducted during the investigation of soil located beneath the parking area outside of the manufacturing building is presented below followed by a discussion of the results of the soil and composite asphalt samples which were collected as a part of the investigation activities.

Boring/Sampling Activities

A total of 16 soil borings (soil borings SB-1 to SB-8 and SB-10 to SB-17) were completed within the area outside of the manufacturing building (see Figure 5) to facilitate the collection of soil samples for analysis of PCBs and TCL VOCs. In addition, based on the request of the USEPA, soil boring location SB-18 (shown on Figure 5) was added to investigate the soil in the vicinity of a PCB-oil fill pipe located along the north side of the manufacturing building. Preliminary sampling locations were chosen systematically by overlaying a 120-foot by 120-foot grid across the parking area south of the building. Utilizing this systematic sampling location scheme, 16 individual grid cells were mapped over the parking area on the site map and preliminary sampling locations were chosen in

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a manner which gave a representative distribution across the parking area. The distances from each soil boring location to at least two prominent physical features at the site were measured and recorded on a field site map, and the physical tie distances were used to create a sample location map to help determine the distribution of the samples within the parking area and identify soil boring locations in the future, if necessary. Soil boring SB-9 was marked on a preliminary sampling location figure; however, the proposed soil boring location was eliminated based on the presence of underground electrical lines. Soil boring SB-17 was added south of the manufacturing building to investigate the soil in the vicinity of a waste trough which formerly conveyed waste material from the facility toward the Acushnet River to the east of the site.

Soil borings were completed by BBL's drilling subcontractor, Environmental Drilling, Inc. (Environmental Drilling) using a the hollow-stem auger drilling method. Soil borings were advanced using a truck-mounted drill rig in accordance with the protocols presented in the Soil Sampling Plan. Continuous soil samples were obtained from each soil boring using a two-foot long, two-inch outer diameter split-spoon sampling device as described in American Society for Testing and Materials (ASTM) Method D-1586/Split Barrel Sampling (Standard Method for *Penetration Test and Split-Barrel Sampling of Soils ASTM D-1586-84*) by driving the split spoon device with a 140-lb hammer dropped 30 inches.

Soil sampling for TCL VOCs was conducted in accordance with the USEPA Region 1 document entitled, Standard Operating Procedure for Soil Sample Collection and Handling for the Analysis of Volatile Organic Compounds (March 1997). Immediately after recovering the split spoon device, one soil sample was collected for TCL VOCs from the most visually stained portion of each two-foot soil sampling interval using an Encore^{π} sampling device. One soil sample collected from each soil boring was submitted to Galson for laboratory analysis for TCL VOCs using USEPA SW-846 Method 5035/8260. Samples collected from the remaining sampling intervals which were not selected for laboratory analysis were archived by the laboratory for future analysis, if considered necessary, A representative portion of each two-foot soil sampling interval was then placed in a screening jar for headspace screening using a photoionization detector (PID). Each two-foot soil sample was then split into one-foot sections and one soil sample was collected (where feasible) from each one-foot section for PCB analysis. At least one sample from each soil boring (more if staining was observed in more than one section of soil recovered from the bore hole) was submitted to Galson for laboratory analysis for PCBs using USEPA SW-846 Method 8082. If no areas of visible staining were observed in a particular soil boring, the PCB sample was submitted from the one-foot section of soil located immediately beneath the asphalt. Samples collected from each one-foot soil segment which were not submitted for laboratory analysis were archived by the laboratory for future analysis, if considered necessary.

Each soil boring was completed to the depth of bedrock or the water table, whichever was encountered first. Upon completion of each soil boring, Environmental Drilling hand shoveled grout into each borehole to the original grade using a cement/bentonite grout mixture (based on the relatively shallow depth of the bore holes, tremie grouting was not considered necessary). Subsurface conditions encountered at each boring location are detailed on the soil boring logs included as Attachment 3, and depicted on geologic cross sections that are presented in the following section.

As part of the soil investigation activities, composite samples of the asphalt pavement from the parking area were collected and submitted for laboratory analysis for PCBs. A total of four composite samples were collected by combining discrete asphalt pavement samples collected at each of the boring locations. Composite samples COMP-1, COMP-2, and COMP-3 were each comprised of discrete samples collected from four borings and composite sample COMP-4 was comprised of two discrete asphalt samples.

Detailed field notes describing these investigation activities are presented in Attachment 4.

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Parking Area Soil Sampling Results

Analytical results obtained for the laboratory analysis of the soil and composite asphalt samples collected during the soil investigation activities for PCBs and TCL VOCs are presented below. The discussion includes a comparison of the analytical results obtained from the laboratory analysis of the soil and asphalt samples with the MDEP Soil Category S-3 & GW-3 Standards presented in MCP 310 CMR 40.0000.

<u>PCBs</u>

PCB analytical results obtained for the laboratory analysis of soil samples are listed in Table 6 and shown on Figure 6. Total PCBs were detected in each soil sample at concentrations ranging from 0.05 ppm in sample SB-3-2 (1-2') to 2,900 ppm in sample SB-7-5 (4-5'). As presented in MCP 310 CMR 40.0000, the MDEP Soil Category S-3 & GW-3 Standard for PCBs is 2 ppm. As indicated in Table 6, this standard was exceeded in 12 samples that were analyzed for PCBs as part of the soil investigation activities.

Analytical results obtained for the laboratory analysis of composite asphalt samples for PCBs are listed in Table 7. The concentrations of PCBs within the composite asphalt samples ranged from 1.13 ppm in COMP-4 to 140 ppm in COMP-2.

<u>VOCs</u>

Analytical results obtained for the laboratory analysis of subsurface samples for TCL VOCs are listed in Table 8 and shown on Figure 7. TCL VOCs were detected at concentrations above laboratory detection limits in soil samples collected at six of the seventeen sampling locations. Analytical results obtained for the laboratory analysis of the subsurface soil samples for TCL VOCs are summarized below.

Detected Constituent	Number of Sampling Locations Where Compound was Detected	Range of Detected Concentrations (ppm)	Sample Exhibiting Maximum Concentration	MDEP S-3 & GW-3 Soil Standard (ppm)
Methylene Chloride	1	0.22	SB-11-2 (0.5-2')	700
Trichloroethylene	4	0.24-0.30	SB-16-2 (0-2')	500
1,2,4- Trichlorobenzene	Ι	0.44	SB-07-5 (4-5')	800
Naphthalene	2	0.33-0.39	SB-05-2 (0-2')	1,000
1,2,3- Trichlorobenzene	1	1.1	SB-07-5 (4-5')	-
Notes: 1. MDEP S-3 & GW-3 Soil Standards were obtained from MCP 310 CMR 40.0000. 2. "-" Indicates that an MDEP S-3 & GW-3 Soil Standard was not listed for that particular constituent.				

The results indicate that none of the soil samples collected during the boring activities contained concentrations of TCL VOCs which exceed the MDEP S-3 & GW-3 Soil Standards for TCL VOCs presented in MCP 310 CMR 40.0000.

2.5.3.2 Ground-Water Investigation

This section presents a summary of information regarding the geology/hydrogeology of the site and a description of a the ground-water investigation activities which were conducted as part of the removal investigation at the Aerovox facility.

Site-Specific Geology

The following summary of the site-specific geology has been prepared based on information generated through previous investigations performed by GHR Engineering Corporation (GHR). This information was presented in the following GHR reports:

- Report of Sampling and Analysis Program at the Aerovox Property, New Bedford, Massachusetts, October 7, 1982;
- Report of Evaluation of Remedial Alternatives for the Aerovox Property, New Bedford, Massachusetts, February 11, 1983;
- Site Assessment Report of Soils and Groundwater in the Vicinity of a Concrete Oil Containment Bunker at the Aerov x Property, New Bedford, Massachusetts, August 23, 1988; and
- Phase I Limited Site Investigation Addendum of Soils and Groundwater in the Vicinity of a Concrete Oil Containment Bunker at the Aerovox Property, New Bedford, Massachusetts, June 30, 1989.

GHR prepared and presented a series of cross sections (A-A' through E-E') illustrating the subsurface geology across the northern and eastern portions of the site (GHR, 1983). Copies of these cross sections, as well as the figure showing the locations of these sections, are presented in Attachment 5 for ease of reference. Site-specific stratigraphic information acquired since 1982 does not change the interpretation of subsurface conditions reflected in the G HR cross sections. Geologic data was also generated through the drilling of 17 soil borings by BBL for the soil investigation activities described in Section 2.5.3.1. To supplement GHR's cross sections, BBL has utilized data from the recently performed soil borings activities to prepare an additional cross section (X-X') beginning in the northwestern corner of the site, continuing across the center of the site, and extending through the parking lot along the southern portion of the site. This cross section is presented as Figure 8. The location of this cross section is illustrated on Figure 5.

As depicted on these cross sections, the sequence of overburden materials encountered below the surface at the site include: a layer of fill; a sand and gravel layer; a peat layer; a fine to medium sand; a medium to coarse sand; and a till. A brief description of these overburden materials follows.

- The heterogeneous backfill materials encountered at the surface across the entire site are composed of sand and gravel with various refuse and construction debris.
- The shallow sand and gravel layer encountered below the fill was a light brown to gray fine to coarse sand and fine to medium gravel characterized as homogeneous, unsorted deposit.

- The layer of peat was consistently encountered between approximately 5 and 10 feet below grade in borings located within the eastern portion of the site, along the Acushnet River. However, this peat layer is laterally discontinuous as it was not observed at boring locations within the western or central portions of the site.
- The deposits of light brown to yellow fine to medium sand as well as the medium to coarse sand were observed primarily below the peat, however, these deposits were also observed to be interbeded within the peat at some locations.
- The clay-rich glacial till was encountered at only a single location (MW-5) in the northwest corner of the site.

Bedrock was encountered at the site during the investigation and removal of the concrete oil containment bunker (see Section 2.4). The bedrock was characterized by GHR (GHR, 1989) as a chlorite gneissic schist, with some high angle fractures parallel to the foliation, and a two to three foot zone of weathering at the bedrock surface. The schist appears as a localized knob or ridge, found as shallow as 1.5 feet below grade near the eastern edge of concrete bunker area, but sloping away to the north and east. Rock was not been observed in any well or boring drilled more than 120 feet from the concrete bunker, except at SB-2 near the western property boundary, at just S feet below grade.

Ground-Water Investigation Activities

Based on the objectives of the removal investigation, ground-water investigation activities were conducted which consisted of the following:

- Assessing the condition at each of the 13 existing ground-water monitoring wells at the facility, including volatile headspace measurement and measuring depth to ground water, total well depth, and the extent of sediment deposition in the well;
- Collecting low-flow ground-water samples for unfiltered PCBs and TCL VOCs analyses from each of the existing ground-water monitoring wells; and
- Obtaining one round of ground-water elevation measurements from each of the 13 existing ground-water monitoring wells over a relatively short period of time, and using this information, as well as previously existing site information, to develop a comprehensive understanding of hydrogeologic conditions at the site.

A detailed description of the activities and results of the ground-water investigation is presented below.

Ground-Water Monitoring Well Assessment and Sampling Activities

The ground-water sampling activities were conducted in accordance with the USEPA document entitled *Low Stress* (low flow) Purging and Sampling Procedure for the Collection of Ground-Water Samples from Monitoring Wells Revision 2, dated June 30, 1996. Prior to sampling each ground-water monitoring well, monitoring well assessment activities were conducted which included probing each well to determine the presence and depth (if any) of sediment within the well, measuring headspace concentrations of VOCs using a PID, measuring the depth to water, and determining the total depth of the well. Based on these inspection activities, small amounts of sediment were found at the bottom of eight out of the thirteen existing on-site monitoring wells. Measurable headspace VOC concentrations were not obtained at any of the thirteen existing ground-water monitoring wells. Field notes

summarizing the conditions observed during the monitoring well assessment activities are presented as Attachment 6.

Following these inspection activities, a low flow submersible pump with polyethylene tubing was placed within the well and ground water was purged from the well until indicator field parameters were stabilized within the ranges presented in the above-referenced USEPA document (indicator field parameters included turbidity, dissolved oxygen, specific conductance, temperature, pH, and oxidation/reduction potential). Ground-water samples collected as part of the removal investigation activities were submitted to Galson for laboratory analysis for PCBs (using USEPA SW-846 Method 8082) and TCL VOCs (using USEPA SW-846 Method 8260). In addition, three trip blank samples (one for each day of sampling) and one rinse blank sample were collected for quality assurance/quality control (QA/QC) purposes.

Ground-water monitoring well MW-4A was pumped dry during purging activities conducted on May 27, 1998 at approximately 9:30 a.m. A ground-water sample was collected the following morning at approximately 6:30 a.m. after the well had recharged just enough to collect the ground-water samples. Detailed ground-water well sampling logs summarizing the field parameters measured during ground-water sampling activities are included as Attachment 7. Detailed field notes describing the ground-water investigation field activities are presented in Attachment 8.

Ground-Water Sampling Results

Analytical results obtained for the laboratory analysis of ground-water samples collected during the ground-water investigation activities for PCBs and TCL VOCs are presented below. The discussion includes a comparison of the analytical results obtained for the laboratory analysis of the ground-water samples with MDEP Ground-Water Category GW-3 Standards presented in MCP 310 CMR 40.0000.

<u>PCBs</u>

Analytical results obtained for the laboratory analysis of ground-water samples for PCBs are listed in Table 9 and shown on Figure 9. Total PCBs were detected in four of the thirteen ground-water samples collected during the ground-water investigation at concentrations ranging from 3 ppb in sample MW-8S to 36 ppb in sample MW-4A. As indicated in MCP 310 CMR 40.0000, the MDEP Ground-Water Category GW-3 Standard for PCBs is 0.3 ppb. As indicated in Table 9, this standard is exceeded in all four of the ground-water samples in which PCBs were detected. In addition, analytical detection limits for several of the ground-water samples collected at the facility were elevated due to matrix interference (due to siltation, salinity, hydrocarbon interferences, etc.).

<u>VOCs</u>

Analytical results obtained for the laboratory analysis of ground-water samples for TCL VOCs are listed in Table 10 and shown on Figure 10. TCL VOCs were detected at concentrations above laboratory detection limits in samples collected at 12 of the 13 sampling locations. Analytical results obtained for the laboratory analysis of the ground-water samples for TCL VOCs are summarized below.

Detected Constituent	Number of Sampling Locations Where Compound was Detected	Range of Detected Concentrations (ppb)	Sample Exhibiting Maximum Concentration	MDEP GW-3 Ground-Water Standard (ppb)
Vinyl Chloride	4	76-520	MW-7	40,000
cis-1,2- Dichloroethylene	6	29-2,900	MW-7	50,000
1,1-Dichloroethylene	1	37	MW-4B	50,000
Methylene Chloride	1	12 B	MW-4B	50,000
1,1-Dichloroethane	1	9	MW-4B	50,000
Chloroform	1	9	MW-4B	10,000
1,1,1-Trichloroethane	1	41	MW-4B	50,000
Benzene	2	35-60	MW-3A	7,000
Trichloroethylene	2	3,600-8,900	MW-7	20,000
Tetrachloroethylene	2	17-33	MW-4B	5,000
Chlorobenzene	5	19-1,000	MW-3A	500
Ethylbenzene	2	95-150	MW-3	4,000
1,3-Dichlorobenzene	1	150	MW-2	8,000
1,4-Dichlorobenzene	4	7-220	MW-2	8,000
1,2,4- Trichlorobenzene	1	5	MW-4B	500
Naphthalene Notes:	1	18	MW-2A	6,000

The results indicate that Chlorobenzene was detected in ground-water samples collected from monitoring wells MW-2 (570 ppb) and MW-3A (1,000 ppb) at concentrations which exceeded the MDEP Ground-Water Category GW-3 Standard of 500 ppb as presented in MCP 310 CMR 40.0000.

Ground-Water Elevations and Hydrogeologic Characterization

Ground water was encountered under water table conditions across the site at depths ranging from approximately 3.5 below grade near the river to nearly 12 feet below grade at the western edge of the site. Along the eastern portion of the site ground water was also observed to exist perched above the fines-rich peat layer. Water level

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measurements obtained from the 13 existing wells at the site on May 21, 1998 (provided in Table 11) were used to generate the ground-water potentiometric surface contour maps illustrating the hydraulic gradient across the site within the deeper water-bearing unit as well as the shallow/perched water-bearing unit. These maps are presented as Figures 11 and 12, respectively.

Ground-water level data have also been recorded from select monitoring wells at this site on a regular basis by SAIC Engineering, Inc. (SAIC), as part of the Site Post-Closure Monitoring Program associated with the site remediation activities completed in 1984. As discussed in Section 2.4 and the previously mentioned article entitled *On-Site Containment of PCB-Contaminated Soils at Aerovox, Inc., New Bedford, Massachusetts*, those remediation activities included installation of a vertical sheet pile wall to serve as a barrier to ground water and tidal flow into and out of the impacted soils located at the eastern end of the site. The sheet piling cutoff wall is from 9 to 13 feet in depth, the actual depth is dictated by the depth to the peat layer into which the wall is keyed. The wall has been installed along the eastern boundary of the property. In the area directly behind the manufacturing building, the sheet pile wall extends west up to the building foundation; thereby, forming a containment cell with the building foundation serving as the fourth side of this cell. The approximate location of the sheet pile wall is shown on Figure 3.

The Site Post-Closure Program includes obtaining periodic high and low tide water level measurements from a tide gauge and from the eight monitoring wells located at the eastern end of the site (MW-2, MW-2A, MW-3, MW-3A, MW-4, MW-4A, MW-7, and MW-7A). The water level measurements obtained by SAIC during the past three years are provided as Attachment 10. After reviewing this data set, representative water level data obtained during both high-tide and low-tide periods within the shallow and deep wells (provided in Table 12) were used to prepare the ground-water potentiometric contour maps presented as Figures 13 through 16.

The observed hydraulic gradients indicate the direction of ground-water flow would generally be from west to east, in the direction of the river. The deep water-bearing zone appears to respond to high-tide periods with a temporary reversal in the hydraulic gradient in the immediate vicinity of the Acushnet River.

The perched ground-water bearing zone appears to be isolated from hydraulic interaction with the adjacent river to some degree by the presence of the vertical sheet pile wall installed along the river and in the eastern corner of the site to form a containment cell (see Figure 3). A review of water level monitoring data recorded by SAIC over the past several years (provided as Attachment 10) indicate that the ground water within this perched water-bearing unit does not appear to respond to tidal fluctuations in the river, as observed in the deeper monitoring wells within this portion of the site. A review of the water level data at well clusters within the area of the site observed to have a perched water table indicate that downward vertical gradients exist consistently during both high and low tide periods.

2.6 Streamlined Risk Evaluation

2.6.1 Introduction

Consistent with USEPA guidance, the streamlined risk evaluation presented in this section focuses on those risk issues that the EE/CA removal action is intended to address and provides justification for the removal action. This streamlined risk evaluation addresses both soil and ground water, as well as the building at the facility.

2.6.2 Soil and Ground Water

At this facility, the applicable category of soil is S-3 Soils, and the applicable category of ground water is GW-3 Ground Water. These categories have been established by the MDEP for use in characterization of risk posed by a site. The categories are used to determine the applicability of the soil and ground-water standards listed and described in the MCP, 310 CMR 40.0000, issued by the MDEP Bureau of Waste Site Cleanup, effective October 31, 1997. The categories are also considered when determining the appropriate removal action alternative to be implemented at the site.

The soil at the site has been categorized as S-3 Soils based on the criteria listed in Section 40.0933 of the MCP. Site, receptor, and exposure information identified in Sections 40.0904 - 40.0929 of the MCP, in conjunction with current and potential future site activities and uses, were also used to categorize the soil. Category S-3 Soils are appropriate because soil at the facility is essentially inaccessible (i.e., covered with asphalt pavement or concrete), children are not present at the facility, and the frequency and intensity of exposure to the soil by adults is low.

The ground water at the site has been categorized as GW-3 Ground Water based on the criteria listed in section 40.0932 of the MCP. Category GW-3 Ground Water, while considered a potential source of discharge to surface water, represents the minimum-risk ground-water category. The ground water at the site has not been additionally categorized as GW-1 or GW-2 because it is not located within either a current or potential drinking water source area and the building will be demolished as part of the removal action. Therefore, as set forth in the MCP, the total PCB cleanup standard is 0.3 ppb for the GW-3 Ground-Water samples collected from the site.

The MCP Risk Characterization Method I was utilized at the site through the use of promulgated standards described in Sections 40.0970 - 40.0979 of the MCP. Method I relies upon the use of the numerical standards given above for chemicals in ground water and soil to accurately characterize the risk posed by the site. The potential risks posed by the soil and ground water at the facility are characterized by comparing detected concentrations to their respective Method I Standard.

As outlined in Section 40.0975 of the MCP, "the MCP Method 1 Soil Standards consider both the potential risk of harm resulting from direct exposure to the oil and/or hazardous material in the soil and the potential impacts on the ground water at the disposal site. The applicability of a specific numerical Standard is thus a function of both the soil and the ground-water category identified." Therefore, the Soil Category S-3 Standards for the combination of soil and ground-water categories are S-3 and GW-3, respectively, are given in Table 4 in Section 40.0975 of the MCP. These soil standards are identified in Tables 4 through 8 which present the soil analytical data associated with the recent investigation activities conducted at the facility. Ground-Water Category GW-3 Standards are identified in Tables 9 and 10 which present the recent ground-water analytical results. Detected concentrations exceeding Standards have been shaded in these tables.

As shown in these tables, PCBs are the only constituents detected in the soil samples at concentrations in excess of their respective Soil Category S-3 & GW-3 Standard (2 ppm); and PCBs and chlorobenzene are the only constituents detected in the ground-water samples at concentrations in excess of Standards. PCBs were detected in excess of the Category GW-3 Standard of 0.3 ppb in 4 of the 13 samples collected, at a maximum concentration of only 36 ppb. The only other constituent detected in the ground-water samples at concentrations in excess of the Standard was chlorobenzene, which was detected in only 2 out of the 13 ground-water samples. The Category GW-3 Standard for chlorobenzene is 500 ppb. The ground-water samples collected from MW-2 and MW-3A contained chlorobenzene at 570 ppb and 1,000 ppb, respectively. These monitoring wells, however, are located in the eastern portion of the property, within the area addressed by the remedial action completed in 1984, and not subject to this

removal action. That remedial action was completed in compliance with a 1982 Consent Order entered into by Aerovox with the USEPA (September 21, 1984 letter from the USEPA).

Thus, PCBs in soils represent the only constituents of interest in environmental media at the facility. Because concentrations of PCBs at the site considerably exceed Standards in a number of soil sampling locations both beneath the building and the parking lot, implementation of a PCB removal action is appropriate to mitigate potential exposure and migration pathways.

2.6.3 Building Materials

The results of the PCB Building Material/Equipment Investigation conducted by BBL on November 24 and 25, 1997 are presented in Section 2 of the *Building Demolition Alternative Report*. These analytical results are summarized below.

- The wood floor on the second and third floors of the eastern section of the building contains PCBs at concentrations greater than 50 ppm.
- Two of the three wood floor full core samples collected from the second floor in the western section of the building contained PCBs at concentrations greater than 50 ppm.
- One of the two concrete floor full core samples collected from the second floor in the western section of the building contained PCBs at concentrations greater than 50 ppm.
- The PCB concentrations in all of the full core dust and dirt scrape samples ranged from 2.48 ppm to as high as 56,000 ppm.
- PCBs were detected in each of the 12 dust and dirt scrape samples at concentrations greater than 50 ppm.
- 17 of the 18 wipe samples collected from non-porous building materials and appurtenances contained PCBs at concentrations greater than 10 ug/100cm², which is the TSCA PCB Spill Policy cleanup objective for low- and high-contact interior surfaces.
- 10 of the 13 wipe samples collected from the surfaces of building equipment contained PCBs at concentrations greater than 10 ug/100 cm². The PCB concentrations in all of the wipe samples ranged from 2.5 ug/100 cm² to 520 ug/100 cm².

Based on these data these data, PCB concentrations at many different sampling locations within the Aerovox facility exceeded 50 ppm within building materials and 10 ug/100 cm² on the surfaces of building materials. Accordingly, demolition of the building is an appropriate removal action to mitigate potential exposure and migration pathways.

3. Potentially Applicable or Relevant and Appropriate Requirements (ARARs)

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3. Potentially Applicable or Relevant and Appropriate Requirements (ARARs)

This section presents a list of potential ARARs under federal and Massachusetts environmental laws. The purpose of this list is to present each potential ARAR identified and define its applicability to the removal action for this facility.

In accordance with the NCP, removal actions taken pursuant to Section 106 of CERCLA must, to the extent practicable considering the exigencies of the situation, attain ARARs under federal environmental or state environmental or facility siting laws [40 CFR 300.415(j)]. ARARs are state and federal human health and environmental regulations and statutes generally used to evaluate the appropriate extent of site cleanup, formulate and scope removal action alternatives, and govern the implementation and operation of a selected removal action alternative.

For a regulation or statute to be considered an ARAR, it must be substantive and not administrative, formally promulgated by the effective date of the decision document by a federal or state agency, and of general applicability and legally enforceable. If they are legally enforceable statewide, state requirements may also be considered ARARs. However, only state requirements that are promulgated, more stringent than federal requirements, and identified by the state in a timely manner may be considered ARARs [40 CFR 300.400(g)(4)]. The NCP defines two types of ARARs:

- Applicable Requirements: Cleanup standards, standards of control and other substantive requirements, criteria, or limitations promulgated under federal or state environmental laws that specifically address a hazardous substance, pollutant, contaminant, response action, location, or other circumstance found at the CERCLA site (40 CFR 300.5). These include federal requirements that are directly applicable as well as those incorporated by a federally authorized state program. Only those state standards that are identified by the state in a timely manner and that are more stringent than the federal requirements may be applicable.
- Relevant and Appropriate Requirements: Promulgated cleanup standards, standards of control, and other substantive requirements, criteria, or limitations that, while not applicable to a hazardous substance, pollutant, contaminant, response action, or other circumstance at the CERCLA site, address problems or situations sufficiently similar to those encountered at the site so that their use is well suited to the particular site (*ibid*). To fall within this category, the requirements must be both relevant and appropriate to the site-specific circumstances. Factors considered in the determination of the relevance and appropriateness of a requirement are presented in 40 CFR 300.400(g)(2).

In addition, to applicable or relevant and appropriate requirements, other advisories, criteria, or guidance may be considered, as appropriate. The "to be considered" (TBC) category consists of advisories, criteria, or guidance that have been developed by the USEPA, other federal agencies, or states that may be useful in developing CERCLA remedies [40 CFR 300.400(g)(3)].

Removal actions under Section 106 of CERCLA must attain ARARs only to the extent practicable considering the exigencies of the situation [40 CFR 300.415(j)]. In determining whether compliance with an ARAR is practicable, the lead agency may consider all appropriate factors including: 1) the urgency of the situation; and 2) the scope of the removal action [40 CFR 300.415(j)(1) and (2)]. Even if compliance with an ARAR is deemed practicable based on the consideration of the above factors, compliance may nevertheless be waived under any of the circumstances for which CERCLA allows a waiver for remedial actions [see Section 121(d)(4) of CERCLA; 40 CFR 300.430(f)(1)(ii)(C)].

The identified potential ARARs that pertain to the removal action at this facility are listed in Tables 13, 14a, and 14b:

- Table 13 summarizes the potential chemical-specific ARARs. Chemical-Specific ARARs are health or riskbased numeric values or methodologies that establish the acceptable amount or concentration of a chemical that may be found in or discharged to the ambient environment. These ARARs govern the extent of site remediation by providing either actual cleanup concentrations or the basis for the calculation of such concentrations. These ARARs may also be used to indicate the acceptable concentrations of discharge, in determining treatment and disposal requirements, and to assess the effectiveness of future remedial alternatives;
- Table 14a summarizes the potential action-specific ARARs. Action-Specific ARARs are technology- or activity-based requirements or limitations on actions involving the management of hazardous substances, pollutants, or contaminants. These ARARs often set controls or restrictions on the design, implementation, and/or performance of the removal actions. These ARARs also provide a basis for assessing the feasibility and effectiveness of various proposed alternatives by specifying performance requirements and limitations, actions or technologies, and/or specific discharge or residual concentrations; and
- Table 14b summarizes the potential location-specific ARARs. Location-specific ARARs are restrictions placed on the concentration of hazardous substances or the conduct of activities solely because they occur in specific locations.

These tables identify each ARAR, outline its requirements, define its applicability or appropriateness, and include how the ARAR will be attained by the removal action at the facility. ARARs are state and federal human health and environmental regulations and statutes and are only identified for work activities that occur on-site. Occupational safety and health protection standards under the Occupational Safety and Health Act (OSHA) were not considered to be environmental standards; however, applicable OSHA standards, as well as other applicable non-environmental regulations, will be met during implementation of the removal action.

Finally, the Commonwealth has noted that the remedy calls for leaving material behind which exceeds the State's upper concentration limit of 100 ppm PCBs in soil. As a result, the Massachusetts Contingency Plan, Class A-4 Response Action Outcome requires an engineered barrier as cover for those soils. An engineered barrier in accordance with the Massachusetts Hazardous Waste Management Closure Requirements, identified in ARARs Table 14a, will be part of the removal action.

4. Identification of Removal Action Scope, Goals, and Objectives

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4. Identification of Removal Action Scope, Goals, and Objectives

4.1 General

This section presents the goals and objectives for conducting a removal action at the Aerovox facility.

4.2 Statutory Limits on Superfund-Financed Non-Time Critical Removal Actions

Section 300.415(b)(2) of the NCP lists eight factors for the USEPA to consider in determining if a removal action is appropriate at a particular site. One factor applicable to this facility includes the actual or potential exposure to nearby human populations, animals, or the food chain from hazardous substances, pollutants, or contaminants. Accordingly, site conditions meet the criteria listed in the NCP and provide appropriate justification for the decision to implement a removal action at the Aerovox facility. This removal action will be non-time critical because more than six months planning time is available before on-site activities must be initiated.

In the event that this EE/CA must be undertaken by the USEPA rather than the potentially responsible parties, there are certain statutory and regulatory requirements that must be addressed. In particular, as stated in 40 CFR 300.415(b)(5), "Fund-financed removal actions, other than those authorized under Section 104(b) of CERCLA, shall be terminated after \$2 million has been obligated for the action or 12 months have elapsed from the date that the removal activities begin on site" unless the lead agency grants an exemption in accordance with the criteria set forth in CERCLA Section 104(c)(1).

The criteria set forth in 40 CFR 300.415(b)(5) include two exemptions for the \$2 million and 12 month statutory limits. They are the "emergency" waiver and the "consistency" waiver. The "emergency" waiver allows for actions to exceed the statutory limit if there is an immediate risk to public health or welfare, or the environment, and continued response actions are immediately required to prevent, limit, or mitigate an emergency and such actions would not otherwise be provided on a timely basis. The "consistency" waiver allows for the action to continue if the removal action is otherwise appropriate and consistent with the anticipated future use of the site.

As discussed in Sections 5 and 6, the alternatives evaluated by this EE/CA would, if implemented, exceed the \$2 million and one year statutory limits applicable to USEPA fund-lead removal actions. If USEPA were to be required to perform the removal action using Superfund money, a consistency waiver would likely be sought on the grounds that the removal action is appropriate and consistent with anticipated future use of the site.

4.3 Removal Action Objectives

The general removal action goals for the site are to minimize future potential impacts to human health and the environment caused by the presence of PCBs in the manufacturing building materials/equipment and site soils. Based on this general removal action goal, the following specific removal action objectives have been developed:

- 1. Demolish the manufacturing building in a manner, to the extent practicable, that is both in compliance with applicable ARARs and cost effective; and
- 2. Prevent future direct contact with site soils containing PCBs at concentrations greater than 2 ppm through the installation of a low-permeability cap that will facilitate future reuse of the property.

5. Identification and Analysis of Removal Action Alternatives

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5. Identification and Analysis of Removal Action Alternatives

5.1 General

This section presents detailed descriptions of three removal action alternatives developed to achieve the removal action objectives presented in Section 4.3. Descriptions of the criteria outlined in the EE/CA guidance document (USEPA, 1993) are also presented below.

5.2 Description of Evaluation Criteria

Removal action alternatives are evaluated against the short- and long-term aspects of three broad criteria presented in the CERCLA Guidance document: effectiveness, implementability, and cost. Subcriteria to be evaluated under each of these criteria are identified and discussed below.

5.2.1 Effectiveness

The effectiveness of an alternative refers to it's ability to meet the objective within the scope of the removal action. Each alternative is evaluated against the scope of the removal action and against each specific objective for final disposition of the wastes and the level of cleanup desired. The following subcriteria will be evaluated under this criterion.

<u>Overall Protection of Public Health and the Environment</u> - How the alternative, as a whole, protects human health and the environment and will reduce, control or eliminate risks at the site through the use of treatment, engineering, or institutional controls. This evaluation will also identify any unacceptable short-term impacts associated with the alternative.

<u>Compliance with ARARs</u> - How the alternative complies with the chemical, local, and action specific ARARs, or other advisories and guidance. The applicable requirements associated with each alternative will be identified, and it will be determined how (or if) the alternative meets the applicable requirements.

<u>Long-Term Effectiveness and Permanence</u> - Assesses the extent and effectiveness of the controls that may be required to manage the risk posed by treatment residuals and/or untreated wastes at the site. The following components will be considered for each alternative:

Magnitude of Risk - Assesses the risk from waste and residuals remaining at the conclusion of site activities. Also evaluates whether the alternative contributes to future remedial objectives.

Adequacy and Reliability of Controls - A completed removal action may require post-removal site controls (PRSC) to sustain the integrity of a removal action following its conclusion.

<u>Reduction of Toxicity, Mobility, or Volume through Treatment</u> - Evaluate the treatment technologies used by the degree of expected reduction in toxicity, mobility, or volume of hazardous material. This criterion also evaluates the irreversibility of the treatment process and the type and quantity of residuals remaining after treatment.

<u>Short-Term Effectiveness</u> - Addresses the effects of the alternative during implementation before the removal objectives have been met. The following factors will be addressed as appropriate for each alternative.

Protection of the Community - Addresses any risk to the affected community that results from implementation of the proposed action, whether from air quality, fugitive dust, transportation of hazardous materials, or other sources.

Protection of the Workers - Assesses any threats to site workers and the effectiveness and reliability of protective measures that would be taken.

Environmental Impacts - evaluates the potential adverse environmental impacts from the implementation of each alternative. Also assesses the reliability of mitigation measures in preventing or reducing the potential impacts.

Time Until Response Objectives are Achieved - Estimates the time needed to achieve protection for the site itself or for individual elements or threats associated with the site.

5.2.2 Implementability

The implementability of an alternative refers to the ability to construct and operate the technology; the reliability of the technology; the ease of undertaking additional remedial actions; and the ability to monitor the effectiveness of the remedy. The following factors will be considered under this criterion.

<u>Technical Feasibility</u> - The ability and reliability of the technology to implement the remedy. Each alternative will be evaluated for implementation factors such as assembling, staffing, and operating the alternative within the time frames in the removal schedule. Each alternative will also be evaluated for technology maturity, prior use under similar conditions for similar wastes, and possible difficulty in operation once it is constructed. This evaluation will also take into consideration environmental conditions, potential future remedial actions, and the ability to monitor the effectiveness of the alternative.

<u>Administrative Feasibility</u> - Evaluate those activities needed to coordinate with other offices and agencies. The administrative feasibility of each alternative should be evaluated including the need for permits, adherence to applicable non-environmental laws, and concerns of other regulatory agencies. Factors that will be considered include statutory limits and required permits and waivers.

<u>Availability of Services and Materials</u> - Evaluate whether off-site treatment, storage and disposal capacity, equipment, personnel, services and materials, and other resources necessary to implement an alternative will be available in time to maintain the removal schedule.

<u>State Acceptance</u> - Evaluates the technical and administrative concerns the State may have regarding a removal alternative. This will be addressed once the State's comments on the EE/CA have been received.

<u>Community Acceptance</u> - Evaluates the issues and concerns the public may have regarding a removal alternative. This will be addressed once the public's comments on the EE/CA have been received.

5.2.3 Cost

Each removal action alternative will be evaluated to determine its projected costs. Each alternative's capital and PRSC costs will be compared. The present worth of alternatives that will last longer than 12 months will be calculated. To compare the cost of each alternative, the direct and indirect capital costs and PRSC costs of each alternative will be projected. The following items are examples of direct and indirect capital costs and PRSC costs:

Direct Capital Costs

- construction costs
- · equipment and material costs
- transport and disposal costs
- treatment and operating costs

Indirect Capital Costs

- · engineering and design costs
- legal fees and license or permit costs
- start-up costs

PRSC Costs

- operational costs
- maintenance costs
- monitoring costs
- support costs

5.3 Identification of Removal Action Alternatives

Under each of the removal action alternatives presented in this EE/CA, the manufacturing building at the Aerovox facility would be demolished and the site would be restored by installing an impermeable liner and an asphalt cap following placement of backfill materials at the former location of the building. Each of the removal action alternatives would consist of the seven major work activities listed below.

- Work Activity 1 Additional Building Characterization;
- Work Activity 2 Equipment/Appurtenances Inventory;
- Work Activity 3 Pre-Demolition Cleaning;
- Work Activity 4 Post-Cleaning Verification Sampling;
- Work Activity 5 Utility Modifications and Removal;
- · Work Activity 6 Building Demolition and Disposal; and
- Work Activity 7 Site Restoration/Asphalt Cap Construction.

Each of these work activities is discussed below.

Work Activity 1 - Additional Building Characterization

Prior to implementing building demolition activities, additional sampling would be conducted to confirm that the brick walls in the pump room located on the first floor and the brick walls in the impregnation room (tank room) located on the second floor directly above the pump room do not contain PCBs at concentrations greater than or equal to 50 ppm. The additional sampling work would involve collecting an appropriate number of discrete core samples from the brick walls in these two rooms (i.e., six samples) for laboratory analysis for PCBs.

If the analytical results of the core samples indicate that PCBs are present at concentrations less than 50 ppm, the brick walls would be handled with other non-TSCA demolition debris. However, if the analytical results of the core

samples indicate that PCBs are present at concentrations greater than or equal to 50 ppm, the brick walls would require disposal at a TSCA landfill.

Work Activity 2 - Equipment/Appurtenances Inventory

Under this work activity, a detailed inventory of equipment/appurtenances at the facility (both inside and outside the building) would be developed. In addition to listing equipment/appurtenances, the inventory would identify which equipment/appurtenances would be transferred from the facility and returned to commerce at a proposed new facility, which equipment/appurtenances would be offered for sale, and which equipment/appurtenances would be scrapped. In order to develop the inventory, the following work would be conducted:

- A site reconnaissance to identify each piece of equipment/appurtenance in its current location, record applicable information from manufacturer's plates on the equipment/appurtenances, and assess the condition of the equipment/appurtenances; and
- A review of applicable records pertaining to each piece of equipment (if available) and coordination with engineering/operations personnel at the facility. The review/coordination work would be conducted in an effort to identify the age and repair history of the equipment/appurtenances, to estimate the market value for the equipment/appurtenances, and to determine the role (if any) for the equipment/appurtenances in future manufacturing operations.
- Acrovox would be responsible for determining which equipment/appurtenances would be retained for future use at a new manufacturing location, which equipment/appurtenances would be offered for sale, and which equipment/ appurtenances would be scrapped.

Work Activity 3 - Pre-Demolition Cleaning

This work activity would consist of washing interior horizontal surfaces with detergent to remove PCB-containing dust and dirt in order to facilitate general demolition of the building. The pre-demolition cleaning would involve the cleaning of the steel I-beams, HVAC duct work, and other metal surfaces to reduce PCB concentrations to less than 100 ug/100 cm² in order to allow for the removal and disposal of the material at a steel smelting facility.

As part of the pre-demolition cleaning activities, equipment surfaces containing PCBs at concentrations greater than or equal to $10 \text{ ug}/100 \text{ cm}^2$ would require cleaning prior to transferring the equipment off-site.

Based on the presence of vinyl floor tile, pipe insulation materials, and boiler insulation materials within the building that may potentially contain asbestos, an asbestos survey will be conducted to determine if asbestos abatement is required prior to building demolition. For the purpose of this report we have assumed that these materials contain asbestos and would be removed as part of the pre-demolition cleaning activities.

Work Activity 4 - Post-Cleaning Verification Sampling

Following completion of the pre-demolition cleaning activities, a visual inspection will be conducted to confirm that visible dust and dirt has been removed followed by a post-cleaning verification wipe sampling program to:

• Confirm that metal surfaces scheduled for smelting do not contain PCBs at concentrations greater than or equal to 100 ug/100 cm²; and

• Confirm that equipment surfaces scheduled for reuse do not contain PCBs at concentrations greater than or equal to 10 ug/100 cm².

Work Activity 5 - Utility Modifications and Removal

Upon completion of the post-cleaning verification sampling activities, modifications to existing utilities and removal of interior utilities would occur. The utility modifications would include the following:

- Disconnection and plugging of sanitary sewer piping and any additional drain piping;
- Disconnection of the existing potable water supply; and
- Disconnection of electrical services.

The following utility removal actions would also be conducted:

- Removal of electrical equipment, boilers, and compressors;
- Removal of light fixtures (fluorescent light ballasts may contain PCBs);
- · Removal of fire protection and potable water piping; and
- Removal of HVAC system components (excluding steel duct work).

Work Activity 6 - Building Demolition and Disposal

As part of this work activity, the building would be demolished and concrete/brick debris generated by demolition of the building which does not contain PCBs at concentrations greater than or equal to 50 ppm would either be transported for off-site disposal or used as backfill on-site depending on which of the following removal action alternatives is selected: 1) leave the first floor concrete slab in-place; 2) remove a portion of the first floor concrete slab; or 3) remove the entire first floor concrete slab (details associated with the demolition work to be conducted under each of these alternatives are presented below). Materials within the building which do not contain PCBs at concentrations greater than or equal to 50 ppm have been identified based on the analytical results for samples previously collected. The actual amount of building materials which do not contain PCBs at concentrations greater than or equal to 50 ppm may decrease (resulting in an increase in TSCA-regulated building materials) depending on the results of additional sampling that will be conducted prior to the building demolition within the pump room and the tank room.

The demolition Contractor will be required to comply with a set of special conditions specific to project. The special conditions will include, but not be limited to, the following plans and procedures:

- Air monitoring procedures;
- Dust control procedures;
- Surface water control procedures;
- · Equipment decontamination procedures;
- Waste Handling Plan;
- Health and Safety Plan; and
- Contingency plans.

A set of the special conditions will be provided to the USEPA prior to implementing the demolition activities. A description of the work to be conducted by the Contractor under removal action alternatives 1 through 3 are presented below in Sections 5.3.1 through 5.3.3.

Work Activity 7 - Site Restoration/Asphalt Cap Construction

Under this work activity, a capping system would be constructed over the entire facility, including the area where the building was located following the placement and compaction of backfill over the area. The capping system would be constructed in accordance with the precedent that was established for remediation of PCB-impacted soils located outside the building footprint (to the north and east of the building). The capping system may consist of the following materials (referenced, in order, from the surface to the base of the capping system):

- A 1¹/₂-inch thick bituminous concrete wearing surface over a 2¹/₂-inch thick bituminous concrete base course;
- An 8-inch subbase course to provide bearing support for vehicles which will be parked on the bituminous concrete surface. The subbase course would consist of approximately 6 inches of run-of-crush stone over approximately 2 inches of sand. The sand would serve as a protective barrier to help prevent the underlying materials from being damaged during placement of the run-of-crush; and
- A geosynthetic drainage composite overlying a 40 mil impermeable polyvinyl chloride (PVC) or high-density polyethylene (HDPE) membrane. The purpose of the geosynthetic composite would be to convey water (which may penetrate the bituminous concrete surface and would otherwise be trapped above the impermeable PVC or HDPE membrane) away from the capping system in an effort to prevent premature failure of the bituminous concrete resulting from frost action.

The capping system described above was developed for the purposes of preparing a cost estimate. The details of the final cap system for the facility will be selected during the design phase based, in part, on the site conditions and future reuse of the property.

5.3.1 Alternative 1 - Leave the First Floor Concrete Slab In-Place

Under this alternative, the wood and concrete floors that contain PCBs at concentrations greater than or equal to 50 ppm (excluding the first floor concrete slab) would be removed from the building and transported for off-site disposal at a TSCA landfill permitted to accept debris containing PCBs at concentrations greater than or equal to 50 ppm. Based on a preliminary review of the building, BBL has assumed that the wood and concrete floors could be removed (prior to demolition of the entire building) without jeopardizing the structural integrity of the building. However, before preparing a Contractor scope of work for the building demolition, a more comprehensive structural review of the building will be conducted by a Licensed Professional Engineer experienced in performing structural integrity of the building shell prior to general demolition activities. The Engineer will also provide recommendations for temporary structural support that may be needed during the floor removal activities.

Following removal of the wood and concrete floors that contain PCBs at concentrations greater than or equal to 50 ppm, the building would be demolished using traditional demolition techniques (i.e., a wrecking ball, excavators). Dust control measures will be implemented to minimize dust levels generated by the demolition work. The actual techniques/methods to be employed will be recommended by the demolition Contractor and reviewed and approved by the Engineer. The selected Contractor would be required to furnish details regarding demolition techniques/methods and the locations of debris staging/loading areas.

Debris (concrete, wood, brick) which does not contain PCBs at concentrations greater than or equal to 50 ppm would be transported for off-site disposal at a non-TSCA landfill permitted to accept the debris. Steel building components and associated metal materials generated during the demolition activities which do not contain PCBs

on the surfaces at concentrations greater than or equal to 100 ug/100cm² (as determined by verification sampling conducted under Work Activity 4) would be segregated and transported off-site for smelting. We have assumed that the pre-demolition cleaning activities under Work Activity 3 will be successful in removing dust/dirt from the steel building components and associated metal material surfaces so that PCBs will not be detected in post-cleaning verification wipe samples at concentrations greater than or equal to 100 ug/100cm². However, if the concentration of PCBs remaining on the steel building components and associated metal material surfaces following cleaning is greater than or equal to 100 ug/100 cm², then the steel building components and associated metal materials will be transported for off-site disposal as a TSCA waste. Following removal of the debris generated by the building demolition, clean backfill obtained from an off-site source would be placed, graded, and compacted above the remaining building floor slab to the existing grade which surrounds the building. After compacting the backfill, an asphalt cap would be installed as described under Work Activity 7 above.

Effectiveness

Implementing this alternative would meet the removal action objectives for the site and provide for the protection of public health and the environment. This alternative does not involve treatment of impacted materials; however, the demolition of the manufacturing building and cleaning and/or off-site disposal of impacted material/equipment will reduce the volume of impacted materials at the site. In addition, the installation of the cap over impacted soil and/or materials would reduce the mobility of the chemicals of interest (via overland transport and leaching through the subsurface), as well as limit the potential for humans and wildlife to contact these materials.

Long-term cap maintenance will be required for this alternative to remain effective and reliable. The final cap system will be maintained by conducting routine inspections of the integrity of the entire cap and sealing and patching any cracks and holes that may be observed. This alternative will also include the implementation of institutional controls. Institutional controls are minimal actions taken to reduce the potential for exposure to the impacted soil/materials or to mitigate the potential for future activities to compromise the effectiveness of a selected remedy. Institutional controls may include, for example, installation of additional site fences and deed restrictions. The purpose of implementing institutional controls such as deed restrictions would be to ensure that future site activities (e.g., construction and/or excavation) would be conducted in accordance with appropriate health and safety requirements and do not compromise the effectiveness of the final cap system. The specific institutional controls to be implemented at the site will be determined once the potential future use of the site is better known.

Dust may be generated during building demolition, materials handling, or surface preparation activities associated with installation of the cap. A site-specific Health and Safety Plan (HASP) would be developed during the design phase which would identify acceptable dust levels necessary to protect workers and the community from exposure, via inhalation, ingestion, or dermal contact, to chemicals of interest which may be present in the materials. An air monitoring plan would be instituted during implementation of the removal alternative. Detection of dust levels in excess of acceptable levels would indicate the need for additional measures to protect workers and the community from exposure. These additional measures could include, but may not be limited to:

- The use of personal protective equipment (PPE);
- The use of dust suppressants (e.g., water sprays); and
- Modifying the rate of demolition/construction.

It is anticipated that this alternative can be implemented within six months. Following completion of this alternative, the removal action objectives presented in section 4.3 will be met.

Implementability

Implementation of this alternative involves building demolition, off-site transportation and disposal of waste, and the construction of an asphalt cap. These activities have been commonly used as remedial measures at sites with similar conditions and wastes, and can be implemented to meet identified ARARs (see Tables 13 and 14). Implementation of this alternative can be completed within six months. The materials, labor, and services necessary to implement this alternative are readily available. The effectiveness of this alternative can be monitored by conducting routine inspections and maintenance of the integrity of the cap. Therefore, this alternative is technically feasible and could be implemented at the site.

<u>Cost</u>

The total estimated cost of implementing Alternative 1 (Leaving the First Floor Concrete Slab In-Place) is \$8,300,000. Assumptions made in developing this cost estimate as well as a detailed breakdown of the estimated costs are presented in Table 15. The total capital costs associated with implementation of Alternative 1 are \$8,125,169. Annual PRSC costs associated with Alternative 1 are \$17,390. Present worth of the annual PRSC costs for Alternative 1 is \$219,790.

5.3.2 Alternative 2 - Remove a Portion of the First Floor Concrete Slab

Under this alternative, the wood and concrete floors that contain PCBs at concentrations greater than or equal to 50 ppm (including a portion of the first floor concrete slab from areas potentially containing PCB concentrations greater than 50 ppm) would be removed from the building and transported for off-site disposal at a TSCA landfill permitted to accept debris containing PCBs at concentrations greater than or equal to 50 ppm. The portion of the first floor concrete slab to be removed for off-site disposal under this alternative is shown on Figure 17. Based on a preliminary review of the building, BBL has assumed that the wood and concrete floors could be removed (prior to demolition of the entire building) without jeopardizing the structural integrity of the building. However, before preparing a Contractor scope of work for the building demolition, a more comprehensive structural review of the building will be conducted by a Licensed Professional Engineer experienced in performing structural evaluations in order to confirm that the wood and concrete floors can be removed without impacting the structural integrity of the buildings the structural integrity of the building shell prior to general demolition activities. The Engineer will also provide recommendations for temporary structural support that may be needed during the floor removal activities.

Following removal of the wood and concrete floors that contain PCBs at concentrations greater than or equal to 50 ppm, the building would be demolished using traditional demolition techniques (i.e., a wrecking ball, excavators). Dust control measures will be implemented to minimize dust levels generated by the demolition work. The actual techniques/methods to be employed will be recommended by the demolition Contractor and reviewed and approved by the Engineer. The selected Contractor would be required to furnish details regarding demolition techniques/methods and the locations of debris staging/loading areas.

Debris generated by the building demolition which does not contain PCBs at concentrations greater than or equal to 50 ppm (excluding wood, drywall materials, or steel) would be placed as backfill within the below-grade portions of the first floor area. Additional backfill, consisting of a clean sand/gravel obtained from an off-site source, would be mixed in with the debris and placed, graded, and compacted to the existing grade which surrounds the building. Debris, consisting of wood and drywall, would be transported for off-site disposal at a non-TSCA landfill. Steel

building components and associated metal materials generated during the demolition activities which do not contain PCBs on the surfaces at concentrations greater than or equal to 100 ug/100cm² (as determined by verification sampling conducted under Work Activity 4) would be segregated and transported off-site for smelting. We have assumed that the pre-demolition cleaning activities under Work Activity 3 will be successful in removing dust/dirt from the steel building components and associated metal material surfaces so that PCBs will not be detected in post-cleaning verification wipe samples at concentrations greater than or equal to 100_ug/100cm². However, if the concentration of PCBs remaining on the steel building components and associated metal material surfaces following cleaning is greater than or equal to 100 ug/100cm², then the steel building components and associated metal materials will be transported for off-site disposal as a TSCA waste. After placing, grading, and compacting the backfill within the below grade portions of the first floor area, an asphalt cap would be installed as described under Work Activity 7.

Effectiveness

Implementing this alternative would meet the removal action objectives for the site and provide for the protection of public health and the environment. Similar to Alternative 1, this alternative does not involve treatment of impacted materials. However, the demolition of the manufacturing building and cleaning and/or off-site disposal of impacted material/equipment will reduce the volume of impacted materials at the site. In addition, the installation of the cap over impacted soil and/or materials would reduce the mobility of the chemicals of interest (via overland transport and leaching through the subsurface), as well as limit the potential for humans and wildlife to contact these materials.

The effectiveness and reliability of this alternative will be maintained through the implementation of cap maintenance activities and institutional controls, as described under Alternative 1.

A site-specific HASP and air monitoring plan (as described under Alternative 1) would also be developed during the design phase of this alternative to address any dust that is generated during building demolition, materials handling, or surface preparation activities associated with installation of the cap.

It is anticipated that this alternative can be implemented within six months. Following completion of this alternative, the removal action objectives presented in Section 4.3 will be met.

Implementability

Similar to Alternative 1, implementation of this alternative involves building demolition, off-site transportation and disposal of waste, and the construction of an asphalt cap. As discussed under Alternative 1, these activities are technically feasible and could be implemented at the site in compliance with identified ARARs.

<u>Cost</u>

The total estimated cost of implementing Alternative 2 (Remove a Portion of the First Floor Concrete Slab) is \$9,700,000. Assumptions made in developing this cost estimate as well as a detailed breakdown of the estimated costs are presented in Table 16. The total capital costs associated with implementation of Alternative 2 are \$9,515,051. Annual PRSC costs associated with Alternative 2 are \$17,227. Present worth of the annual PRSC costs for Alternative 2 is \$217,729.

5.3.3 Alternative 3 - Remove the Entire First Floor Concrete Slab

Under this alternative, the wood and concrete floors that contain PCBs at concentrations greater than or equal to 50 ppm (including the entire portion of the first floor concrete slab) would be removed from the building and transported for off-site disposal at a TSCA landfill permitted to accept debris containing PCBs at concentrations greater than or equal to 50 ppm. Based on a preliminary review of the building, BBL has assumed that the wood and concrete floors could be removed (prior to demolition of the entire building) without jeopardizing the structural integrity of the building. However, before preparing a Contractor scope of work for the building demolition, a more comprehensive structural review of the building will be conducted by a Licensed Professional Engineer experienced in performing structural evaluations in order to confirm that the wood and concrete floors can be removed without impacting the structural integrity of the building shell prior to general demolition activities. The Engineer will also provide recommendations for temporary structural support that may be needed during the floor removal activities.

Following removal of the wood and concrete floors that contain PCBs at concentrations greater than or equal to 50 ppm, the building would be demolished using traditional demolition techniques (i.e., a wrecking ball, excavators). Dust control measures will be implemented to minimize dust levels generated by the demolition work. The actual techniques/methods to be employed will be recommended by the demolition Contractor and reviewed and approved by the Engineer. The selected Contractor would be required to furnish details regarding demolition techniques/methods and the locations of debris staging/loading areas.

Debris generated by the building demolition which does not contain PCBs at concentrations greater than or equal to 50 ppm (excluding wood, drywall materials, or steel) would be placed as backfill within the below-grade portions of the first floor area. Additional backfill, consisting of a clean sand/gravel obtained from an off-site source, would be mixed in with the debris and placed, graded, and compacted to the existing grade which surrounds the building. Debris, consisting of wood and drywall, would be transported for off-site disposal at a non-TSCA landfill. Steel building components and associated metal materials generated during the demolition activities which do not contain PCBs on the surfaces at concentrations greater than or equal to 100 ug/100cm² (as determined by verification sampling conducted under Work Activity 4) would be segregated and transported off-site for smelting. We have assumed that the pre-demolition cleaning activities under Work Activity 3 will be successful in removing dust/dirt from the steel building components and associated metal material surfaces so that PCBs will not be detected in postcleaning verification wipe samples at concentrations greater than or equal to 100 ug/100cm². However, if the concentration of PCBs remaining on the steel building components and associated metal material surfaces following cleaning is greater than or equal to 100 ug/100cm², then the steel building components and associated metal materials will be transported for off-site disposal as a TSCA waste. After placing, grading, and compacting the backfill within the below grade portions of the first floor area, an asphalt cap would be installed as described under Work Activity 7.

Effectiveness

Implementing this alternative would meet the removal action objectives for the site and provide for the protection of public health and the environment. Similar to Alternatives 1 and 2, this alternative does not involve treatment of impacted materials. However, the demolition of the manufacturing building and cleaning and/or off-site disposal of impacted material/equipment will reduce the volume of impacted materials at the site. In addition, the installation of the cap over impacted soil and/or materials would reduce the mobility of the chemicals of interest (via overland transport and leaching through the subsurface), as well as limit the potential for humans and wildlife to contact these materials.

The effectiveness and reliability of this alternative will be maintained through the implementation of cap maintenance activities and institutional controls, as described under Alternative 1.

A site-specific HASP and air monitoring plan (as described under Alternative 1) would also be developed during the design phase of this alternative to address any dust generated during building demolition, materials handling, or surface preparation activities associated with installation of the cap.

It is anticipated that this alternative can be implemented within six months. Following completion of this alternative, the removal action objectives presented in Section 4.3 will be met.

Implementability

Similar to Alternatives 1 and 2, implementation of this alternative involves building demolition, off-site transportation and disposal of waste, and the construction of an asphalt cap. As discussed in Section 5.3.1, these activities are technically feasible and could be implemented at the site in compliance with identified ARARs.

<u>Cost</u>

The total estimated cost of implementing Alternative 3 (Remove the Entire First Floor Concrete Slab) is \$11,300,000. Assumptions made in developing this cost estimate as well as a detailed breakdown of the estimated costs are presented in Table 17. The total capital costs associated with implementation of Alternative 3 are \$11,037,432. Annual PRSC costs associated with Alternative 3 are \$17,486. Present worth of the annual PRSC costs for Alternative 3 is \$221,003.

6. Comparative Analysis of Removal Action Alternatives

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6. Comparative Analysis of Removal Action Alternatives

6.1 General

This section presents a detailed assessment of the removal action alternatives based on the evaluation criteria outlined in the USEPA's EE/CA guidance document. This section compares the relative performance of each alternative with respect to effectiveness, implementability, and cost. The purpose of this comparative analysis is to identify the advantages and disadvantages of the alternatives relative to each other and to aid in the selection of the appropriate removal action.

6.2 Effectiveness

Each of the alternatives evaluated meets the removal action objectives specified in Section 4.3. Each of the alternatives involves the demolition of the manufacturing building and the off-site disposal or cleaning of impacted materials/equipment. Each alternative also involves the installation of a cap over impacted soils/materials to reduce the mobility of chemicals of interest and mitigate direct exposure to these materials. Therefore, the three alternatives are equally effective at meeting the removal action objectives developed for the site.

6.3 Implementability

Building demolition and cap installation are well established technologies that have been used at a number of sites. Construction activities for each of the alternatives are not expected to be difficult to implement. The materials and services required for each alternative are readily available from local contractors. Therefore, the three alternatives are equally implementable at the site.

6.4 Cost

The following table summarizes the projected capital, PRSC, present worth, and total costs associated with each of the three alternatives.

Alternative	Capital Costs			Total Cost (rounded)
<u>Alternative 1</u> - Leave First Floor Concrete Slab In-Place	\$8,125,169	\$17,390	\$219,790	\$8,300,000
<u>Alternative 2</u> - Remove a Portion of the First Floor Concrete Slab	\$9,515,051	\$17,227	\$217,729	\$9,700,000
<u>Alternative 3</u> - Remove Entire First Floor Concrete Slab	\$11,037,432	\$17,486	\$221,003	\$11,300,000

Based on the above table, Alternative 1 is the least expensive removal action alternative to implement.

6.5 Recommended Removal Action Alternative

Based on the results of the comparative analysis presented in the previous section, the recommended removal action alternative to satisfy the removal action objectives for the Aerovox site is Alternative 1 (Leave the First Floor Concrete Slab In-Place). The results of the analysis indicate that each of the three alternatives are equally effective and implementable. However, the estimated cost of implementing Alternative 1 is \$1.4 million less than the estimated cost of implementing Alternative 2 and \$3 million less than the estimated cost of implementing Alternative 3. Therefore, the recommended removal action alternative is Alternative 1.

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Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>PCB Analytical Results</u> Full Core and Dust & Dirt Scrape Samples

Sample Type	Surface Material	Sample I.D.	PCBs Concentration ⁽¹⁾ [ppm]
First Floor	- Eastern Section		
Full Core	Brick Wall (painted)	1-WC-1	7.4
Scrape	Composite	1-DD-1	880.0
Scrape	Composite	1-DD-2	121.0
Scrape	Composite	1-DD-3	420.0
First Floor	- Across Sections	• · · · · · · · · · · · · · · · · · · ·	
Scrape	Composite	1-DD-4	2010.0
Scrape	Composite	1-DD-5	950.0
Scrape	Composite	1-DD-6	268.0
Second Flo	or - Eastern Section		
Full Core	Wood floor (stained)	2-FC-1	1,900.0
Full Core	Wood floor (stained)	2-FC-2	5,600.0
Full Core	Wood floor (stained)	2-FC-3	106.0
Scrape	Composite	2-DD-3	260.0
Scrape	Composite	2-DD-4	490.0
Full Core	Brick wall (painted)	2-WC-3	8.0
Full Core	Brick wall (painted)	2-WC-4	2.5
Second Flo	or - Western Section		· · · · · · · · · · · · · · · · · · ·
Full Core	Wood floor (stained)	2-FC-4	145.00
Full Core	Wood floor (stained)	2-FC-5	56,000.0
Full Core	Wood floor (stained)	2-FC-6	28.0
Full Core	Concrete floor (stained)	2-FC-7	12.7

Table 1 (Cont'd)

(Cont a) Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>PCB Analytical Results</u> Full Core and Dust & Dirt Scrape Samples

Sample Type	Surface Material	Sample I.D.	PCBs Concentration ⁽¹⁾ [ppm]
Full Core	Concrete floor (stained)	2-FC-8	156.0
Full Core	Ceiling beam (painted)	2-CC-1	28.3
Scrape	Composite	2-DD-1	1,020.0
Full Core	Brick Wall (painted)	2-WC-1	3.6
Full Core	Brick wall (painted)	2-WC-2	26.4
Second Flo	or - Across Sections	<u></u>	
Scrape	Composite	2-DD-2	300.0
Third Floor	- Eastern Section		· · · · · · · · · · · · · · · · · · ·
Full Core	Wood floor (stained)	3-FC-1	86.0
Full Core	Brick wall (stained)	3-WC-1	2.48
Full Core	Wood floor (stained)	3-FC-2	204.0
Scrape	Composite	3-DD-1	1,170.0
Scrape	Composite	3-DD-2	470.0

Notes:

- 1. ⁽¹⁾ Concentrations are given for total PCBs in parts per million (ppm).
- 2. < Indicates the compound was analyzed for but not detected. The associated value is the laboratory detection limit.
- 3. Values in **bold** exceed 50 ppm.

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>PCB Analytical Results</u> Wipe Samples

Surface Material	Sample I.D.	PCBs Concentration ⁽¹⁾ {ug/100cm ²]	
First Floor - Eastern Section		· · · · · · · · · · · · · · · · · · ·	
Concrete floor (painted)	1-FW-1	18.0	
Top of electrical duct. Horizontal steel surface (painted).	1-AW-2	20.8	
Concrete floor (painted)	1-FW-3	350.0	
Brick wall (painted)	1-WW-4	15.4	
Concrete floor (painted)	1-FW-5	59.0	
Top of start/stop panel of air compressor. Horizontal metal surface (painted).	1-EW-1	66.0	
Top of horizontal metal plate (painted).	1-EW-2	330.0	
Side of drying oven # 4. Horizontal metal surface (painted).	1-EW-3	13.7	
Side of rear base leg of federal press. Horizontal metal surface (painted).	1-EW-4	199.0	
First Floor - Western Section			
Wood column (painted). Vertical surface.	1-AW-6	10.5	
Elevated light fixture. Horizontal steel surface (painted).	1-AW-7	84.0	
Inside left door of despatch oven. Vertical metal surface (unpainted).	1-EW-5	<2.5	
"I" beam. Horizontal painted steel surface (pre-clean)	1-PSW-1	520.0	
"I" beam. Horizontal painted steel surface (post-clean: vacuumed).	1-PSW-1A	226.0	
Second Floor - Eastern Section	· · · · · · · · · · · · · · · · · · ·		
Wood floor	2-FW-4	17.8	
Tile floor	2-FW-5	14.8	
Tile floor	2-FW-6	14.6	

(Cont'd) Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>PCB Analytical Results</u> Wipe Samples

Surface Material	Sample I.D.	PCBs Concentration ⁽¹⁾ [ug/100cm ²]
Tile floor	2-FW-7	3.3
Top of stainless steel horizontal surface.	2-EW-2	217.0
Top of machine housing. Horizontal metal surface (painted).	2-EW-3	2.5
Horizontal diamond steel plate (pre-clean).	2-PSW-1	163.0
Horizontal diamond steel plate (post-clean: washed)	2-PSW-1A	34.0
Second Floor - Western Section		
Top of electrical box. Horizontal steel surface (painted).	2-AW-2	235.0
Wood floor (painted)	2-FW-3	90.0
Top of electrical box. Horizontal steel surface (painted).	2-AW-1	320.0
Base of press. Horizontal metal surface (painted).	2-EW-1	16.0
Third Floor - Eastern Section		
Tile floor	3-FW-1	22.6
Tile floor	3-FW-2	176.0
Tile floor	3-FW-3	98.0
Tile floor	3-FW-4	30.0
Top of assembly machine. Horizontal metal surface (painted).	3-EW-1	15.2
Top of gear housing of lead welding machine. Horizontal metal surface (painted).	3-EW-2	11.9
Top shelf of domino ink jet. Horizontal metal surface (painted).	3-EW-3	265.0
Top of base unit of metal winder. Horizontal metal surface (painted).	3-EW-4	68.0
Top of test/sort machine. Horizontal metal surface (painted).	3-EW-5	<2.5

<u>PCB Analytical Results</u> Wipe Samples

<u>Notes</u>:

- 1. ⁽¹⁾ Concentrations are given for total PCBs in micrograms per 100 cm².
- 2. Indicates the compound was analyzed for but not detected. The associated value is the laboratory detection limit.
- 3. Values in bold exceed $10 \text{ ug}/100 \text{ cm}^2$.

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>PCB Analytical Results</u> Soil Sampling from Beneath Concrete Floor Slab

Sample ID	Total PCBs (ppm)
IB6(0-2")	18,000
IB6(2-6")	3,200
IB8(0-2")	1,800
IB10(0-2")	11.8
IB20(0-2")	0.94
IB35(0-2")	19.6
IC5(0-2")	980
IC52(0-2")	0.218
ID7(0-2")	14,000
ID7(2-6")	4,900
ID63(0-2")	180
IE38(0-2")	0.62
IE59(0-2")	10.5
JF7(0-2")	13.0
IF10(0-2")	12.4
IH6(0-2")	2.3

Notes:

1. All concentrations in parts per million (ppm).

2. Samples analyzed using USEPA SW-846 Method 8082.

3. Samples IB6(2-6") and ID7(2-6") exceeded laboratory holding times.

4. Bold values indicate concentrations greater than 50 ppm.

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>PCB Analytical Results</u> Soil Located Beneath the Floor of the Manufacturing Building (ppm)

Sample ID	Sample Collection Date	Sample Collection Depth	Total PCBs (ppm)	
IB-6	5/13/98	1-2'	4,100	
ID-7	5/13/98	3-4'	2,000	

NOTES:

- 1. Shaded values represent concentrations which exceed the Massachusetts Department of Environmental Protection (MDEP) Soil Category S-3 & GW-3 Standard of 2 ppm for PCBs presented in the Massachusetts Contingency Plan (MCP), 310 CMR 40.0000, effective October 31, 1997.
- 2. All concentrations are reported in parts per million (ppm).
- 3. Samples were analyzed using United States Environmental Protection Agency SW-846 Method 8082.

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

TCL VOC Analytical Results

Soil Located Beneath the Floor of the Manufacturing Building (ppm)

	Soil S-3 &	Sample ID
	GW-3	ID-7
Constituent	Standard	(3-4")
Dichlorodifluoromethane	~	< 0.210
Chloromethane	•	< 0.210
Vinyl Chloride	, 2	< 0.210
Bromomethane	700	< 0.210
Chloroethane	-	< 0.210
Trichlorofluoromethane		< 0.210
1,1-Dichloroethylene	9	< 0.210
Methylene Chloride	700	< 0.210
1,1-Dichloroethane	500	< 0.210
cis-1,2-Dichloroethylene	500	< 0.210
trans-1,2-Dichloroethylene	2000	< 0.210
2,2-Dichloropropane	-	< 0.210
Bromochloromethane	-	< 0.210
Chloroform	300	< 0.210
1,1,1-Trichloroethane	500	< 0.210
Carbon Tetrachloride	40	< 0.210
1,1-Dichloropropene	-	< 0.210
Benzene	200	< 0.210
1,2-Dichloroethane	60	< 0.210
Trichloroethylene	500	30
1,2-Dichloropropane	40	< 0.210
Dibromomethane	-	< 0.210
Bromodichloromethane	90	< 0.210
Toluene	2500	< 0.210
1,1,2-Trichloroethane	10	< 0.210
Tetrachloroethylene	100	1.2
1,3-Dichloropropane	-	< 0.210
Dibromochloromethane	70	< 0.210
1,2-Dibromoethane	-	< 0.210
Chlorobenzene	40	< 0.210
Ethylbenzene	500	< 0.210
1,1,1,2-Tetrachloroethane	20	< 0.210
m,p-Xylene	2500	< 0.210
Styrene	100	< 0.210

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

TCL VOC Analytical Results

Soil Located Beneath the Floor of the Manufacturing Building (ppm)

	Soil S-3 &	Sample ID
	GW-3	ID-7
Constituent	Standard	(3-4')
o-Xylene	2500	< 0.210
Isopropylbenzene	-	< 0.210
n-Propylbenzene	-	< 0.210
tert-Butylbenzene	-	< 0.210
Bromoform	700	< 0.210
1,1,2,2-Tetrachloroethane	2	< 0.210
1,2,3-Trichloropropane	-	< 0.210
Bromobenzene	-	< 0.210
1,2,4-Trimethylbenzene	-	< 0.210
1,3,5-Trimethylbenzene	-	< 0.210
2-Chlorotoluene	-	< 0.210
4-Chlorotoluene	-	< 0.210
sec-Butylbenzene		< 0.210
p-Isopropyltoluene	-	< 0.210
1,3-Dichlorobenzene	500	< 0.210
1,4-Dichlorobenzene	200	< 0.210
1,2-Dichlorobenzene	500	< 0.210
n-Butylbenzene	-	< 0.210
1,2-Dibromo-3-chloroprop	-	< 0.210
1,2,4-Trichlorobenzene	800	1.5
Hexachlorobutadiene	40	< 0.210
Naphthalene	1000	< 0.210
1,2,3-Trichlorobenzene	-	0.72

NOTES:

- Soil Category S-3 & GW-3 Standards are presented in the Massachusetts Contingency Plan (MCP), 310 CMR 40.0000, issued by the Massachusetts Department of Environmental Protection (MDEP) Bureau of Waste Site Cleanup, effective October 31, 1997.
- 2. All concentrations are reported in parts per million (ppm).
- 3. Samples were analyzed using United States Environmental Protection Agency SW-846 Method 5035/8260.
- 4. "D" indicates a duplicate sample.
- 5. "<" indicates that the constituent was not detected at a concentration which exceeded the laboratory detection limit.
- 6. "-" indicates that an S-3 & GW-3 Standard Value was not listed for that constituent in the MCP 310 CMR 40.0000 document.

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>PCB Analytical Results</u> Soil Located Beneath the Parking Area (ppm)

Sample ID	Sample Collection	Sample Collection	Total PCBs
	Date	Depth	(ppm)
SB-01-2	5/20/98	1-2'	0.64
SB-02-1	5/21/98	0-1'	0.05
SB-03-2	5/20/98	1-2'	0.05
SB-04-2	5/20/98	1-2'	16
SB-05-2	5/19/98	1-2'	178
SB-06-1	5/19/98	0-1'	65
SB-07-2	5/19/98	0-1'	120
SB-07-5	5/19/98	4-5'	2900
SB-08-1	5/21/98	0-1'	0.14
SB-10-1	5/21/98	0-1'	4.2
SB-11-1.5	5/21/98	0.5-1.5	0.94
SB-12-1	5/20/98	0-1'	7.6
SB-13-1	5/20/98	0-1'	100
SB-14-5	5/20/98	4-5'	310
SB-14-5D	5/20/98	4-5'	170
SB-15-2	5/19/98	1-2'	0.12
SB-16-2	5/19/98	1-2'	12.2
SB-17-2	5/19/98	1-2'	0.14
SB-17-5	5/19/98	4-5'	0.6
SB-18-1	5/20/98	0-1'	84

NOTES:

- 1. Shaded values represent concentrations which exceed the Massachusetts Department of Environmental Protection (MDEP) Soil Category S-3 & GW-3 Standard of 2 ppm for PCBs presented in the Massachusetts Contingency Plan (MCP), 310 CMR 40.0000, effective October 31, 1997.
- 2. All concentrations are reported in parts per million (ppm).
- 3. Samples were analyzed using United States Environmental Protection Agency SW-846 Method 8082.
- 4. "D" in the Sample ID column indicates a duplicate sample.

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>PCB Analytical Results</u> Asphalt Located in the Parking Area (ppm)

Sample ID	Sample Collection	Composited from	Total PCBs
	Date	Discrete Samples from	(ppm)
COMP-1	5/19/98	SB-6, SB-7, SB-15, SB-16	136
COMP-2	5/20/98	SB-4, SB-5, SB-13, SB-14	140
COMP-3	5/21/98	SB-3, SB-10, SB-11, SB-12	33
COMP-4	5/21/98	SB-2, SB-8	1.13

NOTES:

1. All concentrations are reported in parts per million (ppm).

2. Samples were analyzed using United States Environmental Protection Agency SW-846 Method 8082.

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<u>TCL VOC Analytical Results</u> Soil Located Beneath the Parking Area (ppm)

	Soil S-3 &					Sample ID				
	GW-3	SB-01-8	SB-02-2	SB-03-2	SB-03-2D	SB-04-2	SB-05-2	SB-06-2	SB-07-5	SB-08-2
Constituent	Standard	(6-8')	(0-2')	(0-2')	(0-2')	(0-2')	(0-2')	(0-2')	(4-5')	(0-2')
Dichlorodifluoromethane	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Chloromethane		< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Vinyl Chloride	2	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Bromomethane	700	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Chloroethane	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Trichlorofluoromethane	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,1-Dichloroethylene	9	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Methylene Chloride	700	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,1-Dichloroethane	500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
cis-1,2-Dichloroethylene	500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
trans-1,2-Dichloroethylene	2000	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
2,2-Dichloropropane	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Bromochloromethane	•	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Chloroform	300	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,1,1-Trichloroethane	500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Carbon Tetrachloride	40	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,1-Dichloropropene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Benzene	200	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,2-Dichloroethane	60	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Trichloroethylene	500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	0.24	< 0.21	< 0.22	< 0.22
1,2-Dichloropropane	40	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Dibromomethane	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Bromodichloromethane	90	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Toluene	2500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,1,2-Trichloroethane	10	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Tetrachloroethylene	100	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>TCL VOC Analytical Results</u> Soil Located Beneath the Parking Area (ppm)

J	Soil S-3 &			<u> </u>	·····	Sample ID			<u> </u>	
	GW-3	SB-01-8	SB-02-2	SB-03-2	SB-03-2D	SB-04-2	SB-05-2	SB-06-2	SB-07-5	SB-08-2
Constituent	Standard	(6-8')	(0-2')	(0-2')	(0-2')	(0-2')	(0-2')	(0-2')	(4-5')	(0-2')
1,3-Dichloropropane	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Dibromochloromethane	70	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,2-Dibromoethane	<u> </u>	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Chlorobenzene	40	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Ethylbenzene	500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,1,1,2-Tetrachloroethane	20	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
m,p-Xylene	2500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Styrene	100	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
o-Xylene	2500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Isopropylbenzene	•	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
n-Propylbenzene		< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
tert-Butylbenzene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Bromoform	700	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,1,2,2-Tetrachloroethane	2	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,2,3-Trichloropropane	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Bromobenzene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,2,4-Trimethylbenzene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,3,5-Trimethylbenzene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
2-Chlorotoluene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
4-Chlorotoluene		< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
sec-Butylbenzene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
p-Isopropyltoluene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,3-Dichlorobenzene	500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,4-Dichlorobenzene	200	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,2-Dichlorobenzene	500	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
n-Butylbenzene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22

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<u>TCL VOC Analytical Results</u> Soil Located Beneath the Parking Area (ppm)

	Soil S-3 &	& Sample ID								
	GW-3	SB-01-8	SB-02-2	SB-03-2	SB-03-2D	SB-04-2	SB-05-2	SB-06-2	SB-07-5	SB-08-2
Constituent	Standard	(6-8')	(0-2')	(0-2')	(0-2')	(0-2')	(0-2')	(0-2')	(4-5')	(0-2')
1,2-Dibromo-3-chloropropane	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
1,2,4-Trichlorobenzene	800	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	0.44	< 0.22
Hexachlorobutadiene	40	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	< 0.22	< 0.22
Naphthalene	1000	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	0.39	< 0.21	0.33	< 0.22
1,2,3-Trichlorobenzene	-	< 0.21	< 0.23	< 0.23	< 0.23	< 0.22	< 0.23	< 0.21	1.1	< 0.22

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<u>TCL VOC Analytical Results</u> Soil Located Beneath the Parking Area (ppm)

	Soil S-3 &					Sample 1D			<u></u>	
	GW-3	SB-10-2	SB-11-2	SB-12-2	SB-13-2	SB-14-6	SB-15-2	SB-16-2	SB-17-2	SB-18-8
Constituent	Standard	(0-2')	(0.5-2')	(0-2')	(0-2')	(4-6')	(0-2')	(0-2')	(0-2')	(6-8')
Dichlorodifluoromethane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Chloromethane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Vinyl Chloride	2	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Bromomethane	700	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Chloroethane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Trichlorofluoromethane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
1,1-Dichloroethylene	9	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Methylene Chloride	700	< 0.21	0.22	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
1,1-Dichloroethane	500	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
cis-1,2-Dichloroethylene	500	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
trans-1,2-Dichloroethylene	2000	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
2,2-Dichloropropane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Bromochloromethane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Chloroform	300	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
1,1,1-Trichloroethane	500	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Carbon Tetrachloride	40	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
1,1-Dichloropropene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Benzene	200	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24 :	< 0.23	< 0.22
1,2-Dichloroethane	60	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Trichloroethylene	500	< 0.21	< 0.20	0.28	0.25	< 0.23	< 0.22	0.30	< 0.23	< 0.22
1,2-Dichloropropane	40	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Dibromomethane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Bromodichloromethane	90	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Toluene	2500	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
1,1,2-Trichloroethane	10	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Tetrachloroethylene	100	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22

<u>TCL VOC Analytical Results</u> Soil Located Beneath the Parking Area (ppm)

	Soil S-3 &				<u></u>	Sample ID			Sample ID							
	GW-3	SB-10-2	SB-11-2	SB-12-2	SB-13-2	SB-14-6	SB-15-2	SB-16-2	SB-17-2	SB-18-8						
Constituent	Standard	(0-2')	(0.5-2')	(0-2')	(0-2')	(4-6')	(0-2')	(0-2')	(0-2')	(6-8')						
1,3-Dichloropropane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
Dibromochloromethane	70	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
1,2-Dibromoethane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
Chlorobenzene	40	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
Ethylbenzene	500	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
1,1,1,2-Tetrachloroethane	20	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
m,p-Xylene	2500	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
Styrene	100	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
o-Xylene	2500	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
Isopropylbenzene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
n-Propylbenzene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
tert-Butylbenzene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
Bromoform	700	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
1,1,2,2-Tetrachloroethane	2	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
1,2,3-Trichloropropane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
Bromobenzene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
1,2,4-Trimethylbenzene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
1,3,5-Trimethylbenzene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24 :	< 0.23	< 0.22						
2-Chlorotoluene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
4-Chlorotoluene	•	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
sec-Butylbenzene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
p-Isopropyltoluene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
1,3-Dichlorobenzene	500	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
1,4-Dichlorobenzene	200	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
1,2-Dichlorobenzene	500	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						
n-Butylbenzene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22						

<u>TCL VOC Analytical Results</u> Soil Located Beneath the Parking Area (ppm)

	Soil S-3 &				···	Sample ID				• • •
Constituent	GW-3 Standard	SB-10-2 (0-2')	SB-11-2 (0.5-2')	SB-12-2 (0-2')	SB-13-2 (0-2')	SB-14-6 (4-6')	SB-15-2 (0-2')	SB-16-2 (0-2')	SB-17-2 (0-2')	SB-18-8 (6-8')
1,2-Dibromo-3-chloropropane	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
1,2,4-Trichlorobenzene	800	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Hexachlorobutadiene	40	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
Naphthalene	1000	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22
1,2,3-Trichlorobenzene	-	< 0.21	< 0.20	< 0.21	< 0.21	< 0.23	< 0.22	< 0.24	< 0.23	< 0.22

NOTES:

1. Soil Category S-3 & GW-3 Standards are presented in the Massachusetts Contingency Plan (MCP), 310 CMR 40.0000, issued by the Massachusetts Department of Environmental Protection (MDEP) Bureau of Waste Site Cleanup, effective October 31, 1997.

2. All concentrations are reported in parts per million (ppm).

3. Samples were analyzed using United States Environmental Protection Agency SW-846 Method 5035/8260.

4. "D" indicates a duplicate sample.

5. "<" indicates that the constituent was not detected at a concentration which exceeded the laboratory detection limit.

6. "-" indicates that an S-3 & GW-3 Standard Value was not listed for that constituent in the MCP, 310 CMR 40.0000.

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>PCB Analytical Results</u> Ground Water Samples (ppb)

Sample ID	Sample Collection	Total PCBs
	Date	(ppb)
MW-2	5/27/98	< 5
MW-2A	5/27/98	< 48
MW-3	5/26/98	< 0.48
MW-3A	5/26/98	< 5
MW-4	5/27/98	< 2.5
MW-4A	5/27/98	36
MW-4B	5/28/98	< 0.48
MW-5	5/27/98	< 0.5
MW-6	5/27/98	33
MW-6A	5/27/98	9.6
MW-7	5/26/98	< 0.48
MW-7A	5/26/98	< 0.48
MW-8S	5/27/98	3.0

NOTES:

- Shaded values represent concentrations which exceed the Massachusetts Department of Environmental Protection (MDEP) Ground-Water Category GW-3 Standard of 0.3 ppb for PCBs presented in the Massachusetts Contingency Plan (MCP), 310 CMR 40.0000, effective October 31, 1997.
- 2. All concentrations are reported in parts per billion (ppb).
- 3. Samples were analyzed using United States Environmental Protection Agency SW-846 Method 8082.

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TCL VOC Analytical Results

Ground Water Samples (ppb)

	Ground Water	<u> </u>				<u> </u>								
	GW-3		Sample ID											
Constituent	Standard	MW-2	MW-2A	MW-3	MW-3A	MW-4	MW-4A	MW-4B	MW-5	MW-6	MW-6A	MW-7	MW-7A	MW-8S
Dichlorodifluoromethane	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Chloromethane	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Vinyl Chloride	40,000	< 25	< 5	270	76	490	< 5	55	< 5	< 250	< 5	520	< 5	< 5
Bromomethane	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Chloroethane	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Trichlorofluoromethane	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,1-Dichloroethylene	50,000	< 25	< 5	< 25	< 50	< 50	< 5	37	< 5	< 250	< 5	< 250	< 5	< 5
Methylene Chloride	50,000	< 25	< 5	< 25	< 50	< 50	< 5	12 B	< 5	< 250	< 5	< 250	< 5	< 5
1,1-Dichloroethane	50,000	< 2.5	< 5	< 25	< 50	< 50	< 5	9	< 5	< 250	< 5	< 250	< 5	< 5
cis-1,2-Dichloroethylene	50,000	< 25	< 5	98	< 50	850	9	470	< 5	890	95	2,900	< 5	29
trans-1,2-Dichloroethylene	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
2,2-Dichloropropane	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Bromochloromethane	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Chloroform	10,000	< 25	< 5	< 25	< 50	< 50	< 5	9	< 5	< 250	< 5	< 250	< 5	< 5
1,1,1-Trichloroethane	50,000	< 25	< 5	< 25	< 50	< 50	< 5	41	< 5	< 250	< 5	< 250	< 5	< 5
Carbon Tetrachloride	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
l,1-Dichloropropene		< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Benzene	7,000	< 25	< 5	< 25	60	< 50	< 5	< 5	< 5	< 250	< 5	< 250	35	< 5
1,2-Dichloroethane	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Trichloroethylene	20,000	< 25	< 5	< 25	< 50	< 50	10	3,600	< 5	5,000	< 5	8,900	< 5	< 5
1,2-Dichloropropane	30,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Dibromomethane		< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5

TCL VOC Analytical Results

Ground Water Samples (ppb)

	Ground Water GW-3							Sample ID				<u></u> . <u>.</u>	<u></u>	
Constituent	Standard	MW-2	MW-2A	MW-3	MW-3A	MW-4	MW-4A	MW-4B	MW-5	MW-6	MW-6A	MW-7	MW-7A	MW-8S
Bromodichloromethane	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Toluene	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,1,2-Trichloroethane	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Tetrachloroethylene	5,000	< 25	< 5	< 25	< 50	< 50	< 5	33	< 5	<250	17	< 250	< 5	< 5
1,3-Dichloropropane	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Dibromochloromethane	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,2-Dibromoethane	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Chlorobenzene	500	570	19	47	1000	55	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Ethylbenzene	4,000	< 25	< 5	150	95	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,1,1,2-Tetrachloroethane	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
m,p-Xylene	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Styrene	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
o-Xylene	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Isopropylbenzene	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
n-Propylbenzene	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
tert-Butylbenzene	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Bromoform	50,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,1,2,2-Tetrachloroethane	20,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,2,3-Trichloropropane		< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Bromobenzene	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,2,4-Trimethylbenzene	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,3,5-Trimethylbenzene	· ·	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5

TCL VOC Analytical Results

Ground Water Samples (ppb)

	Ground Water GW-3		Sample ID											<u> </u>
Constituent	Standard	MW-2	MW-2A	MW-3	MW-3A	MW-4	MW-4A	MW-4B	MW-5	MW-6	MW-6A	MW-7	MW-7A	MW-8S
2-Chlorotoluene		< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
4-Chlorotoluene	•	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
sec-Butylbenzene	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
p-Isopropyltoluene	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,3-Dichlorobenzene	8,000	150	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,4-Dichlorobenzene	8,000	220	7	35	< 50	110	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,2-Dichlorobenzene	8,000	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
n-Butylbenzene	-	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,2-Dibromo-3-chloropropane	•	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,2,4-Trichlorobenzene	500	< 25	< 5	< 25	< 50	< 50	< 5	5	< 5	< 250	< 5	< 250	< 5	< 5
Hexachlorobutadiene	90	< 25	< 5	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
Naphthalene	6,000	< 25	18	< 25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5
1,2,3-Trichlorobenzene	-	< 25	< 5	<25	< 50	< 50	< 5	< 5	< 5	< 250	< 5	< 250	< 5	< 5

NOTES:

1. Ground-water Category GW-3 Standards are presented in the Massachusetts Contingency Plan (MCP), 310 CMR 40.0000, issued by the

Massachusetts Department of Environmental Protection (MDEP) Bureau of Waste Site Cleanup, effective October 31, 1997.

2. All concentrations are reported in parts per billion (ppb).

3. Samples were analyzed using United States Environmental Protection Agency SW-846 Method 8260.

4. "<" indicates that the constituent was not detected at a concentration which exceeded the laboratory detection limit.

5. "-" indicates that a GW-3 Standard was not listed for that constituent in the MCP 310 CMR 40.0000 document.

6. "B" indicates that this constituent was also detected in the method blank.

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis

Ground-Water Elevation Data - May 21, 1998

Monitoring Wells	Top of Casing Elevation (AMSL)	Depth to Ground- Water	Ground-Water Elevation (AMSL)
Shallow Monitoring Wel	ls		
MW-2A	6.61	3.52	3.09
MW-3A	8.13	6.02	2.11
MW-4A	10.73	*	*
MW-6A	9.75	7.76	1.99
MW-7A	7.29	4.28	3.01
MW-8S	5.76	3.34	2.42
Deep Monitoring Wells			
MW-2	6.89	4.80	2.09
MW-3	6.91	4.85	2.06
MW-4	10.97	8.36	2.61
MW-5	15.48	11.92	3.56
MW-6	9.21	7.22	1.99
MW-7	7.54	4.80	2.74
MW-4B	8.99	6.40	2.59

Notes:

- 1. All measurements are given in feet.
- 2. AMSL = Above Mean Sea Level
- 3. All elevations were taken at the north side of the casings and are referenced to mean sea level datum per the site benchmark of known elevation of 4.76 feet at a point on sheet piling near monitoring well MW-2, as indicated in a July 15, 1998 letter from Kevin W. Forgue of G.A.F. Engineering, Inc. to Peter Szwaja of Aerovox, Inc. (copy of this letter is provided as Attachment 9).
- 4. The Depth to Ground-Water data were measured at the north side of the outer well casings. These data are presented in Attachment 5 (Field Notes Monitoring Well Assessment) of this Engineering Evaluation/Cost Analysis Report.
- 5. The Depth to Ground-Water and Ground-Water Elevation measurements were obtained on May 21, 1998 by BBL, during high tide.
- 6. * = The depth to ground water measured in MW-4A appears to be incorrect and not representative of actual ground-water conditions. Specifically, the depth to ground water presented in Attachment 6 of the EE/CA provides an anomalously low ground-water elevation when compared to the past several years of ground-water monitoring program. Accordingly, this elevation is not presented in this table or used as part of any hydrogeologic evaluation.

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

Ground-Water Elevation Data - March 11, 1998

Monitoring Wells	Top of Casing Elevation (AMSL)	Depth to Ground- Water Reading	Ground-Water Elevation (AMSL)
High Tide Readings			
Deep Wells			
MW-2	6.89	4.50	2.39
MW-3	6.91	4.57	2.34
MW-4	10.97	8.43	2.54
MW-7	7.54	4.99	2.55
Shallow Wells	·	<u> </u>	<u> </u>
MW-2A	6.61	3.34	3.27
MW-3A	8.13	5.66	2.47
MW-4A	10.73	7.46	3.27
MW-7A	7.29	4.29	3.00
Low Tide Readings	· · · · · · · · · · · · · · · · · · ·		
Deep Wells			· · · · · · · · · · · · · · · · · · ·
MW-2	6.89	5.04	1.85
MW-3	6.91	5.43	1.48
MW-4	10.97	10.21	0.76
MW-7	7.54	6.88	0.66
Shallow Wells	·		**************************************
MW-2A	6.61	3.35	3.26
MW-3A	8.13	5.35	2.78
MW-4A	10.73	7.47	3.26
MW-7A	7.29	4.29	3.00

Ground-Water Elevation Data - March 11, 1998

Notes:

- 1. All measurements are given in feet Above Mean Sea Level (AMSL).
- 2. Monitoring wells denoted by "A" are shallow monitoring wells; monitoring wells not denoted by "A" are deep monitoring wells.
- 3. All elevations were taken at the north side of the outer well casings and are referenced to mean sea level datum per the site benchmark of known elevation of 4.76 feet at a point on sheet piling near monitoring well MW-2, as indicated in a July 15, 1998 letter from Kevin W. Forgue of G.A.F. Engineering, Inc. to Peter Szwaja of Aerovox, Inc. (copy of this letter provided as Attachment 9).
- 4. The Depth to Ground-Water Readings were measured at the north side of the exterior casings and were obtained by SAIC Engineering, Inc. on March 11, 1998.
- 5. The Depth to Ground-Water Readings were obtained as part of the Aerovox Site Post-Closure Monitoring Program conducted by SAIC Engineering, Inc. following the remedial action completed at the Aerovox, Inc. Facility in 1984. That remedial action was completed in compliance with a 1982 Consent Order entered into by Aerovox, Inc. with the USEPA (September 21, 1984 letter from the USEPA).

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Potential Chemical-Specific ARARs

Regulation	Cfrattion and in sec.	D.C. Elterion/Statidard Provise	Applicability Appropriateness of	Consideration in the Removal - Process/Action for Attainmenter
Massachusetts Contingency Plan	 310 CMR 40.0000, Subpart I: 310 CMR 40.0974(2) - Table 1, MCP Method 1: Ground-Water Standards 310 CMR 40.0975(6)(c) - Table 4, MCP Method 1: Soil Category S-3 Standards 	Soil and ground-water standards for Method I only.	MCP Method 1, Category GW-3 standards are appropriate for this site because ground water in the vicinity of the building is not used as a current source of drinking water and is not a potential future source. For soils, MCP Method 1, Category S-3/GW-3 standards are appropriate because the soil at the facility is essentially inaccessible (i.e., covered with pavement or concrete), children are not present at the facility, and the frequency and intensity of exposure to soil by adults is low.	Applicable to use for screening the analytical data associated with this site to identify chemicals of interest.
USEPA's Integrated Risk Information System (IRIS)	To Be Considered Guidance	Cancer Slope Factors (CSFs)	CSFs are "to be considered" guidance values used to evaluate the potential carcinogenic hazard caused by exposure to certain contaminants from the site.	Demolition and capping of the facility will minimize exposure to potential receptors and provide protection of human health.
USEPA's Integrated Risk Information System (IRIS)	To Be Considered Guidance	Reference Doses (RfDs)	RfDs are "to be considered" guidance values used to evaluate the potential noncarcinogenic hazard caused by exposure to contaminants from the site.	Demolition and capping of the facility will minimize exposure to potential receptors and provide protection of human health.
PCB Cancer Dose Response Assessment and Application for Environmental Mixtures (EPA/600/P-96/801F, September 1996)	To Be Considered Guidance	Guidance for USEPA's reassessment of the carcinogenicity of PCBs.	Information presented in this USEPA document is "to be considered" in assessing potential carcinogenic risks associated with potential exposure to PCBs. This guidance document includes revised slope factors for PCBs based on the potential pathways of exposure.	Demolition and capping of the facility will minimize exposure to potential receptors and provide protection of human health.

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Potential Action-Specific ARARs

Regulation	Citation	Criterion/Standard, ale	Applicability/Appropriateness	Consideration in the Removal
National Emission Standards for Hazardous Air Pollutants (NESHAP)	40 CFR 61	Provides regulations for emissions of particular air pollutants from specific sources.	Applicable to the list of pollutants identified in 40 CFR 61.01, which includes asbestos, and applies to the owner or operator of any stationary source for which a standard is prescribed in 40 CFR 61. The majority of 40 CFR Part 61 pertains to air emissions from a specific facility operation (i.e., not building demolition); however, Subpart M of 40 CFR 61 is applicable to the removal action, as detailed below.	Subpart M of 40 CFR 61 will be followed, as appropriate, based on the results the asbestos survey to be conducted prior to building demolition.
40 CFR Subpart M - National Emission Standard for Asbestos	40 CFR 61.145	Provides standards for demolition of asbestos-containing materials.	Based on the presence of vinyl floor tile, pipe insulation materials, and boiler insulation materials within the building that may potentially contain asbestos, an asbestos survey will be conducted to determine if abatement is required prior to building demolition. Depending upon the results of that survey, this regulation (40 CFR 61.145 - Standard for Demolition and Renovation) may be applicable.	This regulation will be followed, as appropriate, based on the results the asbestos survey to be conducted prior to building demolition.
Massachusetts Air Pollution Control Regulations	310 CMR 7.09 and 7.15	Building demolition activities shall not cause or contribute to a condition of air pollution.	Applicable to building demolition activities.	Appropriate measures will be implemented during the building demolition activities to prevent excessive emissions of particulate matter, as required by this regulation. Potential mitigative measures to be implemented, as well as the associated air and dust monitoring activities, will be detailed in special conditions and plans/procedures to be developed during the design phase. Additionally, an asbestos survey will be conducted to determine if abatement measures are required prior to the building demolition.

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Potential Action-Specific ARARs

Regulation	* *****Citation	Criterion/Standard, 19	ApplicabilityAppropriateness	24 Consideration in the Removale Second Action for Arthrentian
Ambient Air Quality Standards for the Commonwealth of Massachusetts	310 CMR 6.04	Provides primary and secondary ambient air quality standards, including standards for particulate matter.	Applicable to the generation of particular matter during building demolition activities.	An air monitoring plan will be developed and implemented as part of the removal action, and appropriate dust suppression methods will be conducted (as necessary) based on the air monitoring results.
MDEP Recommended Threshold Effect Exposure Limits (TELs) and Allowable Ambient Limits (AALs)	To Be Considered Guidance	TEL and AAL values are exposure concentrations for air contaminants.	This guidance will be considered in evaluation of air emissions against TEL and AAL values.	These values will be considered in the development of an air monitoring plan that will be implemented as part of the removal action.
MDEP Noise Regulation	310 CMR 7.10	No person owning, leasing, or controlling a source of sound shall willfully, negligently, or through failure to provide necessary equipment, service, or maintenance or to take necessary precautions cause, suffer, allow, or permit unnecessary emissions from said source of sound that may cause noise.	Applicable to construction and demolition equipment which characteristically emit sound but which may be fitted and accommodated with equipment to suppress sound or may be operated in a manner so as to suppress sound.	Building demolition activities will be conducted to meet this regulation by implementing appropriate measures during building demolition activities to minimize unnecessary noise, as required by 310 CMR 7.10. Monitoring for noise will be conducted in accordance with the applicable requirements of the MDEP of Air Quality Control (DAQC)'s Policy 90-001, as detailed below.
MDEP Division of Air Quality Control (DAQC) Policy - Allowable Sound Emissions, Policy 90-001, dated February 1, 1990	To Be Considered Guidance	This policy sets-forth criteria for determining if a source of sound is in violation of the Department's noise regulation which applies to building demolition activities (i.e., 310 CMR 7.10, identified above). The DAQC policy criteria are to be measured both at the property line and the nearest inhabited residence.	"To be considered guidance" that will be considered for construction and demolition equipment which characteristically emit sound, but which may be fitted and accommodated with equipment to suppress sound or may be operated in a manner so as to suppress sound.	The criteria identified in DAQC Policy 90-001 will be measured at the property line during the building demolition activities (there are no inhabited residences in close proximity to the Aerovox facility).

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Potential Action-Specific ARARs

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TSCA Regulations	40 CFR 761.60	PCB disposal requirements.	Applicable to the disposal of certain PCB containing materials, including PCB liquids and PCB articles which includes leaking PCB small capacitors.	PCB contaminated waste materials will be disposed of in accordance with this citation, as required.
TSCA Regulations	40 CFR 761.61(a)(5)	Requirements for off-site disposal of bulk PCB remediation wastes, porous and non-porous PCB remediation waste, and liquid PCB remediation waste.	Applicable to the off-site disposal of PCB remediation wastes during implementation of the removal action.	PCB remediation wastes will be decontaminated or disposed of in accordance with the substantive requirements of this section.
TSCA Regulations	40 CFR 761.61(c)	Risk-based clean-up approval requirements for PCB remediation wastes	Applicable to sampling, clean-up, or disposal of PCB remediation waste in a manner other than the self- implementing provisions of 40 CFR 761.61(a) or performance based provisions of 40 CFR 761.61(b), or storage of PCB remediation waste in a manner other than 40 CFR 761.65.	The EPA Regional Administrator must determine that the removal action will not pose an unreasonable risk of injury to health or the environment.
TSCA Regulations	40 CFR 761.62	Disposal of PCB bulk product waste requirements.	Applicable to the disposal of PCB bulk product waste resulting from implementation of the removal action, including fluorescent light ballasts containing PCBs in the potting material.	Disposal of PCB bulk product waste will be conducted in accordance with this citation. Fluorescent light ballasts will be disposed of as PCB waste or decontaminated under 40 CFR 761.79, as required.

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Potential Action-Specific ARARs

Regulation	· Citation	Criterion/Standard	Applicability/Appropriateness	Consideration in the Removal Process/Action for Attainment
TSCA Regulations	40 CFR 761.65(a) and (c)(9)	Storage limitations for disposal.	Applicable to the storage for disposal of PCBs at concentrations of 50 ppm or greater and PCB items with PCB concentrations of 50 ppm or greater.	Any PCB waste generated from the removal action work activities will be disposed of within one year and stored in facilities described in 40 CFR 761.65. Liquid PCB remediation wastes will be stored in accordance with 40 CFR 761.61(c). Bulk PCB remediation wastes or bulk PCB product may be stored at the site for 180 days subject to conditions specified in 40 CFR 761.65(c)(9).
TSCA Regulations	40 CFR 761.79	Decontamination standards and procedures for removing PCBs which are regulated for disposal, from water, organic liquids, non- porous surfaces (including scrap metal from disassembled electrical equipment), concrete, and non- porous surfaces covered with a porous surface such as paint or coating on metal.	Applicable decontamination standards and procedures for removing PCBs from materials.	Decontamination procedures will be followed during work activities, as required.
TSCA PCB Spill Cleanup Policy	40 CFR 761 Subpart G, Sections 761.120 through 135	This policy establishes USEPA criteria used to determine the adequacy of the cleanup of spills resulting from the release of materials containing PCBs at concentrations of 50 ppm or greater.	The PCB Spill Cleanup Policy is "to be considered" to address PCB spills or leaks (if any) during implementation of the removal action.	The requirements of this policy will be considered, as appropriate, when determining the appropriate method(s) to address PCB spills or leaks (if any) that may occur during implementation of the removal action.

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Potential Action-Specific ARARs

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Guidance on Remedial Actions for Superfund Sites with PCB Contamination, OSWER Directive No. 9355.4-01, August 1990	To Be Considered Guidance	This guidance document outlines the remedial investigation/ feasibility and selection of remedy process as it specifically applies to the development, evaluation, and selection of remedial actions that address PCB contamination at Superfund sites.	This USEPA guidance document is "to be considered" during the EE/CA and removal action process.	This document will be used, as appropriate, as guidance during the EE/CA and removal action process.
Massachusetts Hazardous Waste Management	310 CMR 30.100	Establishes standards for the identification and listing of hazardous wastes.	Applicable to identifying and listing materials (if any) that are hazardous under Massachusetts regulations.	Materials associated with the removal action that require off-site disposal may be identified and listed (if any) as hazardous wastes.
Massachusetts Hazardous Waste Management Requirements for Generators	310 CMR 30.300	Establishes standards for various classes of generators.	Applicable to the generation of hazardous waste (if any) from removal action work activities.	Work activities will be managed in accordance with substantive requirements of these standards.

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Potential Action-Specific ARARs

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Massachusetts Hazardous Waste Management Closure and Post- Closure Care	310 CMR 30.633, 30.660- 30.669	Requirements for closure and post- closure care of a landfill or cell.	Applicable to the installation of a cap and post-closure activities to be conducted as part of the removal action.	The closure and post-closure care requirements of CMR 30.633 [and the requirements of 40 CFR 761.61(a)(7), whichever are more stringent for the type of cap to be designed/installed] will be implemented to meet these requirements, as appropriate for the type of cap to be constructed. As discussed in Section 5.3, the details of the final cap will be selected during the design phase of the project. Compliance with substantive requirements of these regulations will be achieved through development and implementation of a long-term operations and maintenance (O&M) plan. A long-term ground-water monitoring program will be part of the removal action. That monitoring program will comply with applicable and substantive ground-water protection requirements of 310 CMR 30.660 through 699.

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Potential Location-Specific ARARs

Regulation	Server California and Server	State Criterio D'Scheren Warner	A ADDIC BOILS AND SUBSIDIATE THE	Consideration in the Removalue
Floodplain Management - Executive Order 11988	40 CFR Appendix A to Part 6	Procedures on floodplain management and wetlands protection.	Applicable due to work activities being conducted in the 100-500 year floodplain and 100 year coastal floodplain (Federal Emergency Management Agency Flood Insurance Rate Map, Community Panel No. 255216- 0007B, dated January 5, 1984).	The removal action selected must be the best practical acceptable alternative. Remedial activities will be implemented to minimize potential harm to the floodplain and will observe floodplain protective measures.
Wetlands Protection - Executive Order 11990	40 CFR Appendix A to Part 6	Wetlands protection policy.	As identified in Section 6 - Requirements, if there is no wetlands impact identified, the action may proceed without further consideration.	The substantive portions of this regulation apply to work performed in a wetland, if wetlands are identified.
Coastal Zone Management	16 USC Parts 1452 et seq. 301 CMR 21.00	Procedures and requirements for the protection of the coastal zone.	Applicable - entire site is located in a coastal zone management area.	Actions must be consistent with State approved coastal zone management programs, to the maximum extent possible.
Waterways	301 CMR 9.00	Protection of waterways.	This regulation will be applicable if any portion of the site is within a filled tideland.	Remedial activities within a filled tideland (if any) will be consistent with substantive requirements of this regulation, as appropriate.
Wetlands Protection	310 CMR 10.00	Requirements for the protection of wetlands and other natural resource areas.	The site is located within the buffer zone of several coastal resources.	See particular resource areas listed below and actions to be taken within the buffer zones of those areas.
Areas Subject to Protection	310 CMR 10.02	Requirements for conducting activities within the areas subject to protection or Buffer Zone.	Relevant and appropriate to site activities within the Buffer Zone and within 25 feet of a Riverfront Area.	Some site activities will be conducted within the Buffer Zone or areas subject to protection. Remedial activities conducted will be consistent with substantive requirements of this regulation, as appropriate.

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Potential Location-Specific ARARs

Regulation	Citation	Criterion/Standardviges	+ Applicability/Appropriateness	Consideration in the Removal
Coastal Wetlands	310 CMR 10.24	Additional general provisions for conducting work activities within coastal resource areas to ensure coastline development is conducted to protect public interests in coastal resources.	These provisions apply to 310 CMR 10.21 through 10.37. The site is within buffer zone of several coastal resource areas.	Remedial activities conducted will be consistent with substantive requirements of this regulation, as appropriate.
Land Under the Ocean	310 CMR 10.25	Requirements for conducting activities on land under the ocean or nearshore areas of land under the ocean or within their buffer zones that are found to be significant to the protection of marine fisheries, protection of wildlife habitat, storm damage prevention or flood control.	Site is within buffer zone of Land Under the Ocean.	Remedial activities conducted will be consistent with substantive requirements of this regulation, as appropriate.
Salt Marshes	310 CMR 10.32	Requirements for conducting activities within a salt marsh or within its buffer zone when a salt marsh is determined to be significant to the protection of marine fisheries, the prevention of pollution, storm damage prevention or groundwater supply.	Site is within buffer zone of Salt Marshes.	Remedial activities conducted will be consistent with substantive requirements of this regulation, as appropriate.
Land Containing Shellfish	310 CMR 10.34	Requirements for the protection of marine fisheries as well as to the protection of the interest of land containing shellfish.	Site is within buffer zone of Land Containing Shellfish.	Remedial activities conducted will be consistent with substantive requirements of this regulation, as appropriate.
Land Under the Ocean, Ponds, Streams, Rivers, Lakes, or Creeks that Underlie an Anadromous/Catadromous Fish Run ("Fish Run")	310 CMR 10.35	Requirements for protection of fish runs.	Fish runs occur between banks of Acushnet River. The site is within the buffer zone of this area.	Remedial activities conducted will be consistent with substantive requirements of this regulation, as appropriate.

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Potential Location-Specific ARARs

Regulation	Cintion -	Contration/Standard Standard	Applicability Appropriatences	Consideration in the Removal
Riverfront Area	310 CMR 10.58	Requirements for the protection of private and public water supply; groundwater; provide flood control; prevent storm damage; prevent pollution; protect land containing shellfish; protect wildlife habitat; and to protect the fisheries.	Applicable to activities conducted within the Riverfront Area. The site is within the Riverfront Area (25 feet landward of the mean annual high-water line); thus the provisions of 310 CMR 10.58 apply.	The presumption requirements of 10.58 will be met, as the removal action is necessary to abate, minimize, stabilize, mitigate or eliminate the actual or potential release of PCBs from the site (Section III of the USEPA's Approval Memorandum). The work to be conducted within the Riverfront Area will be conducted with substantive requirements of this regulation, as appropriate.

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Table 15

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>Cost Estimate</u> Alternative 1 - Leave First Floor Concrete Floor Slab In-Place

Ж	Work Activities	Quantity	Units	Unit/Cost	Total
Cap	ital Costs		X		
1. Ą	dditional Building Characterization Sam	pling	an a		
А.	Sampling and analysis of brick walls in Pump Room and Tank Room for PCBs	1	LS	\$2,500	\$2,500
B	RCRA characterization sampling	1	LS	\$20,000	\$20,000
	Subtotal Additiona	l Building Ch	aracterizatio	on Sampling:	\$22,500
2. E	quipment/Appurtenances Inventory				
А.	Conduct equipment/appurtenances inventory. Includes site reconnaissance activities, reviewing documentation for equipment/appurtenances, and meeting with an Aerovox operations personnel.	1	LS	\$4,500	\$4,500
	Subtotal	Equipment/A	ppurtenance	es Inventory:	\$4,500
3. P	re-Demolition Cleaning	· · · · · · · · · · · · · · · · · · ·	•		
A.	Hand-wash interior surfaces to remove visible dust and dirt and to clean steel surfaces to ≤100 ug/100 cm ² . Includes disposal of cleaning water, dirt, and dust.	450,500	SF	\$2/SF	\$901,000
В.	Hand-wash equipment surfaces to ≤ 10 ug/100 cm ² . Includes disposal of cleaning water, dirt, and dust.	200	EA	\$250/EA	\$50,000
C.	Asbestos Removal and Disposal	1	LS	\$100,000	\$100,000
		Subtotal P	're-D emoliti	on Cleaning:	\$1,051,000
4. Pé	ost-Cleaning Verification Sampling				1992 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -
A.	Post-cleaning verification sampling for building materials.	1	LS	\$50,000	\$50,000
В.	Post-cleaning verification sampling for equipment	1	LS	\$45,000	\$45,000
	Subtotal	Post-Cleanin	g Verificatio	on Sampling:	\$95,000

<u>Cost Estimate</u> Alternative 1 - Leave First Floor Concrete Floor Slab In-Place

	Work Activities	Quantity	Units	Unit/Cost	Total
5. U	tility Modifications and Removal				
А.	Utility modifications, removal, and disposal prior to building demolition.	1	LS	\$100,000	\$100,000
	Subto	tal Utility Mo	difications a	nd Removal:	\$100,000
6. B	uilding Demolition and Disposal (Exclud	ing Concrete	Floor at Gra	de)	
А.	Removal of wood floor (TSCA material)	235,800	SF	\$5/SF	\$1,179,000
В.	Removal of concrete floor above first floor level (TSCA material)	15,000	SF	\$5.50/SF	\$82,500
C.	Building demolition	6,703,000	CF	\$0.23/CF	\$1,541,690
D.	Transportation and disposal of demolition debris: - to TSCA landfill (mainly wood and concrete floor materials)	2,000	Ton	\$2 00/Ton	\$400,000
	 to non-TSCA landfill (mainly brick, wood, and drywall) to steel smelting facility (mainly "I"-beams) 	6,250 1,225	Ton Ton	\$50/Ton \$10/Ton	\$312,500 \$12,250
	<u> </u>	Subtotal I	Demolition a	nd Disposal:	\$3,527,940
7. Si	te Restoration/Asphalt Cap Construction				
Α.	Placement and compaction of backfill over the concrete floor slab	22,400	СҮ	\$13.50/CY	\$302,400
B.	40 mil PVC liner	378,613	SF	\$0.34/SF	\$128,728
C.	Geosynthetic drainage composite	378,613	SF	\$1.40/SF	\$530,058
D.	2" Sand/gravel layer	2,337	CY	\$13.00/CY	\$30,381
E.	6" Run-of-crush stone layer	7,011	CY	\$18.47/CY	\$129,493
F.	2 ¹ / ₂ " Bituminous concrete base course	42,068	SY	\$4.50/SY	\$189,306
G.	1 ¹ / ₂ " Bituminous concrete wearing surface	42,068	SY	\$3.30/SY	\$138,824
	Subtotal Site 1	Restoration/A	sphalt Cap C	Construction	\$1,449,190

<u>Cost Estimate</u> Alternative 1 - Leave First Floor Concrete Floor Slab In-Place

Work Activities	Quantity	Units	Unit/Cost	Total
	Subtotal Work	Activities #	l through #7:	\$6,250,130
Engineer	ing, Administrati	ve, and Legal	Fees (10%):	\$625,013
		Conting	gency (20%):	\$1,250,026
	Total	Estimated C	Capital Cost:	\$8,125,169
Annual Post Removal Site Control (PRSC)	Costs	and a second		
Annual Cap Maintenance				\$14,492
		Subtotal I	PRSC Costs:	\$14,492
		Contin	gency (20%)	\$2,898
		Total	PRSC Costs	\$17,390
Pre Pre	sent Worth Cost	of PRSC (30	years @ 7%)	\$219,790
	Total Estima	ted Cost of A	lternative 1	\$8,344,959
		F	ounded To:	\$8,300,000

<u>Notes</u>:

- 1. Costs are based on contractor estimates from previous projects and BBL's experience.
- 2. Transportation and disposal costs are based on verbal quotations received in December 1997 from Chemical Waste Management, Inc. and Laidlaw PCB Services.
- 3. Volume, area, and mass calculations were conducted using the tables and calculations presented in Attachment 11.
- 4. Annual cap maintenance costs were estimated by assuming that 1% of the cap would be replaced every year. Therefore, 1% of the capital costs to construct the cap were used as the estimated annual cap maintenance cost.
- 5. Present worth was calculated using a 30-year duration and an annual interest rate of 7%.

Assumptions:

For each work activity, the cost estimate presented does include costs associated with mobilizing/ demobilizing equipment and materials to and from the site, as well as preparation and implementation of required plans and procedures. These plans and procedures may include, depending upon the work activity, a Sampling and Analysis Plan, a Health and Safety Plan, an Air Monitoring Plan, Dust Control Procedures, and a Waste Handling Plan. The assumptions below are listed in order by each work activity.

1A. Sampling and analysis cost estimate includes costs to collect up to 6 discrete full core samples from brick walls in the Pump Room and Tank Room for laboratory analysis for PCBs on a 24-hour turnaround basis.

<u>Cost Estimate</u> Alternative 1 - Leave First Floor Concrete Floor Slab In-Place

- 1B. RCRA characterization sampling cost estimate includes costs for up to 20 building material core samples for laboratory analysis for corrosivity, ignitability, reactivity, and Toxicity Characteristic Leaching Procedure (TCLP) volatile organic compounds (VOCs), TCLP semi-volatile organic compounds (SVOCs), and TCLP metals on a 5-day turnaround basis.
- 2A. Conduct equipment/appurtenances inventory cost estimate includes costs for conducting site reconnaissance activities, reviewing equipment/appurtenances documentation, and meeting with Aerovox facilities personnel to determine equipment/appurtenances (both inside and outside the building) which would be returned to commerce and equipment/appurtenances which would be scrapped.
- 3A. Hand-wash interior surfaces cost estimate includes costs for washing interior horizontal surfaces (including steel beams/columns and HVAC duct work) using detergent and rags to remove visible dust and dirt. Cost includes disposal of cleaning water, rags, dirt, and dust as TSCA waste. Prebuilding demolition cleaning area is based on the area of each floor level.
- 3B. Hand-wash equipment cost estimate includes costs for washing equipment using detergent and rags to remove visible dust and dirt. Cost includes disposal of cleaning water, rags, dirt, and dust as TSCA waste.
- 3C. Asbestos removal and disposal cost estimate includes costs for notifications, posting, permitting, air monitoring, record keeping, protective equipment, and removal and off-site disposal of the asbestos-containing materials in an approved non-hazardous waste landfill.
- 4A. Post-cleaning verification sampling for building materials cost estimate includes costs to collect verification wipe samples for laboratory analysis to confirm that interior building material surfaces (including steel and duct work) do not contain PCBs at concentrations greater than 100ug/100cm².
- 4B. Post-cleaning verification sampling for equipment cost estimate includes costs to collect verification wipe samples for laboratory analysis to confirm that equipment surfaces do not contain PCBs at concentrations greater than 10ug/100cm².
- 5A. Utility modifications, removal, and disposal cost estimate includes disconnecting electrical services; disconnecting the existing potable water supply; plugging sanitary sewer piping/floor drains; removing electrical equipment, boilers, and compressors; removing light fixtures; removing the fire protection and potable water supply piping; and removing HVAC system components.
- 6A. Removal of wood floor cost estimate includes costs for removing wood floors which contain PCBs at concentrations ≥50 ppm. Cost estimate assumes that the wood floors would be removed prior to demolition without affecting the structural integrity of the building.

Table 15

(Cont'd) Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>Cost Estimate</u> Alternative 1 - Leave First Floor Concrete Floor Slab In-Place

- 6B. Removal of concrete floor above first floor level cost estimate includes costs for removing the concrete floor (within the second level of the western section of the building) which contains PCBs at concentrations ≥ 50 ppm. Cost estimate assumes that the concrete floor would be removed prior to building demolition without affecting the structural integrity of the building. Cost estimate assumes that the concrete floor slab located on the first level will remain in-place.
- 6C. Building demolition cost estimate includes costs for the demolition of the remaining portion of the building above the floor slab at grade. Demolition would be conducted following wood and concrete floor removal using conventional demolition techniques (i.e., wrecking ball, excavators).
- 6D. Transportation and disposal cost estimate includes costs for transportation and disposal of TSCA and non-TSCA material generated during the demolition activities. Cost estimate assumes that material generated during the wood and concrete floor removal activities (containing PCBs at concentrations ≥50 ppm) would be disposed at a TSCA facility. Cost estimate assumes that wood and drywall materials generated under the building demolition cost estimate (excluding steel materials) would be disposed at a non-TSCA landfill. Cost estimate assumes that steel materials will be disposed at a steel smelting facility and that the value of the steel will off-set the smelting costs. Cost estimate for steel to smelting facility only includes costs for transportation.
- 7A. Placement and compaction of backfill cost estimate includes costs for providing, placing, and compacting imported clean backfill material (sand/unwashed gravel) over the first floor concrete floor slab to within one foot of existing grade.
- 7B-G. Asphalt cap construction cost estimate includes costs for installing a capping system constructed of a 1½ inch thick bituminous concrete wearing surface, a 2½ inch thick bituminous concrete base course, an 8 inch subbase (consisting of 6 inches of run-of-crush stone and 2 inches of sand), a geosynthetic drainage composite, and a 40 mil impermeable PVC or HDPE membrane.

Table 16

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis (EE/CA)

<u>Cost Estimate</u> Alternative 2 - Remove a Portion of the First Floor Concrete Slab

	Work Activities	Quantity	Units	Unit/Cost	Total
Cap	ital Costs				
1. A	dditional Building Characterization Sam	pling	e e de la seconda		
А.	Sampling and analysis of brick walls in Pump Room and Tank Room for PCBs	1	LS	\$2,500	\$2,500
B.	RCRA Characterization Sampling	1	LS	\$20,000	\$20,000
_	Subtotal Additiona	l Building Ch	aracterizati	on Sampling:	\$22,500
2. E	quipment/Appurtenances Inventory		1114 1		
А.	Conduct equipment/appurtenances inventory. Includes site reconnaissance activities, reviewing documentation for equipment/appurtenances, and meeting with an Aerovox operations personnel.	1	LS	\$4,500	\$4,500
	Subtotal	Equipment/A	ppurtenanc	es Inventory:	\$4,500
3. Pı	re-Demolition Cleaning			- 14	
Α.	Hand-wash interior surfaces to remove visible dust and dirt and to clean steel surfaces to $\leq 100 \text{ ug}/100 \text{ cm}^2$. Includes disposal of cleaning water, dirt, and dust.	450,500	SF	\$2/SF	\$901,000
B.	Hand-wash equipment surfaces to ≤ 10 ug/100 cm ² . Includes disposal of cleaning water, dirt, and dust.	200	EA	\$250/EA	\$50,000
C.	Asbestos Removal and Disposal	1	LS	\$100,000	\$100,000
		Subtotal F	Pre-Demoliti	on Cleaning:	\$1,051,000
4. Pc	ost-Cleaning Verification Sampling				
A.	Post-cleaning verification sampling for building materials	1	LS	\$50,000	\$50,000
В.	Post-cleaning verification sampling for equipment	1	LS	\$45,000	\$45,000
	Subtotal	Post-Cleanin	g Verificatio	on Samoling:	\$95,000

<u>Cost Estimate</u> Alternative 2 - Remove a Portion of the First Floor Concrete Slab

	Work Activities	Quantity	Units	Unit/Cost	Total
5. U	tility Modifications and Removal	2 2 2			
А.	Utility modifications, removal, and disposal prior to building demolition.	1	LS	\$100,000	\$100,000
	Subto	tal Utility Mo	difications a	nd Removal:	\$100,000
6. B	uilding Demolition and Disposal			2	
А.	Removal of wood floor (TSCA material)	235,800	SF	\$5.00/SF	\$1,179,000
B.	Removal of concrete floor above first floor level (TSCA material)	15,000	SF	\$5.50/SF	\$82,500
C.	Removal of concrete floor at first floor level (TSCA material)	96,920	SF	\$4.50/SF	\$436,140
D.	Building demolition	6,703,000	CF	\$0.23/CF	\$1,541,690
Е.	 Transportation and disposal of demolition debris: to TSCA landfill (mainly wood and concrete floor materials) to non-TSCA landfill (mainly brick, wood, and drywall) to steel smelting facility (mainly "I"-beams) 	6,360 1,740 1,225	Ton Ton Ton	\$200/Ton \$50/Ton \$10/Ton	\$1,272,000 \$87,000 \$12,250
		Subtotal I	\$4,610,580		
7. Si	ite Restoration/Asphalt Cap Construction	n sign filler	ate a construction of the second s		
A.	Placement and compaction of backfill over concrete floor slab	21,400	CY	\$13.50/CY	\$288,900
<u>В</u> .	40 mil PVC liner	378,613	SF	\$0.34/SF	\$128,728
С.	Geosynthetic drainage composite	378,613	SF	\$1.40/SF	\$530,058
D.	2" Sand/gravel layer	2,337	CY	\$13.00/CY	\$30,381
E.	6" Run-of-crush stone layer	7,011	CY	\$18.47/CY	\$129,493
F.	21/2" Bituminous concrete base course	42,068	SY	\$4.50/SY	\$189,306
G.	1 ¹ / ₂ " Bituminous concrete wearing surface	42,068	SY	\$3.30/SY	\$138,824

<u>Cost Estimate</u> Alternative 2 - Remove a Portion of the First Floor Concrete Slab

Work Activities	Quantity	Units	Unit/Cost	Total	
Subtotal Site Restoration/Asphalt Cap Construction:				\$1,435,690	
Subtotal Work Activities # 1 through #7: Engineering, Administrative, and Legal Fees (10%):					
	- 🦛 🚛 🗝 Total	Estimated C	Capital Cost:	\$9,515,051	
Annual Post Removal Site Control (PRSC)	Costs				
Annual Cap Maintenance				\$14,356	
		Subtotal H	PRSC Costs:	\$14,356	
		Contin	gency (20%)	\$2,871	
		Total	PRSC Costs	\$17,227	
Pro	esent Worth Cost	of PRSC (30	years @ 7%)	\$217,729	
	Total Estima	ted Cost of A	lternative 2	\$9,732,780	
		R	counded To:	\$9,700,000	

Notes:

- 1. Costs are based on contractor estimates from previous projects and BBL's experience.
- 2. Transportation and disposal costs are based on verbal quotations received in December 1997 from Chemical Waste Management, Inc. and Laidlaw PCB Services.
- 3. Volume, area, and mass calculations were conducted using the tables and calculations presented in Attachment 11.
- 4. Annual cap maintenance costs were estimated by assuming that 1% of the cap would be replaced every year. Therefore, 1% of the capital costs to construct the cap were used as the estimated annual cap maintenance cost.
- 5. Present worth was calculated using a 30-year duration and an annual interest rate of 7%.

Assumptions:

For each work activity, the cost estimate presented does include costs associated with mobilizing/ demobilizing equipment and materials to and from the site, as well as preparation and implementation of required plans and procedures. These plans and procedures may include, depending upon the work activity, a Sampling and Analysis Plan, a Health and Safety Plan, an Air Monitoring Plan, Dust Control Procedures, and a Waste Handling Plan. The assumptions below are listed in order by each work activity.

<u>Cost Estimate</u> Alternative 2 - Remove a Portion of the First Floor Concrete Slab

- 1A. Sampling and analysis cost estimate includes costs to collect up to 6 discrete full core samples from brick walls in the Pump Room and Tank Room for laboratory analysis for PCBs on a 24-hour turnaround basis.
- 1B. RCRA characterization sampling cost estimate includes costs for up to 20 building material core samples for laboratory analysis for corrosivity, ignitability, reactivity, and Toxicity Characteristic Leaching Procedure (TCLP) volatile organic compounds (VOCs), TCLP semi-volatile organic compounds (SVOCs), and TCLP metals on a 5-day turnaround basis.
- 2A. Conduct equipment/appurtenances inventory cost estimate includes costs for conducting site reconnaissance activities, reviewing equipment/appurtenances documentation, and meeting with Aerovox facilities personnel to determine equipment/appurtenances (both inside and outside the building) which would be returned to commerce and equipment/appurtenances which would be scrapped.
- 3A. Hand-wash interior surfaces cost estimate includes costs for washing interior horizontal surfaces (including steel beams/columns and HVAC duct work) using detergent and rags to remove visible dust and dirt. Cost includes disposal of cleaning water, rags, dirt, and dust as TSCA waste. Prebuilding demolition cleaning area is based on the area of each floor level.
- 3B. Hand-wash equipment cost estimate includes costs for washing equipment using detergent and rags to remove visible dust and dirt. Cost includes disposal of cleaning water, rags, dirt, and dust as TSCA waste.
- 3C. Asbestos removal and disposal cost estimate includes costs for notifications, posting, permitting, air monitoring, recordkeeping, protective equipment, and removal and off-site disposal of the asbestos-containing materials in an approved non-hazardous waste landfill.
- 4A. Post-cleaning verification sampling for building materials cost estimate includes costs to collect verification wipe samples for laboratory analysis to confirm that interior building material surfaces (including steel and duct work) do not contain PCBs at concentrations greater than 100ug/100cm².
- 4B. Post-cleaning verification sampling for equipment cost estimate includes costs to collect verification wipe samples for laboratory analysis to confirm that equipment surfaces do not contain PCBs at concentrations greater than 10ug/100cm².
- 5A. Utility modifications, removal, and disposal cost estimate includes disconnecting electrical services; disconnecting the existing potable water supply; plugging sanitary sewer piping/floor drains; removing electrical equipment, boilers, and compressors; removing light fixtures; removing the fire protection and potable water supply piping; and removing HVAC system components.
- 6A. Removal of wood floor cost estimate includes costs for removing wood floors which contain PCBs at concentrations ≥50 ppm. Cost estimate assumes that the wood floors would be removed prior to demolition without affecting the structural integrity of the building.

<u>Cost Estimate</u> Alternative 2 - Remove a Portion of the First Floor Concrete Slab

- 6B. Removal of concrete floor above first floor level cost estimate includes costs for removing the concrete floor (within the second level of the western section of the building) which contains PCBs at concentrations ≥50 ppm. Cost estimate assumes that the concrete floor would be removed prior to building demolition without affecting the structural integrity of the building. Cost estimate assumes that the concrete floor slab is 6 inches thick.
- 6C. Removal of concrete floor at first floor level cost estimate includes costs for removing the concrete floor slab from the first floor level of the western section of the building. Cost estimate assumes that the concrete floor slab is 6 inches thick.
- 6D. Building demolition cost estimate includes costs for the demolition of the remaining portion of the building above the floor slab at grade. Demolition would be conducted following wood and concrete floor removal using conventional demolition techniques (i.e., wrecking ball, excavators).
- 6E. Transportation and disposal cost estimate includes costs for transportation and disposal of TSCA and non-TSCA material generated during the demolition activities. Cost estimate assumes that material generated during the wood and concrete floor removal activities (containing PCBs at concentrations ≥50 ppm) would be disposed at a TSCA facility. Cost estimate assumes that wood and drywall materials generated under the building demolition cost estimate (excluding steel materials) would be disposed at a non-TSCA landfill. Cost estimate assumes that steel materials will be disposed at a steel smelting facility and that the value of the steel will off-set the smelting costs. Cost estimate for steel to smelting facility only includes costs for transportation.
- 7A. Placement and compaction of backfill cost estimate includes costs for providing, placing, and compacting imported clean backfill material (sand/unwashed gravel) over the removed/remaining first floor concrete floor slab to within one foot of existing grade. Cost estimate assumes that demolition materials, including brick and concrete (excluding wood materials), with PCBs at concentrations <50 ppm would be mixed with the backfill material and placed over the removed/remaining concrete floor slab.
- 7B-G. Asphalt cap construction cost estimate includes costs for installing a capping system constructed of a 1½ inch thick bituminous concrete wearing surface, a 2½ inch thick bituminous concrete base course, an 8 inch subbase (consisting of 6 inches of run-of-crush stone and 2 inches of sand), a geosynthetic drainage composite, and a 40 mil impermeable PVC or HDPE membrane.

Table 17

Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis

<u>Cost Estimate</u> Alternative 3 - Remove the Entire First Floor Concrete Slab

	Work Activities	Quantity	Units	Unit/Cost	Total
Сар	ital Costs				
1. A	dditional Building Characterization Sam	pling			<u>د</u>
А.	Sampling and analysis of brick walls in Pump Room and Tank Room for PCBs	1	LS	\$2,500	\$2,500
В.	RCRA characterization sampling	I	LS	\$20,000	\$20,000
	Subtotal Additiona	l Building Ch	aracterizatio	on Sampling:	\$22,500
2. E	quipment/Appurtenances Inventory				
Α.	Conduct equipment/appurtenances inventory. Includes site reconnaissance activities, reviewing documentation for equipment/appurtenances, and meeting with an Aerovox operations personnel.	1	LS	\$4,500	\$4,500
Subtotal Equipment/Appurtenances Inventory:					
3. Pi	re-Demolition Cleaning		······································	•	
Α.	Hand-wash interior surfaces to remove visible dust and dirt and to clean steel surfaces to $\leq 100 \text{ ug}/100 \text{ cm}^2$. Includes disposal of cleaning water, dirt, and dust.	450,500	SF	\$2/SF	\$901,000
В.	Hand-wash equipment surfaces to ≤ 10 ug/100 cm ² . Includes disposal of cleaning water, dirt, and dust.	200	EA	\$250/EA	\$50,000
С.	Asbestos Removal	1	LS	\$100,000	\$100,000
	Subtotal Pre-Demolition Cleaning:				
4. Po	ost-Cleaning Verification Sampling				· · · · · · · · · · · · · · · · · · ·
A.	Post-cleaning verification sampling for building materials	1	LS	\$50,000	\$50,000
В.	Post-cleaning verification sampling for equipment	1	LS	\$45,000	\$45,000
	Subtotal	Post-Cleanin	g Verificatio	on Sampling:	\$95,000

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<u>Cost Estimate</u> Alternative 3 - Remove the Entire First Floor Concrete Slab

	Work Activities	Quantity	Units	Unit/Cost	Total	
5. U	Itility Modifications and Removal	. 45 °				
Α.	Utility modifications, removal, and disposal prior to building demolition.	1	LS	\$100,000	\$100,000	
	Subto	tal Utility Mo	difications a	nd Removal:	\$100,000	
6. B	uilding Demolition and Disposal					
А.	Removal of wood floor (TSCA material)	235,800	SF	\$5.00/SF	\$1,179,000	
В.	Removal of concrete floor above first floor level (TSCA material)	15,000	SF	\$5.50/SF	\$82,500	
C.	Removal of concrete floor at first floor level (TSCA material)	182,134	SF	\$4.50/SF	\$819,603	
D.	Building demolition	6,703,000	CF	\$0.23/CF	\$1,541,690	
Е.	 Transportation and disposal of demolition debris: to TSCA landfill (mainly wood and concrete floor materials) to non-TSCA landfill (mainly brick, wood, and drywall) to steel smelting facility (mainly "l"-beams) 	10,190 1,740 1,225	Ton Ton Ton	\$200/Ton \$50/Ton \$10/Ton	\$2,038,000 \$87,000 \$12,250	
		Subtotal Demolition and Disposal: \$5,760,043				
7. Şi	te Restoration/Asphalt Cap Construction	n			· · · · · ·	
A.	Placement and compaction of backfill material over removed concrete slab area	23,000	CY	\$13.50/CY	\$310,500	
B.	40 mil PVC liner	378,613	SF	\$0.34/SF	\$128,728	
C.	Geosynthetic drainage composite	378,613	SF	\$1.40/SF	\$530,058	
D.	2" Sand/gravel layer	2,337	CY	\$13.00/CY	\$30,381	
Е.	6" Run-of-crush stone layer	7,011	СҮ	\$18.47/CY	\$129,493	
F.	2 ¹ / ₂ " Bituminous concrete base course	42,068	SY	\$4.50/SY	\$189,306	

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<u>Cost Estimate</u> Alternative 3 - Remove the Entire First Floor Concrete Slab

	Work Activities	Quantity	Units	Unit/Cost	Total		
G.	1 ¹ / ₂ " Bituminous concrete wearing surface	42,068	SY	\$3.30/SY	\$138,824		
Subtotal Site Restoration/Asphalt Cap Construction:							
Subtotal Work Activities # 1 through #7:					\$8,490,333		
Engineering, Administrative, and Legal Fees (10%):					\$849,033		
	Contingency (20%):						
Total Estimated Capital Cost:					\$11,037,432		
Ann	Annual Post Removal Site Control (PRSC) Costs						
Annual Cap Maintenance					\$14,572		
Subtotal PRSC Costs:					\$14,572		
Contingency (20%)					\$2,914		
Total PRSC Costs					\$17,480		
	Present Worth Cost of PRSC (30 years @ 7%)						
Total Estimated Cost of Alternative 3					\$11,258,43		
er Na]	Rounded To:	\$11,300,000		

Notes:

- 1. Costs are based on contractor estimates from previous projects and BBL's experience.
- 2. Transportation and disposal costs are based on verbal quotations received in December 1997 from Chemical Waste Management, Inc. and Laidlaw PCB Services.
- 3. Volume, area, and mass calculations were conducted using the tables and calculations presented in Attachment 11.
- 4. Annual cap maintenance costs were estimated by assuming that 1% of the cap would be replaced every year. Therefore, 1% of the capital costs to construct the cap were used as the estimated annual cap maintenance cost.
- 5. Present worth was calculated using a 30-year duration and an annual interest rate of 7%.

Assumptions:

For each work activity, the cost estimate presented does include costs associated with mobilizing/ demobilizing equipment and materials to and from the site, as well as preparation and implementation of required plans and procedures. These plans and procedures may include, depending upon the work activity, a Sampling and Analysis Plan, a Health and Safety Plan, an Air Monitoring Plan, Dust Control Procedures, and a Waste Handling Plan. The assumptions below are listed in order by each work activity.

Table 17

(Cont'd) Aerovox, Inc. Facility New Bedford, Massachusetts Engineering Evaluation/Cost Analysis

<u>Cost Estimate</u> Alternative 3 - Remove the Entire First Floor Concrete Slab

- 1A. Sampling and analysis cost estimate includes costs to collect up to 6 discrete full core samples from brick walls in the Pump Room and Tank Room for laboratory analysis for PCBs on a 24-hour turnaround basis.
- 1B. RCRA characterization sampling cost estimate includes costs for up to 20 building material core samples for laboratory analysis for corrosivity, ignitability, reactivity, and Toxicity Characteristic Leaching Procedure (TCLP) volatile organic compounds (VOCs), TCLP semi-volatile organic compounds (SVOCs), and TCLP metals on a 5-day turnaround basis.
- 2A. Conduct equipment/appurtenances inventory cost estimate includes costs for conducting site reconnaissance activities, reviewing equipment/appurtenances documentation, and meeting with Aerovox facilities personnel to determine equipment/appurtenances (both inside and outside the building) which would be returned to commerce and equipment/appurtenances which would be scrapped.
- 3A. Hand-wash interior surfaces cost estimate includes costs for washing interior horizontal surfaces (including steel beams/columns and HVAC duct work) using detergent and rags to remove visible dust and dirt. Cost includes disposal of cleaning water, rags, dirt, and dust as TSCA waste. Prebuilding demolition cleaning area is based on the area of each floor level.
- 3B. Hand-wash equipment cost estimate includes costs for washing equipment using detergent and rags to remove visible dust and dirt. Cost includes disposal of cleaning water, rags, dirt, and dust as TSCA waste.
- 3C. Asbestos removal and disposal cost estimate includes costs for notifications, posting, permitting, air monitoring, record keeping, protective equipment, and removal and off-site disposal of the asbestos-containing materials in an approved non-hazardous waste landfill.
- 4A. Post-cleaning verification sampling for building materials cost estimate includes costs to collect verification wipe samples for laboratory analysis to confirm that interior building material surfaces (including steel and duct work) do not contain PCBs at concentrations greater than 100ug/100cm².
- 4B. Post-cleaning verification sampling for equipment cost estimate includes costs to collect verification wipe samples for laboratory analysis to confirm that equipment surfaces do not contain PCBs at concentrations greater than 10ug/100cm².
- 5A. Utility modifications, removal, and disposal cost estimate includes disconnecting electrical services; disconnecting the existing potable water supply; plugging sanitary sewer piping/floor drains; removing electrical equipment, boilers, and compressors; removing light fixtures; removing the fire protection and potable water supply piping; and removing HVAC system components.

<u>Cost Estimate</u> Alternative 3 - Remove the Entire First Floor Concrete Slab

- 6A. Removal of wood floor cost estimate includes costs for removing wood floors which contain PCBs at concentrations >50 ppm. Cost estimate assumes that the wood floors would be removed prior to demolition without affecting the structural integrity of the building.
- 6B. Removal of concrete floor above first floor level cost estimate includes costs for removing the concrete floor (within the second level of the western section of the building) which contains PCBs at concentrations ≥50 ppm. Cost estimate assumes that the concrete floor would be removed prior to building demolition without affecting the structural integrity of the building. Cost estimate assumes that the concrete floor slab is 6 inches thick.
- 6C. Removal of concrete floor at first floor level cost estimate includes costs for removing the concrete floor slab from the entire first floor level of the building. Cost estimate assumes that the concrete floor slab is 6 inches thick.
- 6D. Building demolition cost estimate includes costs for the demolition of the remaining portion of the building above the floor slab at grade. Demolition would be conducted following wood and concrete floor removal using conventional demolition techniques (i.e., wrecking ball, excavators).
- 6E. Transportation and disposal cost estimate includes costs for transportation and disposal of TSCA and non-TSCA material generated during the demolition activities. Cost estimate assumes that material generated during the wood and concrete floor removal activities (containing PCBs at concentrations ≥50 ppm) would be disposed at a TSCA facility. Cost estimate assumes that wood and drywall materials generated under the building demolition cost estimate (excluding steel materials) would be disposed at a non-TSCA landfill. Cost estimate assumes that steel materials will be disposed at a steel smelting facility and that the value of the steel will off-set the smelting costs. Cost estimate for steel to smelting facility only includes costs for transportation.
- 7A. Placement and compaction of backfill cost estimate includes costs for providing, placing, and compacting imported clean backfill material (sand/unwashed gravel) over the removed first floor slab area to within one foot of existing grade. Cost estimate assumes that demolition materials, including brick and concrete (excluding wood materials), with PCBs at concentrations <50 ppm would be mixed with the backfill material and placed over the removed first floor slab area.
- 7B-G. Asphalt cap construction cost estimate includes costs for installing a capping system constructed of a 1½ inch thick bituminous concrete wearing surface, a 2½ inch thick bituminous concrete base course, an 8 inch subbase (consisting of 6 inches of run-of-crush stone and 2 inches of sand), a geosynthetic drainage composite, and a 40 mil impermeable PVC or HDPE membrane.

Figures

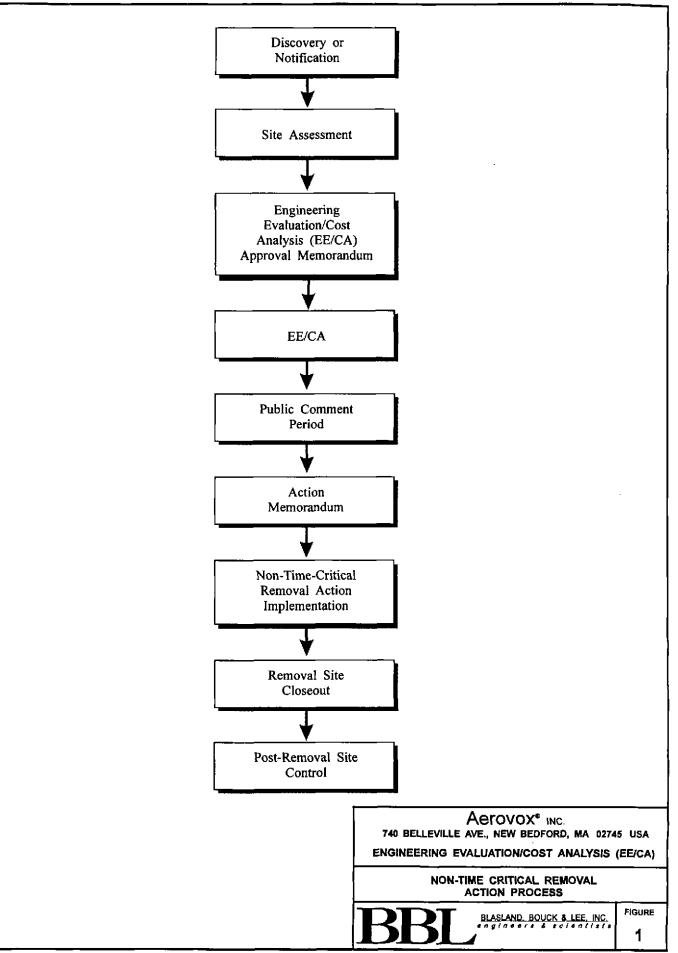
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BLASLAND, BOUCK & LEE, INC. engineers & scientists

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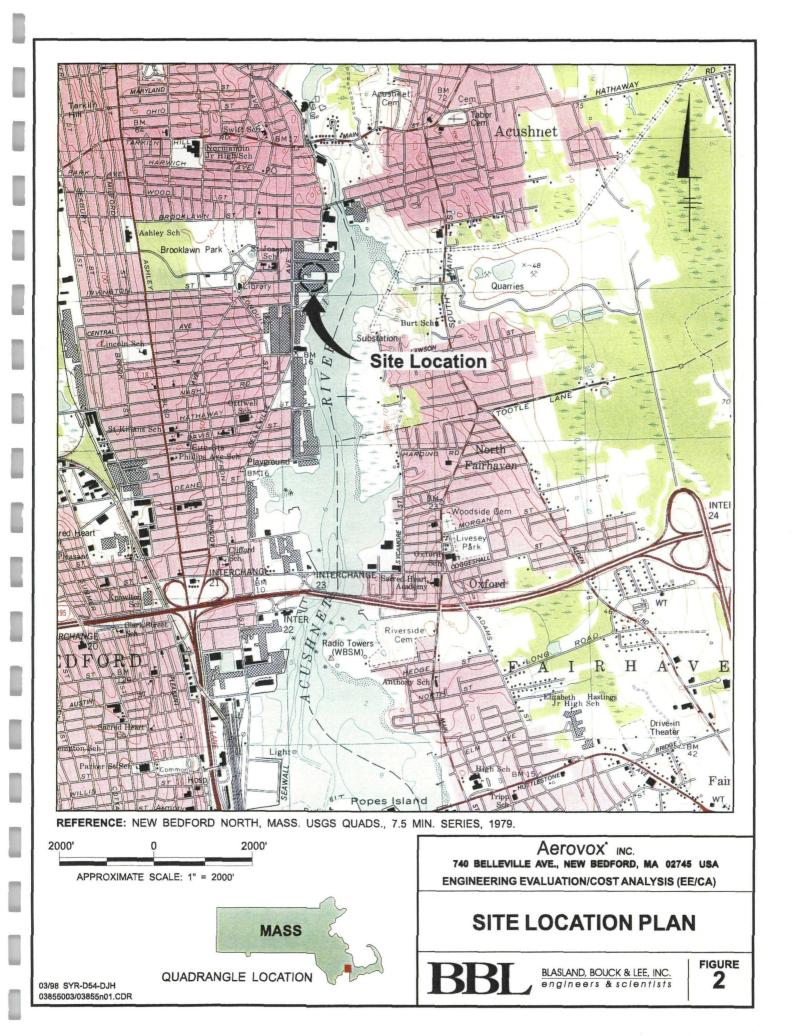
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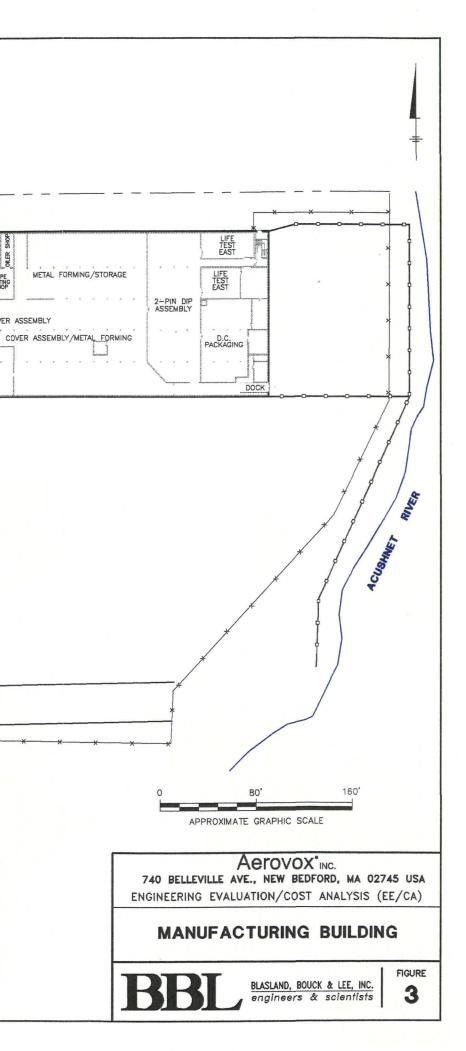


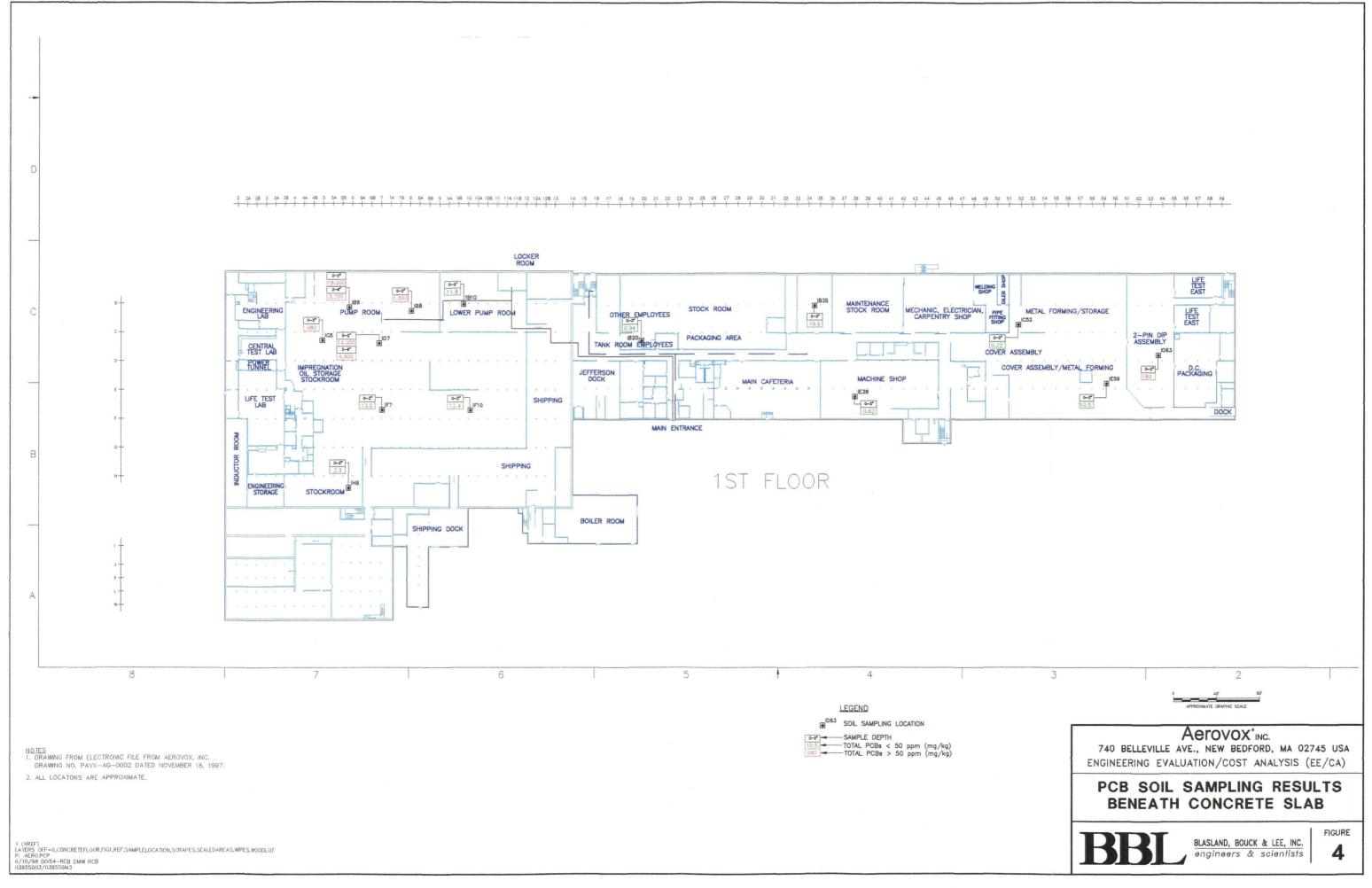
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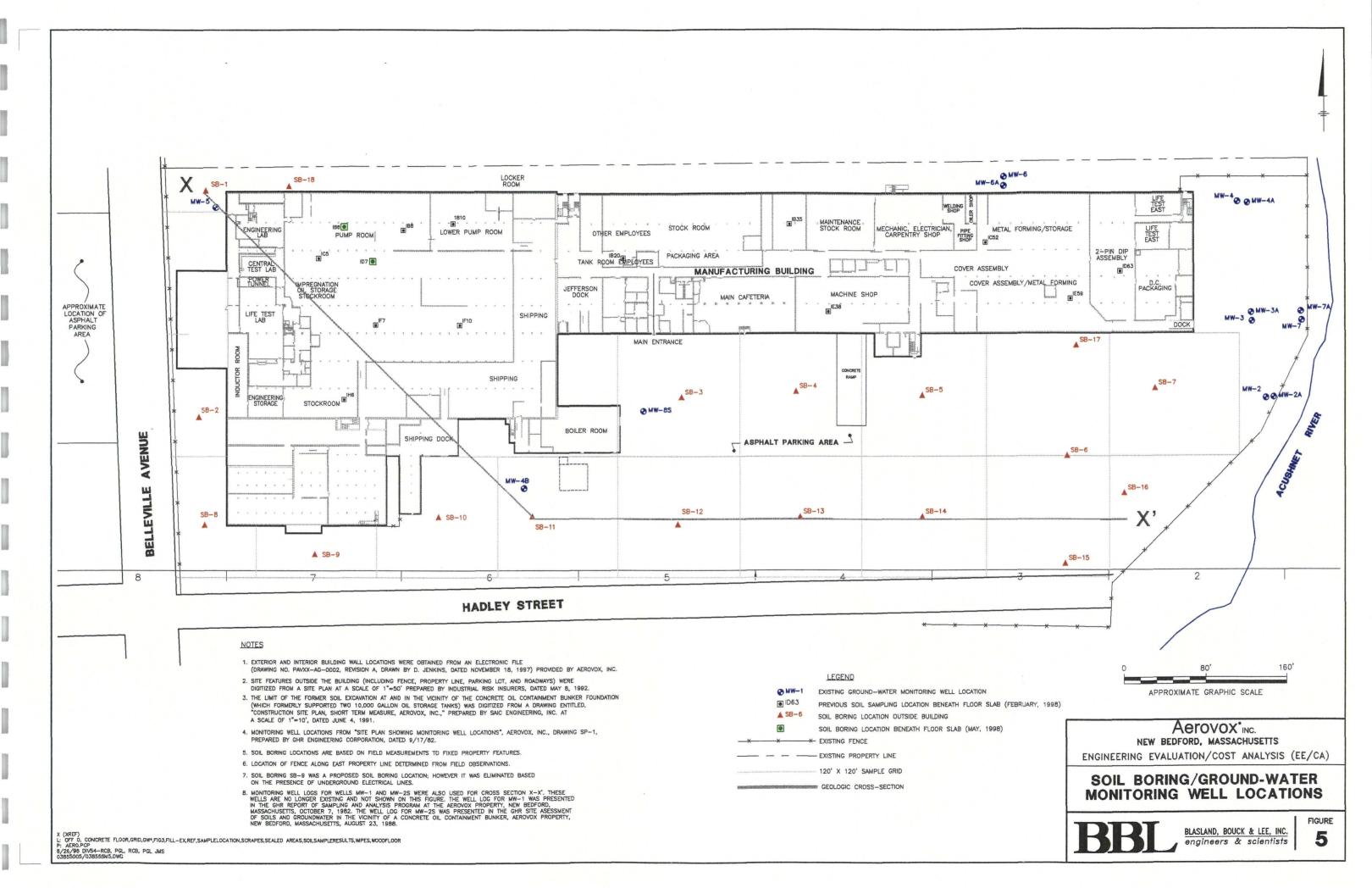
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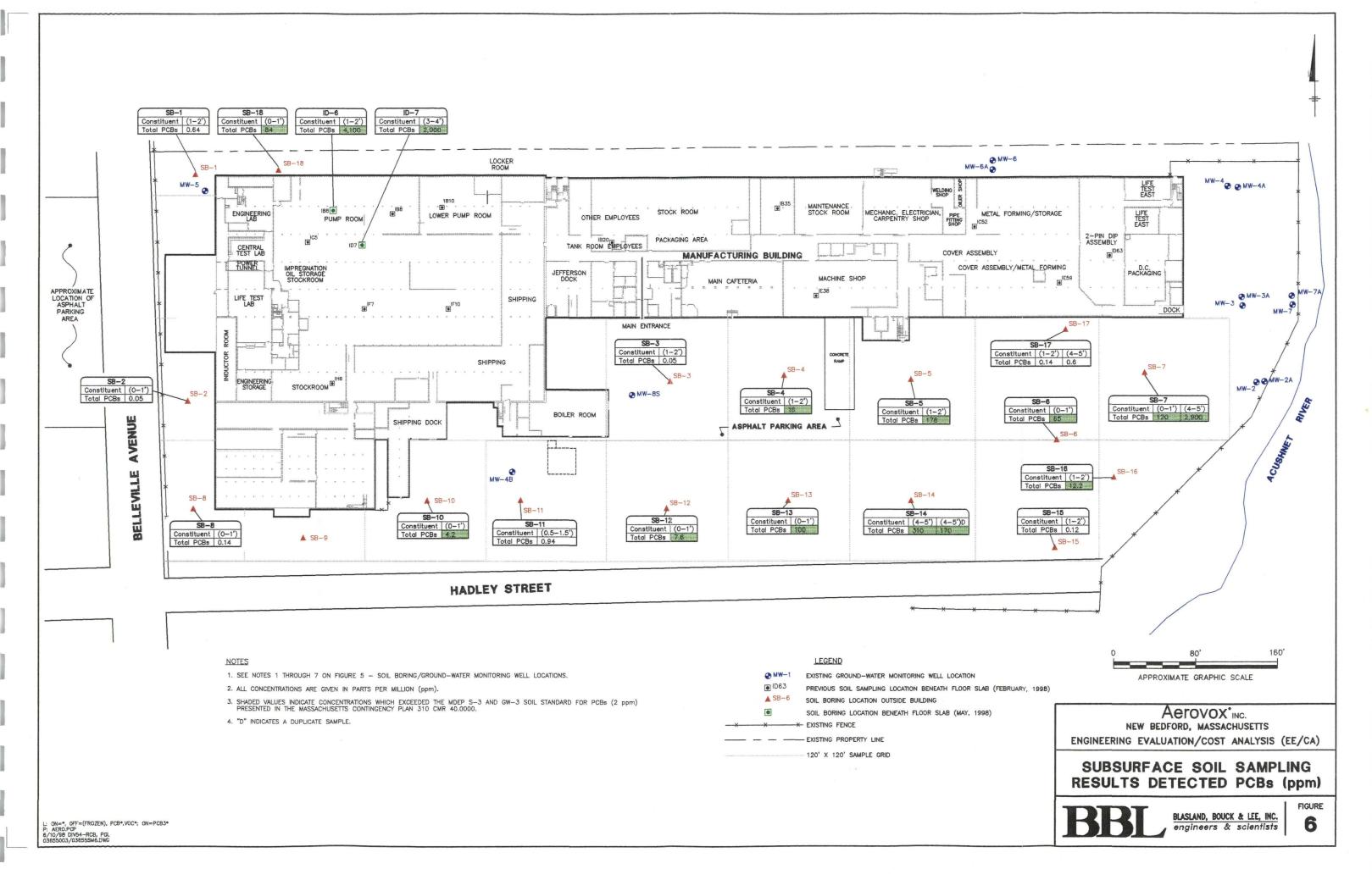


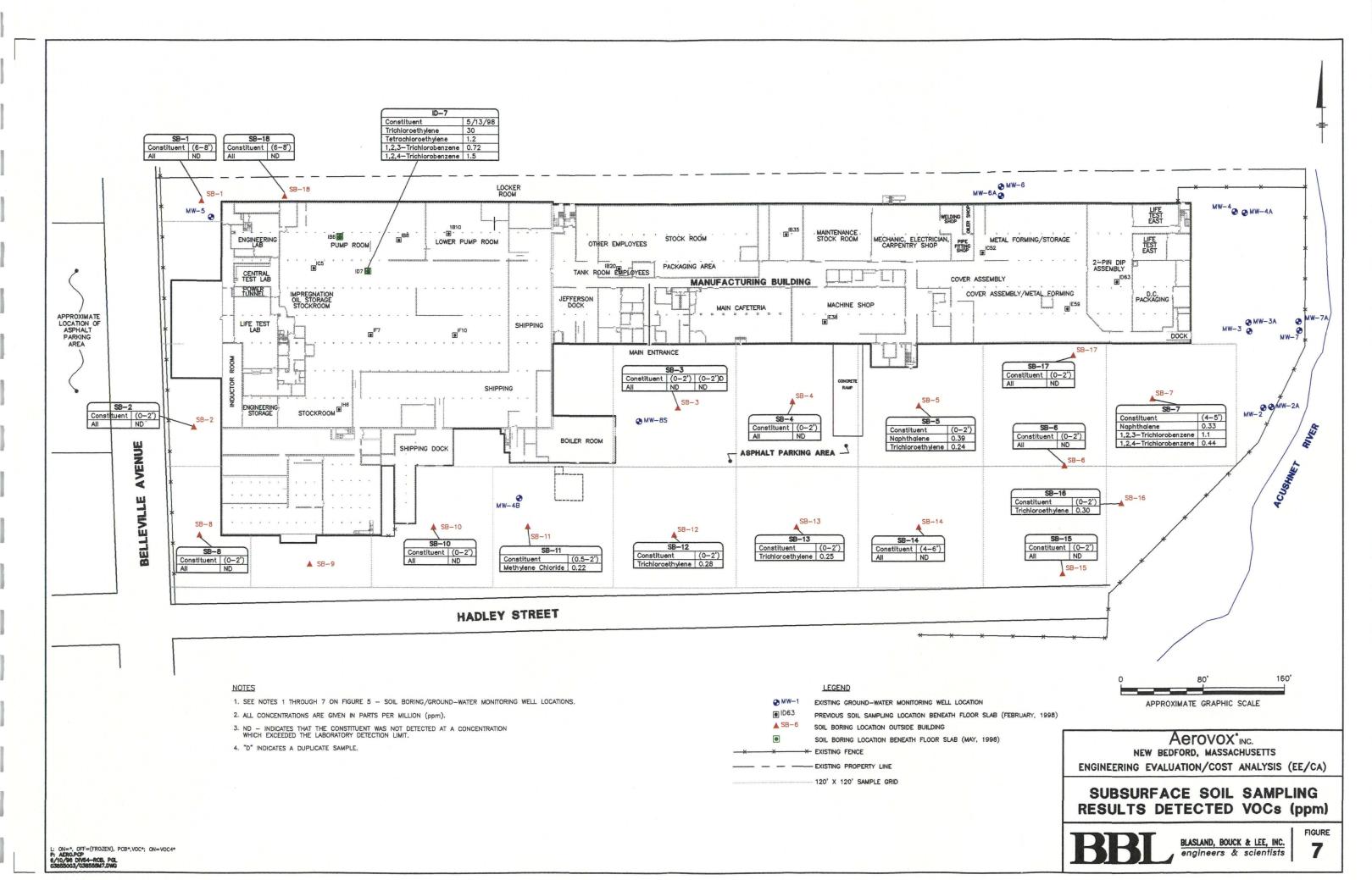
LOCKER ROOM WELDING 5 MECHANIC, ELECTRICIAN, CARPENTRY SHOP MAINTENANCE STOCK ROOM STOCK ROOM ENGINEERING LOWER PUMP ROOM OTHER EMPLOYEES PUMP ROOM PACKAGING AREA TANK ROOM EMPLOYEES CENTRAL TEST LAB 1000 COVER ASSEMBLY MANUFACTURING BUILDING POWER IMPREGNATION OIL STORAGE STOCKROOM JEFFERSON DOCK MACHINE SHOP MAIN CAFETERIA APPROXIMATE LOCATION OF ASPHALT PARKING LIFE TEST LAB SHIPPING AREA MAIN ENTRANCE RANIP SHIPPING INDUC ENGINEERING STORAGE STOCKROOM 6 1183--LIMITS OF EXCAVATION OF OIL IMPACTED SOIL BOILER ROOM AVENUE SHIPPING DOCK ASPHALT PARKING AREA -----REMAINS OF CONCRETE OIL CONTAINMENT BUNKER FOUNDATION . BELLEVILLE HADLEY STREET NOTES LEGEND 1. EXTERIOR AND INTERIOR BUILDING WALL LOCATIONS WERE OBTAINED FROM AN ELECTRONIC FILE (DRAWING NO. PAVXX-AG-0002, REVISION A, DRAWN BY D. JENKINS, DATED NOVEMBER 18, 1997) PROVIDED BY AEROVOX, INC. - EXISTING PROPERTY LINE 2. SITE FEATURES OUTSIDE THE BUILDING (INCLUDING FENCE, PROPERTY LINE, PARKING LOT, AND ROADWAYS) WERE DIGITIZED FROM A SITE PLAN AT A SCALE OF 1"=50' PREPARED BY INDUSTRIAL RISK INSURERS, DATED MAY B, 1992. ____ - SHEET PILING (APPROXIMATE LOCATION) 3. THE LIMIT OF THE FORMER SOIL EXCAVATION AT AND IN THE VICINITY OF THE CONCRETE OIL CONTAINMENT BUNKER FOUNDATION (WHICH FORMERLY SUPPORTED TWO 10,000 GALLON OIL STORAGE TANKS) WAS DIGITIZED FROM A DRAWING ENTITLED, "CONSTRUCTION SITE PLAN, SHORT TERM MEASURE, AEROVDX, INC.," PREPARED BY SAIC ENGINEERING, INC. AT A SCALE OF 1"=10', DATED JUNE 4, 1991. 4. LOCATION OF FENCE ALONG EAST PROPERTY LINE DETERMINED FROM FIELD OBSERVATIONS. X (XREF) LATERS OFF=0,CONCRETEFLOOR,FIG3,REF,SAMPLELOCATION,SCRAPES,SEALEDAREAS,WIPES,WOODLOT P: AERO,PCP 8/27/96 DIV54-RCB, PGL, RCB JMS 03855005/3038558W4

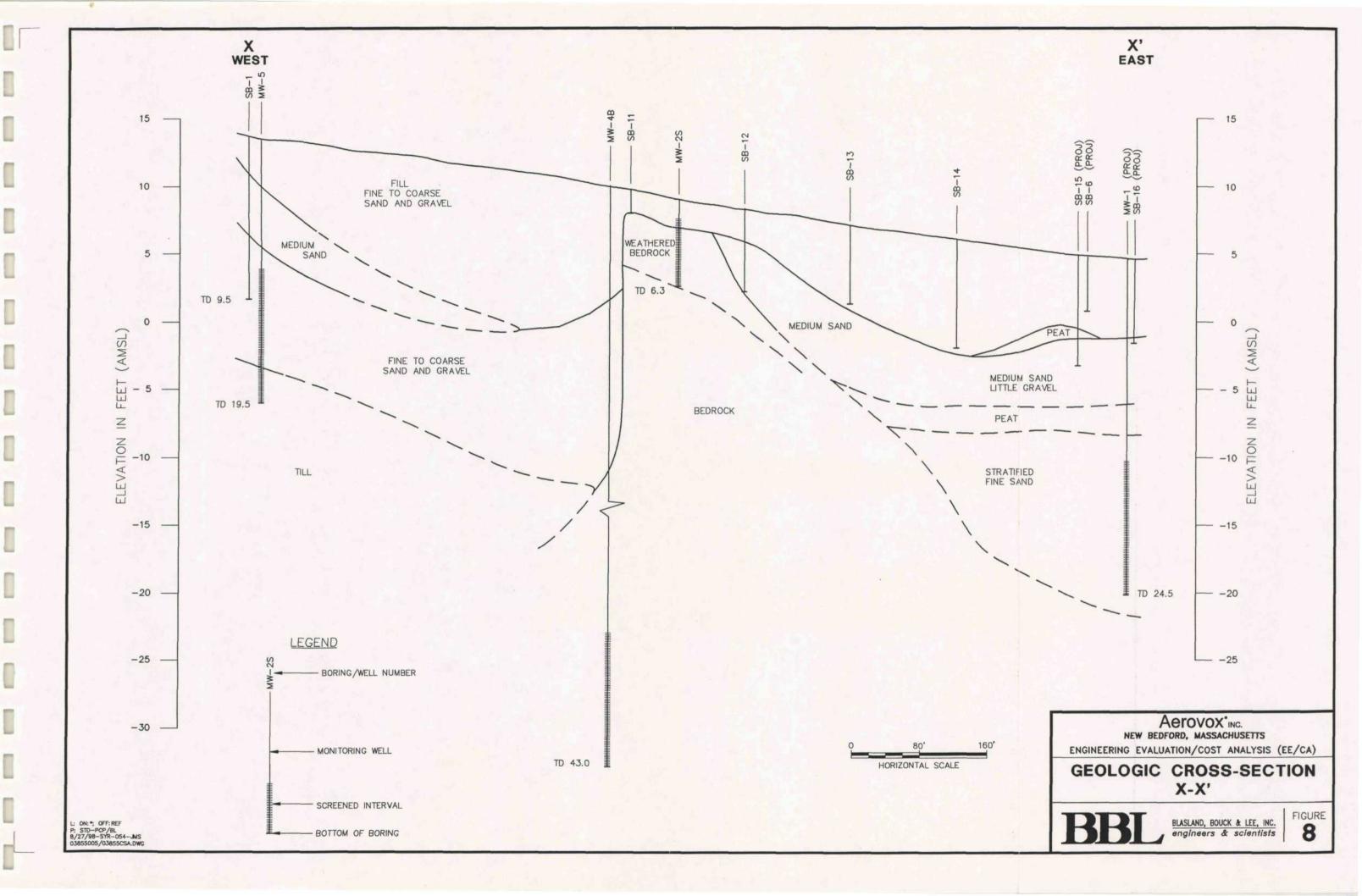






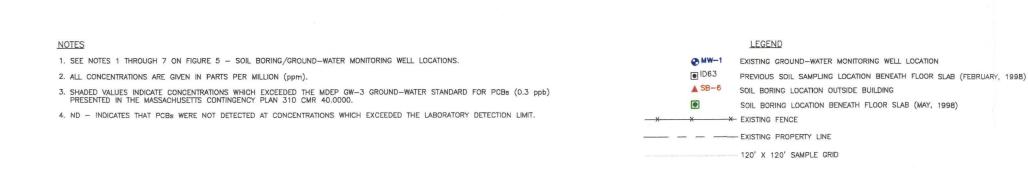


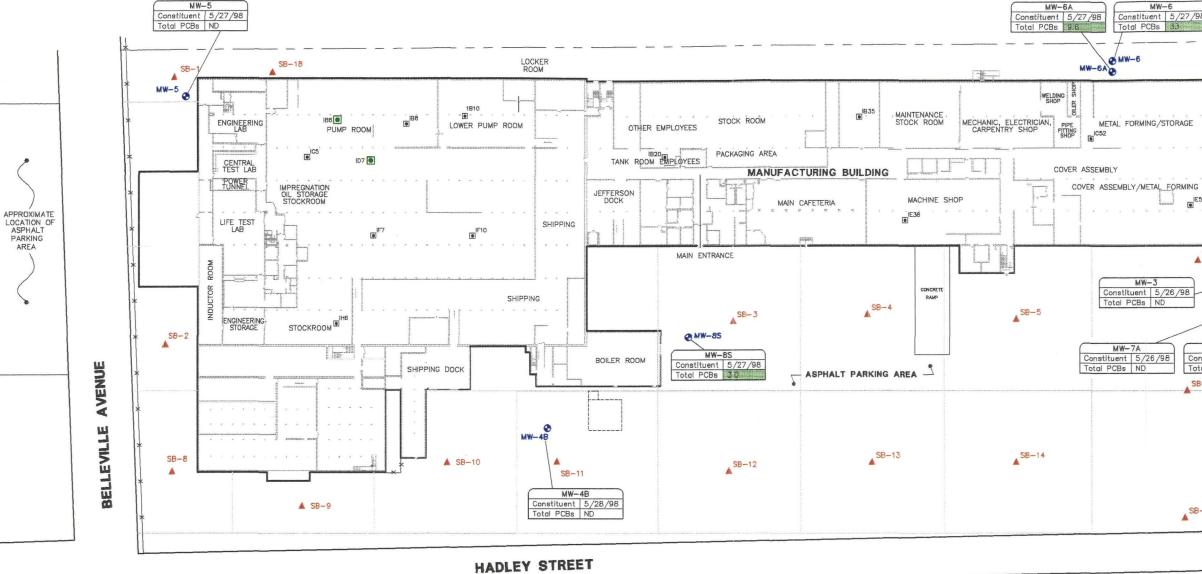


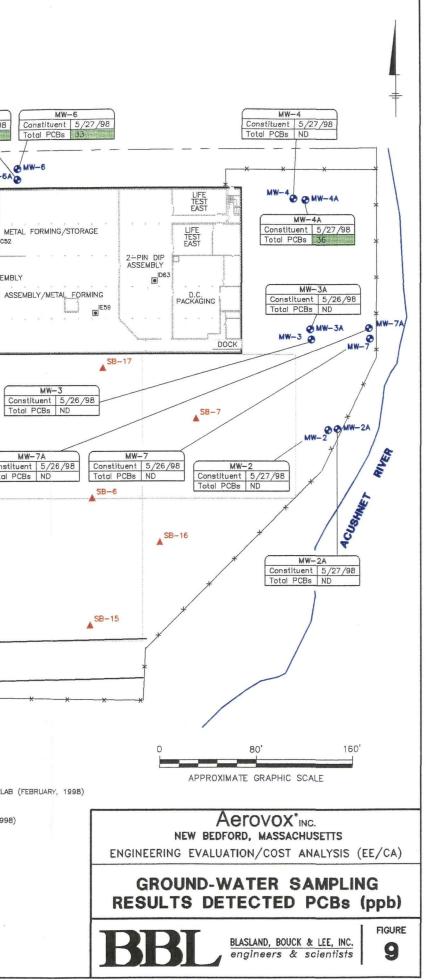


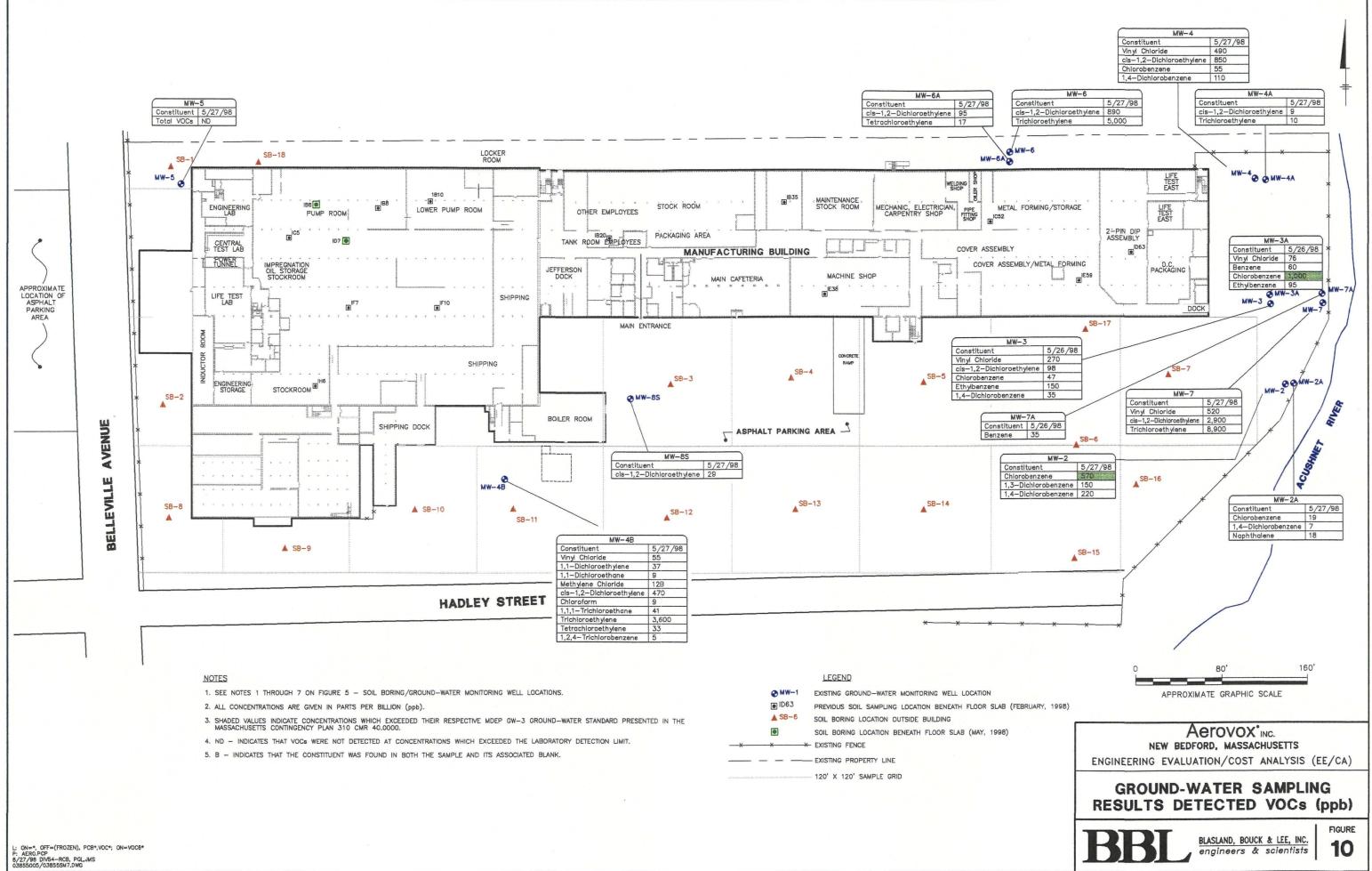
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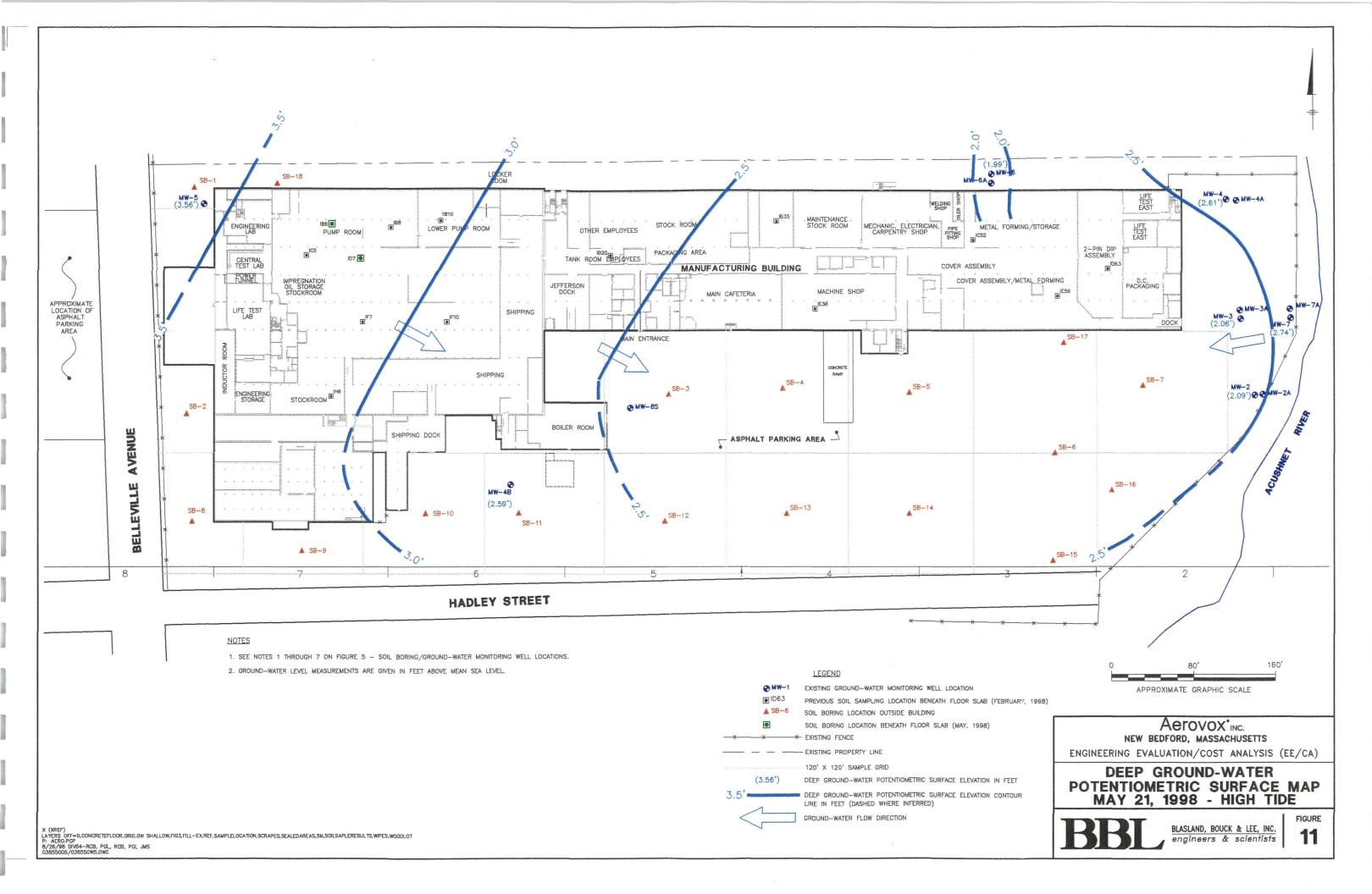
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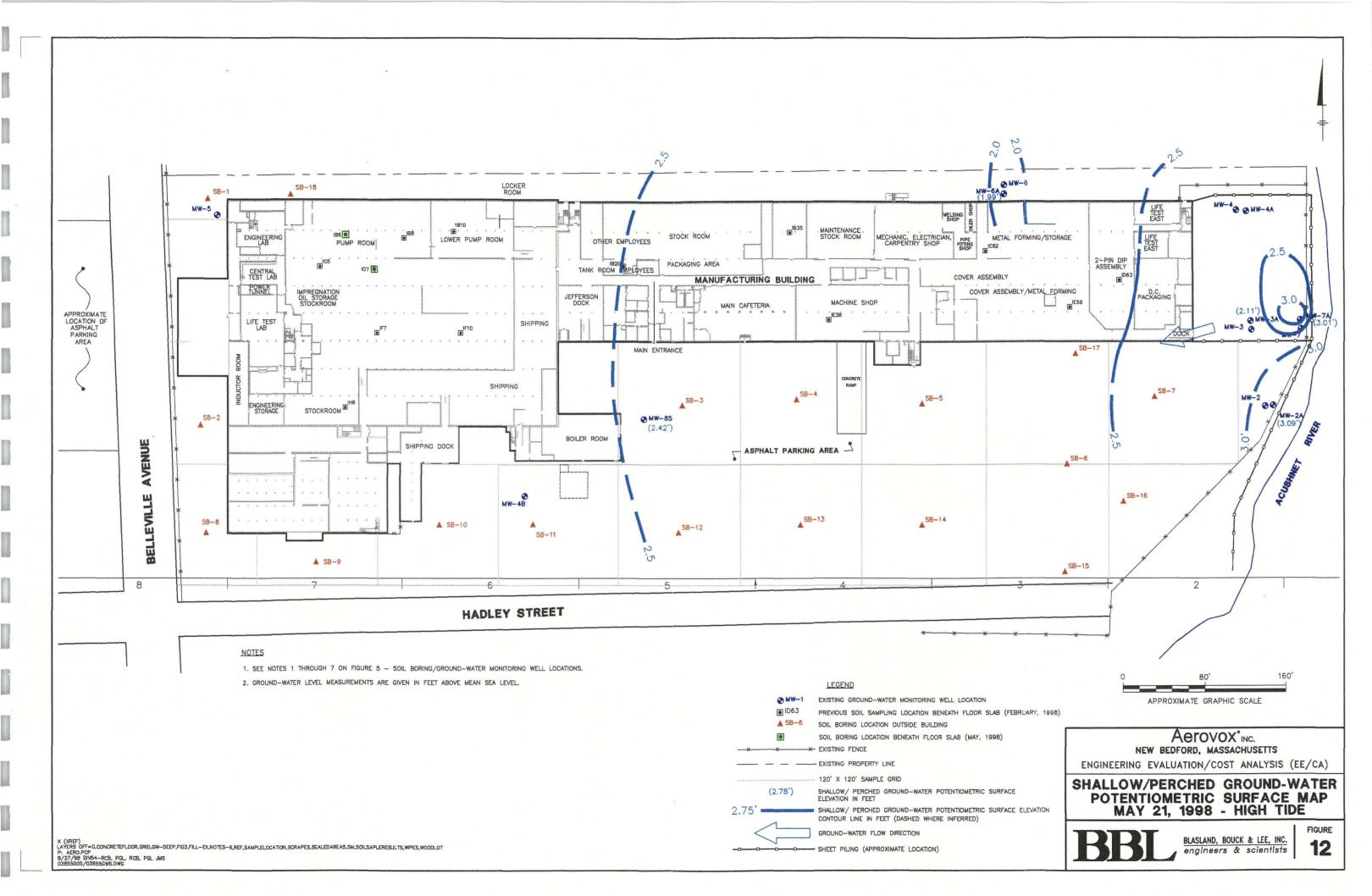


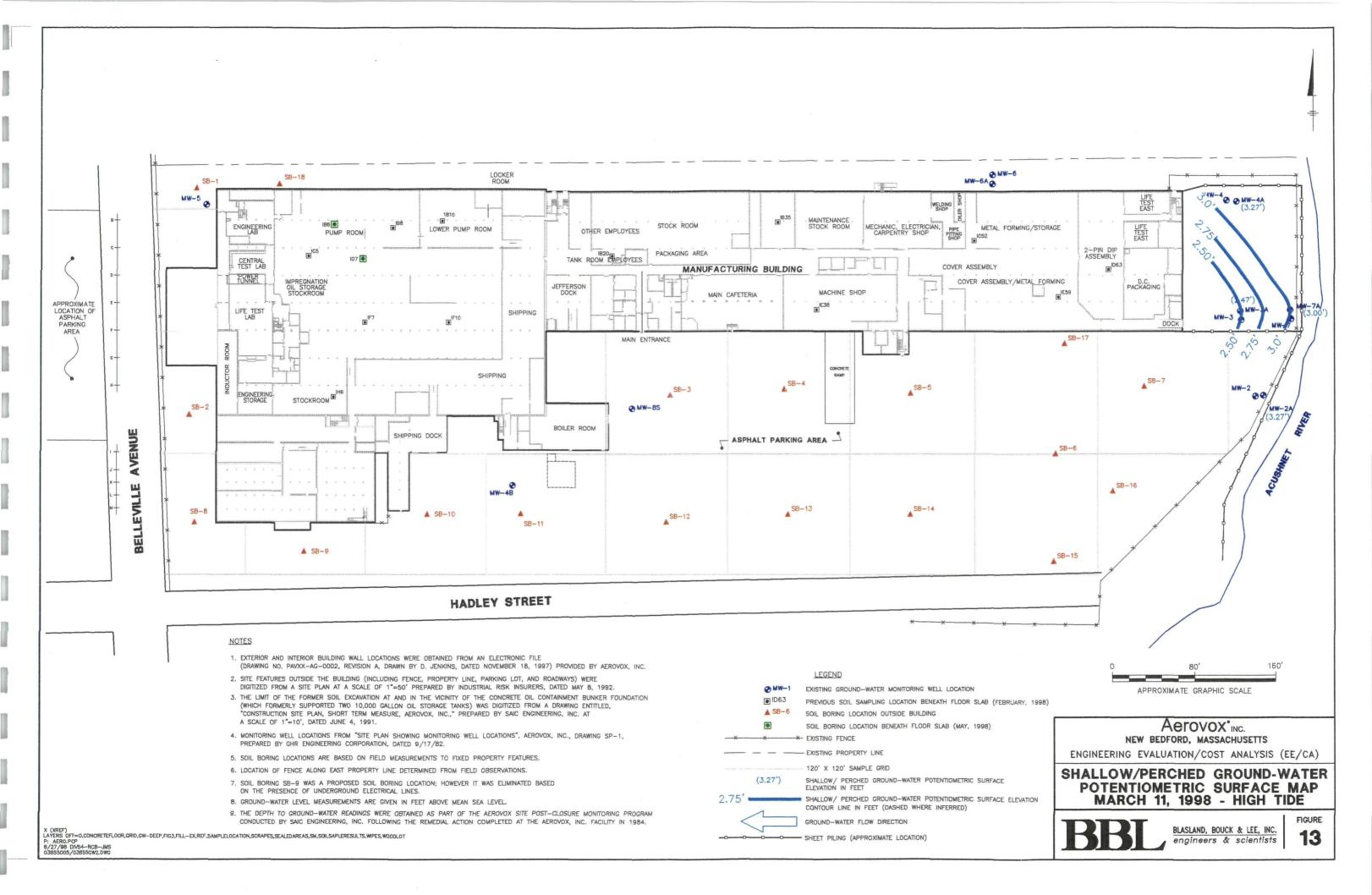




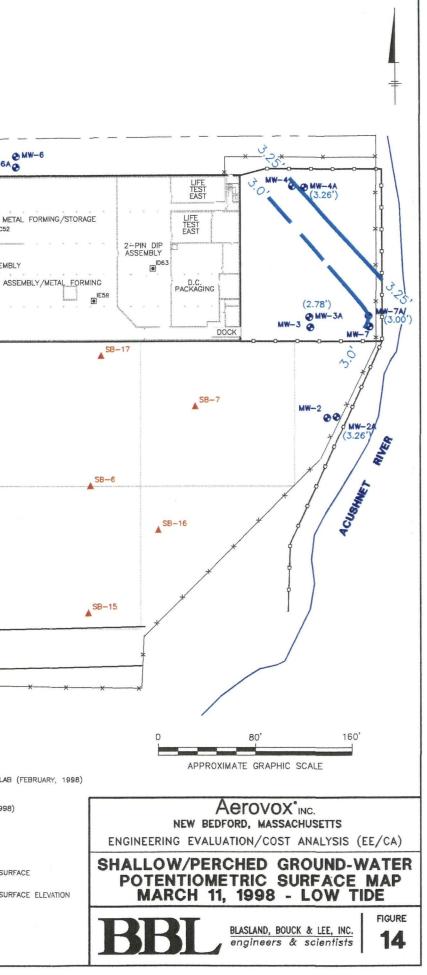




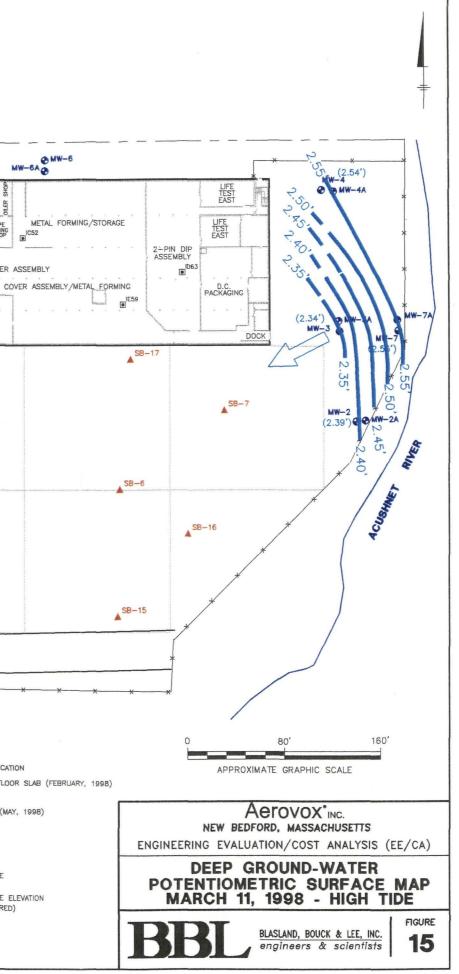


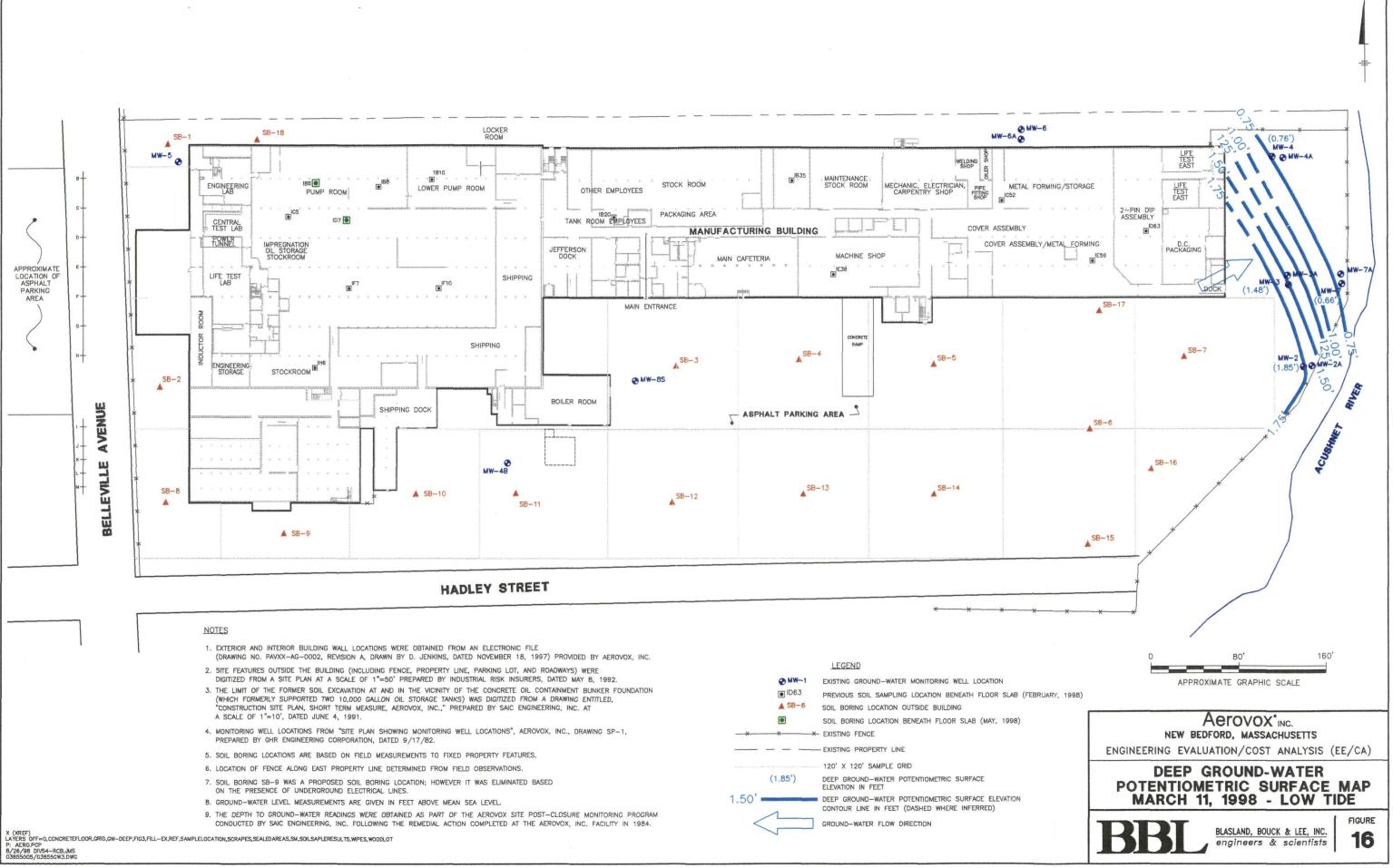


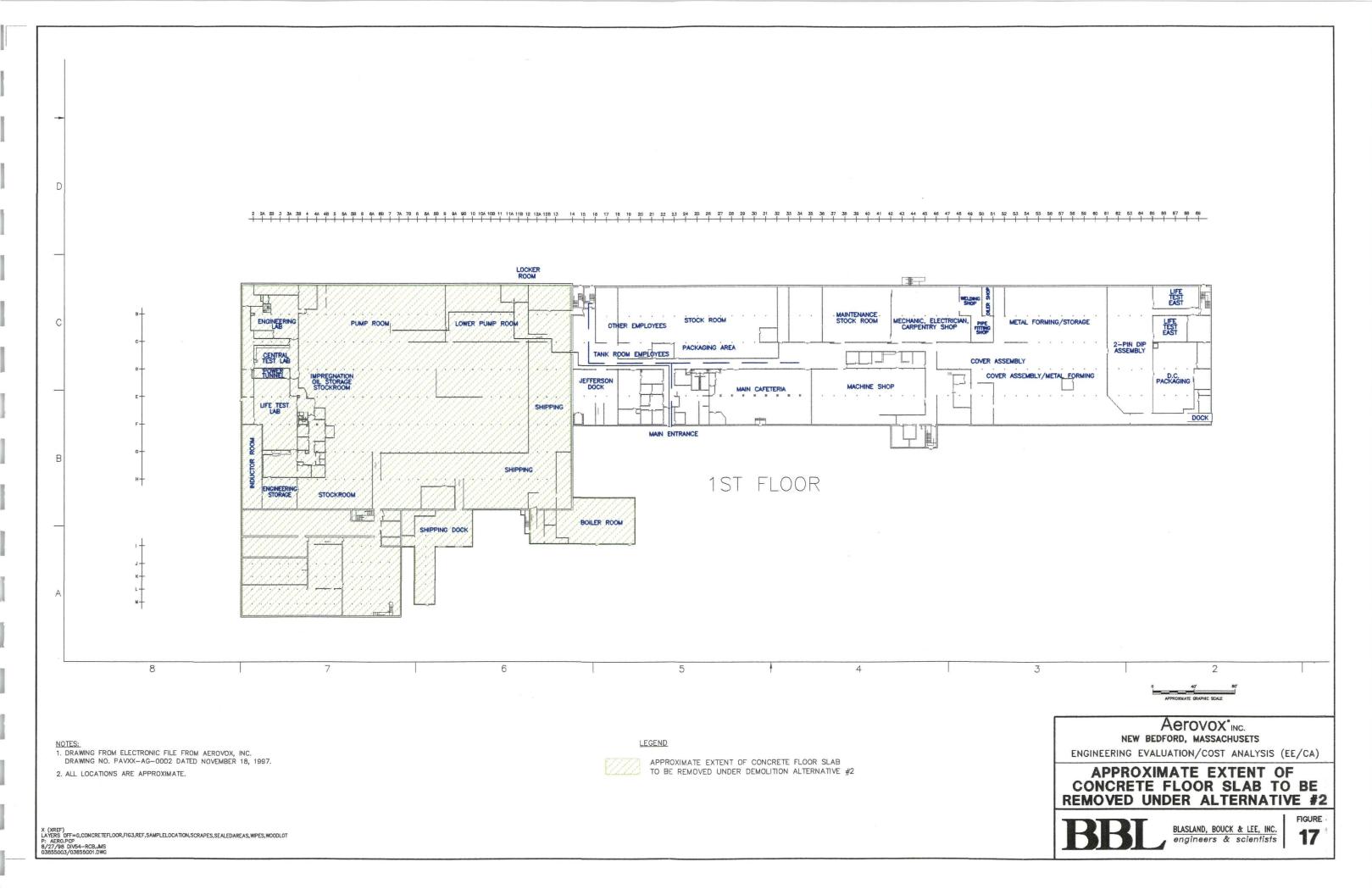
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Attachments

BLASLAND, BOUCK & LEE, INC.

engineers & scientists

Attachment 1

USEPA's Approval Memorandum

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UNITED STATES ENVIRONMENTAL PROTECTION AGENCY Region 1 J.F.K. Federal Building, Boston, MA 02209-2211

MEMORANDUM

- DATE: JUL 7 1998
- SUBJ: <u>Aerovox Incorporated</u> Site-Approval Memorandum to perform an Engineering Evaluation/Cost Analysis for a Non- Time Critical Removal Action
- FROM: Marianne Milette, Senior Enforcement Coordinator 77-9 Kimberly Tisa, PCB Enforcement Coordinator
- TO: Patricia Meaney, Director Office of Site Remediation and Restoration

Ira Leighton, Acting Director Office of Environmental Stewardship

This memorandum recommends that you authorize the preparation of an engineering evaluation/cost analysis (EE/CA) for a non-time critical removal action (NTCRA) at the Aerovox Site in New Bedford, Massachusetts. The EE/CA will evaluate cleanup alternatives for source control measures at this Site. The EE/CA will be prepared by Aerovox, Inc., under EPA oversight. No federal funds will be expended in the preparation of the EE/CA.

This memorandum is not a final Agency decision regarding the selection of a response action for the Site. The Superfund decision making process for this Site will proceed as follows:

- NTCRA (Source Control)
- -- Sign Approval Memorandum to initiate EE/CA
- -- Finalize EE/CA and prepare Fact Sheet of proposed action
- -- Conduct 30 day comment period
- Select the NTCRA in an Action Memorandum and respond to comments
- -- Implement NTCRA through AOC with Aerovox, Inc.,

I. <u>Site Description and History</u>

The Aerovox Site (the Site) is located on an approximately 10 acre parcel at 740 Belleville Avenue in New Bedford, Massachusetts (see Attachment 1). The Site contains an approximately 450,000 square foot manufacturing building which has been used to produce film, paper and aluminum electrolytic capacitors. A parking lot is located south of the manufacturing building. Aerovox, Inc. and various predecessor companies have occupied the site for over 80 years. During 1995, Aerovox, Inc. purchased a small parcel located west of the original property (on the opposite side of Belleville Avenue) which has been used for additional parking space. The Site is located within a highly developed urban/industrial area of New Bedford, Massachusetts. The Acushnet River borders the Site to the east. The ground surface at the Site slopes gently from the west to the east. The elevation along Belleville Avenue at the west edge of the original property is approximately 14 feet above mean sea level (MSL) while the elevation toward the eastern edge of the Site (prior to reaching a seawall constructed along the bank of the Acushnet River) is generally between 4 and 7 feet above MSL. A chronology of significant events related to the Site is detailed below:

1982 Consent Order entered into by Aerovox, Inc., with the USEPA under Section 106 of CERCLA. A similar Consent Order was entered into by Aerovox, Inc. with the Massachusetts Department of Environmental Quality Engineering ("DEQE" now known as the "MADEP") at the same time. A site investigation was conducted pursuant to the Consent Orders. The investigation focused on an unpaved area at the eastern end of the site bordering the Acushnet River and an unpaved strip of land to the north of the manufacturing building. The results of the investigation indicated that PCBs were present in soil at concentrations exceeding 50 ppm and PCBs were also present within the shallow, perched ground-water system at the site.

1983 -

- 1984 As a result of the above investigation, construction of the final remedial action consisting of capping the impacted soil areas (by paving with hydraulic asphalt concrete) and installing a steel sheet pile cutoff wall to serve as a vertical barrier to ground water and tidal flow into and out of the impacted soils.
- 1988 Removal of two 10,000 gallon No.6 fuel oil storage tanks and one 250 gallon condensate collection tank from a former concrete oil containment bunker located south of the manufacturing building boiler room. Assessment of soil and ground water in the vicinity of the former concrete oil containment bunker. A Notice of Responsibility Letter was issued by the DEQE to RTE Aerovox, Inc., for additional assessment and evaluation of remedial measures.

- 1990 Removal of petroleum product and water from the concrete oil containment bunker, excavation of petroleum-impacted soils for on-site treatment and recycling into an asphalt base course for the parking lot, construction of an oil-water separator to control and recover floating petroleum product and post-construction monitoring of the oil-water separator system. The MADEP determined that no further remedial action was necessary for this matter by a letter dated July 26, 1993.
- 1997 Inspection of the manufacturing building conducted by the USEPA and involving the collection of wood shaving samples from floor areas inside the manufacturing building and collection of oil samples from various oil storage tanks/degreaser operations for PCB analysis. The data indicated the presence of PCBs in the wood floor samples at concentrations exceeding 50 ppm. PCBs were not detected above laboratory detection limits in the oil samples collected from tanks/equipment at the Aerovox, Inc., facility.

As a result of EPA's findings, Aerovox, Inc. contractors, East Coast Engineering, Inc. and Cistar Associates, conducted additional building material and air monitoring investigations. The data collected indicated the presence of PCBs throughout the facility.

II. Nature and Extent of Contamination

Based on the 1997 investigations, Blasland, Bouck & Lee, Inc (BBL), contractor for Aerovox, Inc., conducted additional sampling of building materials ie., full-core building material samples (wood, brick, and concretc), composite scrape samples of dust/dirt from elevated horizontal surfaces, wipe samples from non-porous building material surfaces (tile floor, painted walls, steel surfaces), and wipe samples from equipment. BBL also conducted soil sampling activities beneath the concrete floor slab of the manufacturing building and beneath the asphalt parking areas surrounding the building and ground water sampling. The results of all 1997 and 1998 investigations are summarized below:

Building materials (wood, brick, concrete, etc.):

The analytical results indicate that PCBs at concentrations of greater than 50 ppm were present in the wood floors, concrete floors, dust and dirt scrape samples. Analytical results indicate PCBs were detected in full core samples collected from the brick exterior walls and wood ceilings. Analytical results of wipe samples collected from non-porous building materials, appurtances and equipment contained PCBs at concentrations greater than 10 ug/100cm².

Soil samples:

Beneath the building:

The analytical results indicate that PCBs at concentrations up to 18,000 ppm were present. VOCs were detected between 0.7 ppm and 30 ppm.

Underneath the asphalt parking lot:

The analytical results indicate that PCBs at concentrations up to 2,900 ppm were present. VOCs were detected between 0.22 ppm and 1.1 ppm.

Ground water sampling:

The analytical results indicate PCBs up to 36 ppb were present. VOC's were detected up to 5,000 ppb.

Air Sampling:

Data indicated the presence of PCBs in the air samples at concentrations exceeding 0.001 mg/m^3 inside the building.

PCBs are the contaminant which may pose a potential threat to human health or ecological health based upon the above field investigations.

Tables 1 and 2 summarized the potential human health risk associated with the site.

C	CALCULATION OF NOI INGESTION AND DEF	MAL EXPOSURE
EXPOSURE POINT Reasonable maximum e	CONCENTRATION xposure (RME), µg/cm ²	HAZARD INDEX (RME)
Tank room operator	2.71	25.7
Carpenter	2.05	39.0
Pump room operator	5.986	113.7

TABLE 1

TABLE 2 CALCULATION OF CANCER RISK INGESTION AND DERMAL EXPOSURE

EXPOSURE POINT	CONCENTRATION exposure (RME), µg/cm ²	CANCER RISK (RME)
Tank room operator	2.71	5E-04
Carpenter	2.05	7E-04
Pump room operator	5.986	1E-03

III. Endangerment Determination

Actual or potential release of PCBs from this Site may present an imminent and substantial endangerment to public health or welfare or the environment. A removal action is therefore appropriate to abate, prevent, minimize, stabilize, mitigate, or eliminate such threats. In particular, a removal action is necessary to control or contain the release of hazardous substances from the Site through source control measures.

IV. Basis for EE/CA and Non-Time Critical Removal Action

Section 300.415(b)(2) of the National Contingency Plan (NCP) lists a number of factors for EPA to consider in determining whether a removal action is appropriate, including:

- (i) Actual or potential exposure to nearby human populations, animals, or the food chain from hazardous substances or pollutants or contaminants;
- (iv) High levels of hazardous substances or pollutants or contaminants in soils largely at or near the surface, that may migrate;
- (vi) Threat of fire or explosion;
- (viii) Other situations or factors that may pose threats to public health or welfare
 of the United States or the environment.

The above conditions for a removal are met at this Site. The building occupants have actual or potential exposure. The potential non-cancer risk for workers exceeds the hazard index of 1 while the cancer risk ranges from $10^{-3} - 10^{-4}$. The potential for tracking of the contamination to off-site areas also exists. Should the building become vacant with no security measures the threat of fire increases.

This removal is designated as <u>non-time critical</u> because more than six months planning time is available before on-site activities must be initiated. Prior to the actual performance of a non-time critical removal at this Site, Section 300.415(b)(4) of the NCP requires that an engineering evaluation/cost analysis (EE/CA) be performed in order to weigh different response options.

V. <u>Scope of the EE/CA</u>

The purpose of the EE/CA will be to evaluate alternatives for source control response measures at the Site. The EE/CA will consider alternatives which meet the following removal action objectives:

- Prevent, to the extent practicable, direct contact with and ingestion of soil/dust/debris/structures within the building and in the soils beneath the footprint of the building and under the paved parking areas.
- Prevent, to the extent practicable, the potential for water to infiltrate through the soils;
- Control, to the extent practicable, surface water run-off to minimize erosion;
- Prevent, to the extent practicable, the release of pollutants or contaminants at levels that would represent an unacceptable human health exposure to a Site worker or trespasser; and
- * Remove soils/dust/debris/structures at levels that could result in an unacceptable ecological impact.

Pursuant to EPA guidance on EE/CAs, alternatives will be evaluated based upon effectiveness, implementability, cost, and compliance with ARARs. Further, alternatives which exceed \$2 million dollars will be evaluated to determine their consistency with future remedial actions to be taken at the Site.

In developing the range of alternatives to be evaluated in the EE/CA, EPA will consider 300.415(e) of the NCP as well as relevant guidance. Section 300.415 (e) of the NCP identifies various removal actions which may be appropriate in given situations, including:

- (1) Fences, warning signs, or other security or site control precautions where humans or animals have access to the release;
- (2) Drainage controls, for example, run-off or run-on diversion where needed to reduce migration of hazardous substances...;

- (4) Capping of contaminated soils or sludges where needed to reduce migration of hazardous substances or pollutants or contaminants into soil, ground or surface water, or air;
- (6) Excavation, consolidation, or removal of highly contaminated soils from drainage or other areas - where such actions will reduce the spread of the release; and
- (8) Containment, treatment, disposal, or incineration of hazardous materials where needed to reduce the likelihood of human, animal, or food chain exposures.

These alternatives and others may be evaluated in the EE/CA.

VI. <u>Other Considerations</u>

The current schedule is to have a final Administrative Order on Consent (AOC) for the Site signed by September 1998. If a non-time critical removal action were initiated, an Action Memorandum could be issued by November 1998, AOC negotiations would be conducted October - December 1998, and the removal action would commence by December 2000 and be completed by December 2003.

The State supports the proposed action at this Site.

VII. <u>Recommendation</u>

In light of the facts discussed above, the case team recommends that you approve the initiation of an EE/CA for this Site.

7/15/98

Mun

Patricia Meaney, Director / Office of Site Remediation and Restoration

Attachments:

- 1. Site Location Map
- 2. Risk Evaluation

Attachment 2

Field Notes - Soil Investigation Beneath the Concrete Floor Slab

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Attachment 3

Soil Boring Logs

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Drilling Co Driller's N Orilling Me Auger Size	e: ID 4.25 Acker AD e: 2 In.	vironii w Ste in.	ienta	l Ori		Inc.	··		Depth: 12 ft. Depth: 12 ft. Soli Boring Client: Aerovox In Location: New Bedfo st: Doug Ruszczyk	corporated
DEPTH EE EVA TRAN	Sample Run Number	Sample/Int/Type	Blows/8 In.		1 8	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description	Soll Boring Construction
	[0		1 3 7 2 2	10	0.8	۵.5			GROUND SURFACE Loose, Dark brown to black fine to coarse SAND, trace Silt and Gravel, dry. (Black discobration in F-2 interval). Loose, orange~brown, fine to coarse SAND, trace Silt and Gravel, dry.	
 5	(3)		3 5 2 3 3 3	8	12 10	0.0 0.1			Loose, tan fine to coarse SAND, trace Silt and fine Gravel, dry to damp. Dense, tan fine to coarse SAND, some fine to	
	(4)		4 18 22 19 17 20 18 19	40 38	10 0.7	11 0.1			nedium Gravel, trace Sit, damp. Dense, tan fine to coarse SAND, some tine to nedium Gravel, trace Sit, damp to moist.	
- D - - -	(8)		14 18 19 17	35	10	NA			Dense, tan medium to coarse SAND, some fine to medium Gravel, little fine Sand and Sit, wet.	
5 BLASS eng	BLAND, BOLCK		, INC			based	la h d ai	ead: h the	Dace measurement was obtained .	Saturated Zones e / Time Elevation Depth

Drilling Com Driller's Na Drilling Met Auger Size Rig Type: J Spoon Size	me: hod: Hollos : ID 4.25 (Acker AD 1	w Ste n.			ling				Depth: 5 ft.	it: vox Incorporated ition: Bedford, MA.
DEPTH	Sample Run Number	Sample/Int/Type	Blows/8 In.	Z	Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description	Soll Boring Construction
			4					•	GROUND SURFACE Asphalt Nedlum, orange-brown fire to coarse SAND, lit	
- -	(1)		6 6 3 5 8 11 23	12	10 0.5	0.0			Nedium, orange-brown, file to coarse SAND, in fine Gravet, trace Silt, dry. Nedium, orange-brown, file to coarse SAND, s file to coarse Gravel, dry to damp.	
- - 5 -								<u>}</u>	Refusal. Advanced augers to 5 ft. cutting thr gneissic schist.	ough
-										
a-										
				i						
5		21				Remark	(9:			Saturated Zones Date / Time Elevation Dep

Drilling Comp Driller's Nam Grilling Meth Auger Size: Alg Type: A Spoon Size:	ie: ID 4.25 li cker AD I	w Ste n.			ling	B¢			Depth: 4 ft.	vox Incorporated
DEPTH ELEVATION	Sampje Run Number	Sample/Int/Type	Blows/6 In.	z	Recovery (ft.)	PID Headspäce	Geotechnical Test	Geologic Column	Stratigraphic Description	Soll Boring Construction
	(9		17 15 8 3	23	10	0,1		• •	GROUND SURFACE Asphalt Nedium, dark brown to black fine to coarse SAN trace Silt and Gravel, dry to moist.	0,
	(2)		3 2 1	4	0.7	NA .			Loose, brown/black fine to medium SAND, frace and Gravel, wet (2.0' to 2.4') Loose, brown/black PEAT, wet. (2.4' to 4.0')	Sit .
-										
-D										
5	2					Remark	11 11		pace measurement was obtained	Saturated Zones Date / Time Elevation Dep
	N), BOUCK	c ten		7		NA: N basec	t or	the	pace measurement, was dotained presence of saturated soll.	Page: 1

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	*** . 	[]					Ge	olog	Depth: 4 ft. Location: New Bedford, MA. Ist: Doug Ruszczyk
DEPTH		Sample/Int/Type	Blows/6 In.		Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description Construction
	(0		4 5 5 4	10	10	825			GROUND SURFACE Asphalt Loose, fan/brown/black fine to coarse SAND, little Gravel, trace SII, wet/oily appearance in 1' - 2' Interval, dry to damp.
- - - 5	(2)		4 7 6 5	13	0.7	3.5		•	Nedium, black fine to coarse SAND, some Gravel, trace Silt, damp to wel.
- - -									
-D									
						Remari			Saturated Zones

Date Stari Drilling Cor Driller's Na Drilling Mei Auger Size Rig Type: Spoon Size	npany: Env ame: thod: Hollo I: ID 4.25 (Acker AD)	vironm w Ste in.	ental	Orill		Inc.			Depth: 6 ft. I at: Doug Ruszczyk	Soll Boring No: SB-5 Client: Aerovox Incorporated Location: New Bedford, MA.	
DEPTH ELEVATION	Sample Run Number	Sample/Int/Type	Biows/8 In.		Recovery (ft.)	PID Headspace	eotechr	Leologic Column	Stratigraphic Description	Soil Boring Construction	
	[9		4 5 5	10	15	25	•	•	GROUND SURFAC Asphalt Loose, brown/black fine to coarse S Gravel, little brick and glass, trace S	AND, some	
	[2]		8 4 3 4 5 4	6	12	۵٥	0	0.00	Loose, brown/black fine to coarse S GRAVEL, trace Silt, damp to moist. Loose, brown/black fine to coarse S GRAVEL, trace Silt, wet.		
- 5			65	0	0.7	NA	0.0	⊙			
Ð											
B BLASL Engl	BL AND, BOUCK neers 6 s	SI S LEE	INC:			Remark NA: N based	lo hea	ds; he	pace measurement was obtain presence of saturated soil.	ed Date / Time Elevation	

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Driller Orillin Auger Rig T	r's Nan g Meth r Size:	od: Hollo ID 4.25 (cker AD)	w Ste n.			ind.				Depth: 4 ft. I st: Doug Rus;	zczyk	Cilent: Aerovox II Location: New Bedfo		ed	
DEPTH	ELEVATION	Sample Run Number	Sample/Int/Type	Blows/G In.		Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Calumn		Stratigraphic Description		C	Soll Boring Constructio	ີ
		[1]		15 7 4 3	1	14	3.5		•	Asphalt Nedium, brown/t some Gravel, tra	DUND SURFA Diack/tan fine to co ace Silt, dry to mois O' to f interval)	arse SAND,			
- - - 5		(2)		1 (1	2	0.3	NA			Loose, black/ta to coarse Sand	n fine to medium GR , trace Clay and Sili	AYEL, some fine , wet.	-		
-															
-10															
5 11,225							Remar	ks:						ated Zon	
	BLASLA Englin	S. BOLCK	rc ien	INC	9		NA: N	io h	ead: i (he	pace measurem presence of se	ent was obtai sturated soll	ned Da	te / Time	Elevation	Dep

Drilling Com Driller's Nar Drilling Meth Auger Size: Rig Type: A Spoon Size	ne: nod: Hollow ID 4.25 k icker AD 1	w Ste n.:			IIIng				Depth: 6 ft. Location: New Bedford, MA.
DEPTH ELEVATION	Sample Hun Number	Sample/Int/Type	Blows/8 In.	N.	Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Soil Boring Description Construction
	(1)		5 6 5 7	fí	tr	0.2		•	GROUND SURFACE Asphalt Nedium, brown/black coarse SAND, Ittle Gravel, trace Silt, dry to damp. (Black discoloration 1' - 2' hterval)
	(2)		10 7 6 11	ß	0.0			•	No recovery. Loose, brown/black PEAT (4.0' to 4.3').
5 	(3)		15 7 1 1	8	0.7	0.5		•	Loose, brown/black coarse SAND, little gravel, wet.
10									
-									
	BL ND, BOUCK	S 8 LEE	, INC			Remarl	K8:		Saturated Zones Date / Time Elevation De

Drilling Auger Rig Ty	Size:	od: Hollov ID 4.25 li sker AD I	٦.	m Au	ger	- 				Client: Aerovox Depth: 10 ft. Location: New Bed Ist: Doug Ruszczyk		≩d	
DEPTH	ELEVATION	Sample Run Number	Sample/Int/Type	Blows/8 In.		Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description		Soil Boring Constructio	תכ www.son texters
										GROUND SURFACE			
		(1)		4 8 7	и	13	3.1		•••	Asphalt Nedium, orange-brown to tan, fhe to coarse SAND, little fine to Nedium Gravel, trace Sit, dry.			
-		(2)		11 13 15 12	28	0.8	0.0			Medium, orange-brown, the to medium SAND, some tine to medium Gravel, dry.			
- - 5	-	(3)	$\overline{\ }$	15 20 25 19	45	LI	0.9			Dense, orange-brown, fine to medium SAND, some fine to medium Gravel, dry.			
-		[4]		18 25 32 28	57	12	0.1			Very dense, orange-brown, fine to medium SAND, some fine to medium Gravel, dry to damp.			
-0		(5)		10 21 48 34	89	0.4	NA		0.0.0.	Very dense, tan nediun to coarse SAND and nediun to coarse GRAVEL, wet.			
			2										
5					ا مارتخە قار مەرتخە		Remar	() () ()	2 2 13 2		Saturi	ated Zone	28
	BLASLAN	D, BOUCK					NA: N	a hi	eads		ate / Time	Elevation	Dep

Auger Si Rig Type	Name: lethod; Holl ze: ID 4.25 :: Acker AD lze: 2 In.	in.	m Au	ger			 	• .	Depth: 6 ft. Location: New Bedford, MA.
DEPTH	ELEVATION Sample Run Number	Sample/Int/Type	Blows/8 In.	N	Recovery (ft.)	PID Heâdspace	Geotechnical Test	Geologic Column	Stratigraphic Description Construction
			25						GROUND SURFACE
-	(1)		35 40 18 11 7 5 5	75 12	0.9 10	0.0			Very dense, brown/black/tan fine to coarse SAND, some fine to medium Gravel, dry. Nedlum, orange-brown/tan fine to medium SAND, little Gravel, dry to moist.
— 5 — 5	[3]		8 7 8 7	15	0.2	NA	-		Medium, orange-brown/tan fine to medium SAND, little Gravel, wet
-									
- 10 -				Ĩ				3	
- -									
5 8	BI SLAND, BOUCK	S	INC			Remari NA: N Dased	a hi t an	eads the	pace measurement was obtained presence of saturated soil.

Drilling Met Auger Size Rig Type: Spoon Size	: ID 4.25 Acker AD	in. II							Depth: 3 ft. Loca New I	Aerovox Incorporated Location: New Bedford, MA.				
DEPTH	Sample Run Number	Sample/Int/Type	Blows/8 In.		Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description	Soil Boring Construction				
	(1)		5 8 10 50/ 0.1	18	10	112		• • •	GROUND SURFACE Asphalt and Cobbles Nedium, brown/black/tan, tine to coarse SAND, some fine to medium Gravel, Rock at tip of spot dry. Refusal, possible top of rock. Augers advance 3 feet returning fragments of gneissic schist.	Dr.				
 5 														
 D														
-														

Driller's Na Drilling Met Auger Size Rig Type: A Spoon Size	h <mark>od:</mark> Hollo : ID 4.25 Acker AD]	n.	m Aug	ger				ŝ	Depth: 8 ft. Ist: Doug Ruszczyk	Aerovox Incorporated Location: New Bedford, MA.				
DEPTH	Sample Run Number	Sample/Int/Type	Blows/8 In.	N	COV	PID Headsbace	Geotechnical Test	Geologic Column	StratIgraphic Description		Soll Bo Constru			
-	()		4 9 13 15	22	15			•	GROUND SURFA(Asphalt Medium, dark brown/black to orange coarse SAND, little Gravel, trace SI discoboration in 0' to 1 Intervall	-brown fine to				
- - 5	(2)		12 18 18 20 20 21 14	36 35	14	0.0			Nedium, orange-brown to tan tine to trace Silt, dry to damp. Nedium, orange-brown to tan tine to trace Silt, Rock at tip of spoon, dam	nediun SAND,				
-			17				_		Refusal, with gneissic schist rock fra spoon, wet.	ignents in				
- D -														
- -														
5 BLASU Engli	BL ND, BOLCK	S G LEE Clen	- INC			Rema	irks:			Da	Saturated 2 te / Time Eleva	Zones ition Dep		

Driller Drilling Auger Rig Ty	'a Nam Meth Size:	od: Hollo ID 4,25 sker AD	w Ste					•	• .	Depth: 6 ft. Ist: Doug Ruszczyk	Locati	ox Inc on:	orporate d, MA		، پرینی کرینی کرینی
QEPTH	ELEVATION	Sample Run Nymber	Sample/Int/Type	Blows/8 In.	Z	Recovery (ft.)	PID Headspace	Geotechnical Jest	Geologic Column	Stratigraphic Description			100 000000 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	Soil Boring Constructio	
				10						GROUND SURFAC	E				
-		(1)		10 10 13 10 10 10 10 10 10 10 10 10 10 10 10 10	21 8	14 11	0.0 0.0			Nedium, black to brown fine to coars fine to medium Gravel, trace Silt, dry discobration in O' to f Interval) Loose, orange-brown fine to coarse Silt and fine Gravel, dry to moist.	. (Black				
5 5		(3)		4 7 5 3 5	8	۵.7	NÅ			Loose, orange-brown fine to coarse Silt and fine Gravel, wet.	SAND, trace				
	i														
-10															
		:													
6			31		19 19 19 19		Remar	No he	ads	pace measurement was obtain presence of saturated soll.	ed -	Date	Satura / Time	sted Zone Elevation	

Date Start/ Drilling Comp Driller'a Nam Orilling Meth Auger Size: Alg Type: Al Spoon Size:	oany: Env ie: od: Hollo ID 4.25 sker AD]	vironm w Ste :n.	ental	Orill		Inc.			Depth: 8 ft. Ilst: Doug Ruszczyk	Soll Boring No: SB-14 Client: Aerovox Incorporated Location: New Bedford, MA.			
DEPTH	Sample Aun Number	Sample/Int/Type	Blows/G In.	N The second sec	Recovery (ft.)	PID Headspace	Geotechnical Test	Geolagic Column	Stratigraphic Description	,	Soll Boring Construction		
	[1]		11 10 9 7	19	0.4				GROUND SURFA(Asphalt Nedium, black/tan, medium to coarse Gravel, trace Sit, dry.				
	(2)		5 2 1 1 5 1 5	3	0.1				Loose, black, medium to coarse SAN trace Silt, damp. Loose, dark brown/black five to coa Gravel, trace Silt, damp to moist.				
	[4]		9 4 8 7 5	5	13	NA			Nedium, dark brown/black fine to co little Gravel, trace Silt, wet.	urse SAND,			
- D													
	D. BOUCK					NA: N	la hi ba le hi	eads sed	pace measurement was obtain on the lack of sample recover pace measurement was obtain presence of saturated soil.	ίευ 'γ.	Saturated Zones te / Time Elevation Depth		

Drilling Driller' Drilling Auger Rig Ty	g Comp 'a Nam g Methi Size:	od: Hollo ID 4.25 i cker AD 1	vironm w Ste in.	entai	i Drii		Inc.		. :	Depth: 8 ft.	Soli Boring No: SB—15 Client: Aerovox Incorporated Location: New Bedford, MA.				
DEPTH	ELEVATION	Sample Run Number	Sample/Int/Type	Blows/6 In.	Z	Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description			Soll Boring Constructio		
		(8		9	17	15	0.0			GROUND SURFACE Asphalt Nedium, black, medium to coarse SAND, some					
-		(1)		8 5 8 5 6 9	11 11	0.6	0.0			Gravel, trace silt, dry. Nedium, brown to black, medium to coarse SA some Gravel, liftle peat (3.5 to 4.0%), dry to	danp.				
- 5		(3)		9 8 7 2 4	8	0.1	0.0		•••••••••••••••••••••••••••••••••••••••	Nedium, black to brown, medium to coarse SAI AND GRAVEL, damp to moist. Nedium, brown to black, fine to coarse SAND, Gravel, weathered Rock at tip of spoon, wet.		f			
- 	ŀ	[4]		9 19 19	28	12	NA 		•••						
1) - -															
		BL D, BOUCK						Na hi	the	pace measurement was obteined presence of saturated soll.	Dat	Satur i e / Time	ated Zono Elevation	<u> </u>	

Drilling Meth Auger Size: Rig Type: Al Spoon Size:	ID 4.25 I cker AD 1	ń.	m Au	ger		£			Depth: 6 ft. Ist: Boug Ruszczyk	Aerovox Incorporated Location: New Bedford, MA.				
DEPTH	Sample Run Number	Sample/Int/Type	Blows/6 In.	z	Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description			Soli Boring Constructio		
									GROUND SURFAC	Ē				
-	(0	$\left[\right]$	8 9 5	17	10	4.9		•	Nediun, brown/red/black coarse SAN and Brick, trace Sill, dry. (Black dise to 2 Interval)), little Gravel coloration in f	-			
	(2)	\square	5 5 4 1	9	0.7	0.0			Loose, brown/black coarse SAND and little fine to medium Sand, trace Sit, o moist	GRAVEL, Janap to				
- 5	[3]		5 12 15 23	27	0.8	NA		7000 40 C	Nedlun, brown/black, fine to nedium G nediun to coarse Sand, trace Sit, we					
-0														
			1											
-														
5											Satur	ated Zoni	28	
	3 -	3		ere No of			Na h	eads	pace measurement was obtain presence of saturated soil.	ed Dat		Elevation	-	

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Drilling Meth Auger Size: Rig Type: A Spoon Size:	ID 4.25 cker AD]	n.	im Au	ger		B			Depth: 8 ft. Location: New Bedford, MA.
DEPTH ELEVATION	Sample Run Number	Sample/Int/Type	Blows/6 In.	N	Recovery (ft.)	P1D Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description Construction
	(0		17 8 5 7	13	13	L3		•	GROUND SURFACE Asphalt Nedium, tan/brown/black, fine to coarse SAND, some fine to medium Gravel, trace Sit, dry. (Black discoloration in f to 2 interval)
	[2]		2 2 2 2 2	4	0.0			• •	No recovery. Loose, brown/black PEAT, little fine to coarse Sand, trace Gravel, dry to nuclst. (Peat & 4.3 ft.)
5 	(3)		1 1 1 2 1 3	2	20	0.0 NA		a ver ver ver ver	
10			2					U~	
-									
						Remar	1.212		pace measurement was obtained Date / Time Elevation De

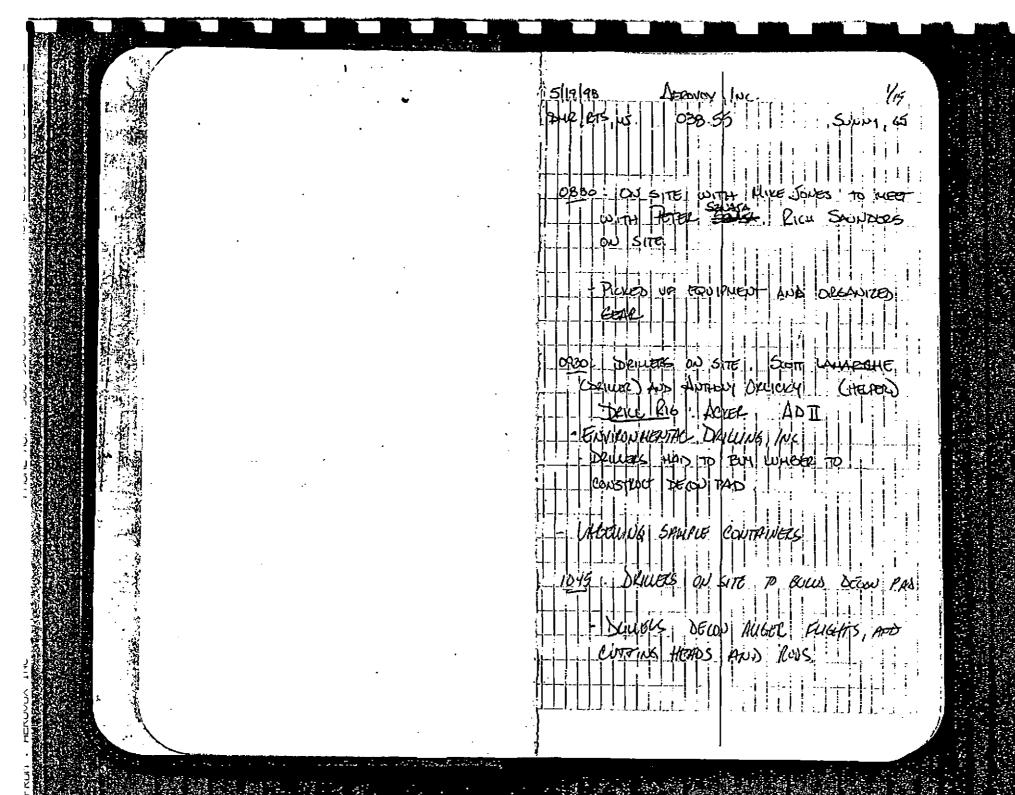
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Date Start/ Drilling Com Driller's Nan Drilling Meth Auger Size: Rig Type: A Spoon Size:	bany: Env ne: nod: Hollo ID 4.25 (cker AD)	vironm w Ste in.	ien ta	l Orli		Inc.			Depth: 10 ft.	Boring No: SB-18 ht: hvox Incorporated htion: Bedford, MA.
DEPTH ELEVATION	Sample Run Number	Sample/Int/Type	Blows/6 In.	z	Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description	Solf Boring Construction
			2 2					۹, ۱	GROUND SURFACE Asphalt Loose, black to grange-brown, medium to coar	<u>se</u>
	(1)		8 10 5 8 8 8	10	0.7	0.1			Loose, black to orange-brown, medium to coar SAND, trace Sil, little Gravel, dry. (Black discoloration in O' to F interval) Nedium, orange-brown, Nedium to Coarse SAND trace Silt and Gravel, dry to damp. Medium, orange-brown, medium to coarse SAND	ц
5	(3) [4]		7 10 12 14 15 22 25	22 47	1) 18	23			trace Silt and Gravel, damp. Dense, tan, fine to medium SAND, little Silt, trac Gravel, damp to moist.	
	(5)		25 10 10 11 11	21	18	NA	100		Nedium, tan SILT and fine SAND, trace fine Gra wet.	avel,
	BE D. BOUCK				2 2 2 2 2 2 2 2	Remar NA: base	No he	ads	pace measurement was obtained presence of saturated soll.	Saturated Zones Date / Time Elevation Depth

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Attachment 4

Field Notes - Soil Investigation Beneath the Parking Lot



5/19/98		ABROLION,	Wc.		K	5/8/95 AOLONOX, IAL 038.55	5
	JB-	1		-			
Deens		Blan (<u>جازاد</u>	RECOIDEN	PID	Description	
0-2'	1202	5-6-	5.7	bil!	02	HERROW BROWN AND BEACE COARSE SAUD, UTIL	: ۱۰۰۰ -
						PRESENCE IN 1-2 SECTURE	
2-41	1209	10-7-	e-11	0.0'			
-							_
4-6	1215	19-7-	-1	0.7	0.5	HOLE, BROWN HUSS BLACK PRAT FLOOR 4 TO	
· · · · · · · · · · · · · · · · · · ·		······································			-	14.31 REWAINING COULD SAUD, LITTLE GRADER	
	• •• •••••••		· [·····				
- YeA	LAYER	INCUD	<u>to in s</u>	SAMPLE .	\$8-7-5	- RID READINGS AT BREADLE 0.0 NO 0.2	
- 5AU	pre 51	-7-2_	to se	SUGHITTED	TO	- 1 - 0.0 NO 0.2	
		1	KIS AU		1	- DUST UTVOIS CUNCISTENT HT D.O.I D. O.OY	
5	<u>B-7-5</u>	FOR VOC	ANALYSIS				
R	B ANAUS	45 SB.	1-5 (1	(5')	······································		
			•			╶┑┝╍╦┿╍┿╍┿╍┿╌┿╍╏┝╴┥╴╏┝┈╌┊╴╎╴╴╸╸╴╴╎╎╌╎╴╴╴	• •• •
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4/15_ 5/19/20 Actoriox INC 038 49 038.55 5/19/98 ERNIOX, /NC # 1145: HEAS HES LEETING, SIGNED OFF - BEL OFF SITE TANKS SHARES TO FED EX IN TANNON MASS. THORED ENVIRONMENTAL SISTEMS /14- MODE :58B OUM SAMPLES Q FODEL - DALLEROUDD HINK O. S PAIN 1330: KIM TISA (USERA) ON SITE 1450: COMPOSITE ASIMACT SAMPLE COMP-1 COLECTED -COMPOSITE OF 56-7, 56-15, 86-16, ANIS SB-6 15/10 - MIKE JONES DEF SITC 1630 - MO: Devers DELON / PRIVERSIT AUGENS TUTPL DETON TIME FOR DAY - 1 Nr 1700 Demos OFF-5178

5/n/18	AGROUDY, INC. 03	<u>e. 55</u>	APROVOX, INC 038.55 7/5
58-			36-14
DEPTH TIME		NELANDY PID	Deseptertive
0-2' 1252	8-8-9-1	1.0 4.9	HEDWIN BLOW, RED, BLAUL COARSE SAND, LITTLE
· · · · · · · · · · · · · · · · · · ·			GRAVED, LITTLE BELL THATE SIT. DAPLES STALLS
			IN 1-2 TUTELINE, DEY
2-4' 1255	5-5-4-1	0.7 0.0	, LASSE BROW , COAKER SAUD, MUD GRAVE
	·	n an	UTTLE F. H. STALE JUT, DANA TO HOUST
4-6' 1305	5-12-16-23	0.8 NA	HERVIN BROWN TO RALE F-1 GLADEL LITTLE
			H- CI SAND TRAVE SICT, WET
			6Kanswara a #14 CBS
PID REPO/USS	C BOLE DIE O	0 10 0,2	
			1 POB DAHOUS SB-116-2 - LAB AWAUKES (1-2')
_DUT gavas (BOLETHOUS 0.01	2010 47	1 + 100 $100 + 100 +$
Lo consult	COLLECTED PROM	4-10' 2014	VOC Sprieve 158-16-2- CAS ANAULSIS (0-2')
MAROUZ			
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			╴ ┍╴ <mark>┠╾┝╍┶╴<mark>╞╼┨╼┑</mark>╼╄╼┠┙┥╍┝╍╠╸╽╎╎┽╌╬┍╎ ┍╴</mark>
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5/19/	NV	AEEN	»¥,1»«	38.93	/15	
	-			Ocurah	010	53-15
0-21	TIME 1340	0000 0 9.9-8		REOUNDAY	PID 0.0	
					·····	TRACE SILT, Deg
2.4	1343	8-5-	6-9	0.6	0.0	MEDUN, BRASN TO BLACK - SAUD Some ELANCE,
			·····			PEAT FROM 3 & TO 4 & DAW TO DAWA
4-6	1340	2-6	7.2	0,1	0,0	HEDWAR, BLAULTOGOWN M-L SAND, AND GRAVE
						- DAMA DE MORET.
						- No SAMPLE FOR ANALYSIS (POOR RECOVERY)
6-8'	1355	4-9-	19-19	1.2	NA	BROOD TO GLACK F.C. SAMDS, LETTLE GRAVEZ, i WEATHERED DEROUG OF 7.5. WET
						$= \{ (u \in H) H \in U \}, D \in Ool (u \in G = \{1, 5\}), U \in [n] \}$
	THINK		0.9 TO			PCB AWALYSIS SB-15-2 (BATT 1-21)
	READIN		0.01 10	1		- VUC AWALYSIS 50-15-2 (0-2)
					·	
	- <u> </u>	-	·	<u> </u>		
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5 5 19 9	, 	AFLOVOX	INC. 038	.51	10/15	5/A/98 AEROVOX; INC :038.55 "/15
Depith		5B-6 BLOW 60	כיזייוב	RECORPY	PID	Descerences
	1422	15-7-4	-3	<u> </u>	<u>3,5</u> *	HED VIL, FRONS/BIACIL/TAN F.C. SAND, SWITE GEAVER, TRACE SUC NEY NO DAGE MOST. 0-1 POSSIBLE OWY/BLACK INTERVAC
2-4'	1425	1-1-	<u>-1</u>	.0.3	<u>, Au</u>	LOOSE BUDGE / TAN GRANCE SOME F.C. SOME, TRAVES SUT AND CLAPY, WET
•·			70 O.1		······	RCB ANALYSIS > 58-6-1 (0-1')
Dist (eurs	0,0	10 0.		······································	Voa Auguysis · 586-2 (0-21)
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_5 19 98		2010x_1/18 B-17		•··· •· - ···		5/19/28 ACROUDX, 1/2 038.55 13/15
DEPT(+	TIHE	BLOW	COUNTS	RECORDEY	PID	DESCRIPTION
0-21	1509	17-8-5	-1	1.31	1.3	MESNUM THU (DENSU) OLDIN FIC SAND SOLLE ME FIN
	~	<u></u>				GRAVEL TRACK SUT, Day
						- David Stando IN 1-2' INTERIAL
2-41	1510	2-2-2-	2	0.0		No Recovery
· · · · · · · · · · · · · · · · · · ·						╴╺┽╷┽╴┝┽╻╪╌╷╧┼╵┥╍╦┊╺╿╌╸┝╌┝╌┥╸╞╧╓╍╸┱┶┶╺┪╎╶╧╵
4-6	1918	2-1-1	<u>/+1</u>	2.0	0.0	1 BEDSE BLOW BLOW PENT LATTE FL SAND, TEACE SEAVE
		2-1-3		1.7	NA	- UDIG, DET TO HOIST (ROAT (R 4.3)) Brund / Bener, Peper, some, F-24 SAND, TRACE
6.8	153.				N.C	Genna, Trine Sut wen
	-					
					·	1 PCB ANDUSIS : \$B 17-5 (4-5)) E
	-			-	· }	10C-ANALYSIS 186-17-2-C021 5B-17-2
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5/19/98	#	<u> Rocestox, In</u>	038.	5	/!/	5/19/98	AGEO VOX , AUC.	038.55	15/15
··· ,	-	3-5					£B,~j		
DORN+	TIME	Plan ia		RECORDEY	Pin	DESCRIPTION			
0-2'	1602	4-5-	5-6	1.5	2.4	wose huns/hu			Her
 _			· 		· •• {	Une but	This GLASS TRAC	E SUT, DEY	
· · · · · · · · · · · · · · · · · · ·	· ·				· · · · · · · · · · · · · · · ·				
			*						
2-4'	1605	4-3-	3-4	1.2	0,0	Loose Burnd/1	CALL F-CSA	NI AND BRAKER	
					` `:	THE SUL	DAMP TO MU	ur i	
	 	·	··· -		Ì				
4-4'	1610	5-4	-4-5	0.1	NA	10050 Some	ever wer		
			······································		. ;	water	P = 43 Fr.		
	·								
12 0.00									
	LEVERS_	we c	.01 - 1 .01 - 1		· · · · · · · · · · · · · · · · · · ·	PCB SAMPLE	5B-5-2 5B-5-2	- (1-2)v	
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5/20/98	AFRONX, WE.	1/14	5/20/73 Atalona, We 030.65 - 2/16
DMR, KIS	03.8.55.003	P. Sowry, 624	
· · · · · · · · · · · · · · · · · · ·			ATTELYTED FOB-11 . IIIT PETERVE UNDED
0700 : BBL OF			
	ON SITE		CRIMA THEODEN, WITH HUEERS BUT CUULD NUT
0710 : EPA la	PLESENTATIVE ON SI	F . K. Trca	POSIBLE BOULSER MUL ATTEMPT ATTI NEW BB-11 CULMTION TOHOLCON - 1675 -
B			- 35 11 000 Miles 10 Mg Coll - 10 29 -
	and Allows luc.		1645' DRIVERS OFF SITE
D4u	LOC: SHALE PUL	AK	
	-		1750: 38L OFF BITE
8820: ERA of	A SITE		
1210 + 1300 : L	Come		1846 - SAMPORS DELIVEROS TO FED ER
			┝┽╌┠┥╍┽┨┥╋┽╪┽╡┫╍┙╍╸╃┽╎╌║╴╴╌╎╴╞┼┨╸╵╸╽╶┊╶┇╽┫╶╎║╍╜╔╍┊
1315-1345: 0	ALLELS DELAN ALL	KAR FLIGHT3	┢┾┾┽┽┼┨┽┫┥┧┙┝┼╎╺┉┪╍┶╶┥┥┛╸┍╌┟╴╎╶╢╍╧┼┼┼┽╍╍┤╸╘╽╴╪╶╍╼╄╴┨
- AND PL	ALE COLD PATCH		
ESREHOLE	<u></u>		
		-	
liklo: Heaver			
	COMPOSITE SAMPL		<u>┥┯╍╼</u> ╼┝╾┝╼╿╼┡╼┞╍┨╼┽╼┽╼┥╼┥┲┥┎┙╔┙╗╖┥╼╎┥┍╌╻┥╎┝╏╴╞╌╟╍╕┍┦╺╸╽╺╎
Cone	SITE-Z Son	BUCINES	
Cone			
Cone	SITE-Z Son		

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3/14 5/25/12 AEROVOX, Are 038.55 5/10/98 ACLOVOY, INC 038 55 58-18 513 18 Recovery TIME BLOW COUPES PUD Deritt DESCRIPTION Love Bink to bRANE - BROW H.C. SAND, LITTLE 0-2' 0817 2-2-8-10 0.7' beaute, TRAVE SUT, DCT_ Dice Caseden in p-1 1-2' 100 SAMPLE CROC HOT COLLECT R-4' 0820 5-8-6-6 0.0 , ORANGE BOUN H-C SAND, TRACE CRANCE D.l. DLY D 4-6' 0825 1-10-12-14 1.1' MEDINA, DEANEE-DECIN ESAUD, TRACE GRAVEL 0.0 DHM DOUG TON F.M SANN, LITTE TO AND SIG, TRACE 1.8' 0345 19.22-25-29 1.8' 2.3 10-10-11-11 1.8' NA 0904 8-10 SILT HUB F. SAUD, TRACE PCB SAMPLE 56-18-1 (0-1') VX SPHPLE 5B-18-8 (6-8') WARR @ = 18 FT. 14 BOLENCIE READINGS D.0 70 10.3

-5/20/23	Á	twok ; /w	1 038:	<u> </u>	<u> </u>	5/20/10 Accentor, Inc. 038.55
		58-1			· · · · · · · · · · · · · · ·	тана и на
DEPTH	TIHE	Blow (RECONTRY	PID	DESCRIPTION
0-2'	_0938		<u>1- 2</u>	0.8	<i>0.5</i>	LOSSE, DAEL BROWN TO BLACK F. C. SAND, THAT SILT AND GRAVEL, BLACK DISCONATION IN 1-2 INFORMA DCY
2-4'	0940	2.3	f-2_	1.2	0.0	, Lange OLANGE- PRON F. C. GAND, TEACE SILT MADE GERNEZ, DRY
4-6'	0949	2.3.		/.v		1 LOOSE THIN. F.C. SANN, TEACE SILT AND
6-8'	נדטן 		22-19	1.0		DENSE, TAN E-C SAND, BUTE F-M GRAVE, TRACE SALT, DAMA:
<u>8-10'</u>	1014		15-19	0.7	0.1	SAME SUCCAY SAMP TO HOURY
10-12'	<u>/817</u>	14-16-		1:6	<u>_N.A</u> .	DEUSE, TAU H.C. SANA, SONG F.M. GRAVEL, LITTE F. SAND AND SILT AT TIP OF SPORL NET
PCB	SAMPLE	<u>58-1-</u>	2 (1	2')	,	
Voe	5Анец(<u>-8')</u>		- GROUDSWATT AT 1/0' - BOREHOUS/BREATHING ZOUE: 0.0 70 0.2 - DUST LETERS : 0.01 - 3 0.07
				and the state		

the distance

3/ 16 5/20/98 ACTOVOX, INC. 038.54 <u>\$ |20 |98_</u> ACKEVOX, WC 038.99 \$8-14 50-14 <u>)</u>8111 Blas Custors TIHE REAL PID BLACK / TAN, N. C. SANS, LITTLE GRAVE 11-10-9-7-0.4 0.21 1104___ TRACE SICT, DUL O'-1' SAMPLE OWLY FOR POSS 2-41 1109 5-2-1-1 & SAME EXCEPT BURCH, DAMP 0.1 LOAGE Sanar only FUR REBS LOSE DY BEAULY BLACK F.C SAND LITTLE SEAVE 4-4' 5-1-9-9 1114 0.9 TEAUS SILT. DAVIE TO MOIST. 4-8' 4-8-7-5 11 20 1.5 NA SANG EXACUT YOUE PREMITING ZONE 0.0 TO 0.2 56-14-5 2 515-14-5 SAMPLES pco 45' north St5-14-6 (4-6') VOC. SAURE

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.!	<u>5/10/98</u>		<u>Aelvix, I</u>	1 <u>1. 038</u>	Ħ	9/16	· · ·	5	120190	-1-1-		AERU 10)	د ابد	 	038	.65	• - -		1-	ند ار	16			
			513-4								<6	-4			; ; ;		:	•					17 18	
_	NEPTHI	ТІНЕ	BLOW 10	UNTS	REWERY	PID	ł	_ >	wer pr	1 '.									1			:		
-	0-2	<u> 1148</u>	4-5-	5-4	1.0	62.5			OB PE	<u>μ</u>		w/b										1		
-				·					GRAVA						· +			6.1	1	2				
-				·	 		-		IN IBU		.Perl	78	6 ALI		-1-1-			-						
-	2-4'	1152	4.7-0	-5	D.7	3.5		M	ED WA	Fus	برر		SA.	. כזנ , כזנ	<u>u</u>	17Ur	r V	901	4.E	 		'		
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-	No	3-11' 0	AB SAL	us cour	105 115	<u>.</u>	51	, 	GENERS			e 5				Ľ.		 		• •				
-	10	tt l	OF REL	puary	14) 102		j +			1			7 67		-4-4	┥┙┥╌			<u>}-</u> +-	<u>- </u> -	·			
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-	PCB	PHALMSI	58	4-2 (1.2')	.}	· (†	-+-	┥	┿╋	++	╶┤╌┥╼╎	┥┥╸	- -	╺┟┥									
-	Voe	ANALYSI		·····›	0-2')		. 31. 		╊╌╎╌┨╌┠╌╟ ┋╎╎╎╽╽	- - -		···						 -	-		1			
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"/14 5/20/98 ATTERDX 110 038.49 GOVOX INC 1.5 20 98 33-13 56-13 PU) REWIEY BLUEN COUNTS Derny TINK BAN TO BRUDN F-C SANN LITTLE DEFE 1.4 0,0 MADILO 10-11-10-8 1355 0-2 PIM ORAVER, TRACE SILT, DAY 0-4 INTERVAL BLACK COLOR 0,0 LOOKE CANEEL-BROWN F-C SAND TRACE SILT YAND F 4-3-3-4 2.4' 1518 1.1 Gernia Day to Same Has SAME BUDT VA 7-9-3-5 46' 1409 0.7 Ų ET 6 lanswerer c + 4 Fr 58-13-1 (0-Peb_ GAMPLE BOREHOUE / GREATHING BOLE 58-13-2 (0-2 0000 SAMELE Voe Dist lags 001 10 10.04

5/22/98	ABROIDX LUC O	3.4 17/10	- 5/23/20 ACTOURDY, /14 039.549 /16
SI TINE	- 12 BLOW OWNES	RECORDER PID	SB-12- DESCRIPTION
0-2 1450	7,9,12,15	1.5' 1.1	- DHEDIUM DHEE BUDU BUHUE DO OLANGE DALLAND, Fr.C. SAUD, LITTLE GRAVEL TUALE SILT, DALY - DALL COLORD 071 INTERVAL
2-4' 1453	12-18-/8- 20	1.4' 0.0	
4-6° 1504	20,21,14,19	1.4'0.0	- SAME EUCOPT: POCK AT TIP OF SPOUN TWO DAME
6-8 1510	28- 4/1	0.1 NA	- GRAVITE/GWOTSS - HIT DETUSIK, WOT WA/BR Q = 6 FT
PCB SAMPLE VOC SAM	S 5B-12-1 NE 58-12-2	(0-1') - (0-2')	BOUGHEUR / PALATIENS BUE 0.0 70 0.01

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2-9' 1413 3-2-2-1 0.7 UA Prove (BURU)-BURU - BURU)-BURU - FETT or F-14 SAD), 	5/20/78 DEPTH 0-2'	5 TINE	Atronox 3-,3 BLOW CC .(7-15-8		15) REZONGRY 1.0	19/140 	SIZOIGO AGEOVOX, INC 038.55 1/14 SE-3. DESSCRIPTION HEDRICH DALL FILL FILL SAND, I. HETRIC GRAVEL AND SIGT DRY D ANOIST
	<u></u> <u>PCB 54</u>	HPUÉ	<u>85-3-</u> \$8-3-	4-3' 2 AND	SB-3-21		2.419 LOSS BLOW BLACK PETT (Z.0 TO Z.4) - 2.419 LOSS BLOW BLACK PETT, WET

5/21/90	ADROVOX INC	<u></u>	5/21/20 AGROVER, INC. 038 55 3/14
DHR, PTS	030.69	4. SUNRY 624	
			1918 COLLECTER ABITHANT COMPOSITE SAUNCE
0700: 30L 00			(CUAR-13) FCOH , SR = 3, 19, 11, +12-
- PRODU Lixtatio	······································		1131 COLUTION ABDILLER COMPOSITE SAMPLE
	<u></u> \ \		(COMP-4) FROM 5B-6 C 5B-2
0740: DRILE	25 ON \$17K		
PARH	We BORINGS WITH ASPI	Hat	1219 SOIL TODENSO INSTALLATIONS
COLD	PATCH		Courteres
	S RINSE DUANE SAMPL		- Aulters was 3 BAGS OF WOT
	TAMINARES SPUT - SPOT		Asputitet Cours 1947 Cut
	GE FUL POlis (HETHOD		1810 BBC OFF SITE
<u>_</u>	123 (HETHOD BULD)		BID: BBC OFF SITE DECIMER SAMPLES AND RENTAL
			EQUIPACINT TO FED EX
	DRILL RIG AT 50-11		
	HON TOP AND HAVE SUT		1900: SUMMER AND EQUIPHENT @
	CAU BLATTER		TEAS
- INFORMES PU	RETER SOLATA THAT COR	RESTORES	
	0 58-11 AND POFADI		
50-10			
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		Attack -	

	5 21 98	.	ALEXOVOX, IN	- 038.1	si	·· <u>*</u> 14		5 21/10		Acoust	1 luc	038.	55	<u>;</u> ; ; , , , , , , , , , , , , , , , , ,	<u>۴</u>	/ . <u>!</u> 4
)1381H	TIME	58-11 BWN CO).uts	CUECOVIALY	P/15		DESCA	L PTA	5.B						
	0.5-2.5		5-8-10		1.0	112	. 1	lesich .	: Recun /	ciace fin						
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Attachment 5

GHR Cross Sections (A-A' through E-E')

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REPORT OF

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EVALUATION OF REMEDIAL ALTERNATIVES FOR THE AEROVOX PROPERTY, NEW BEDFORD, MASSACHUSETTS

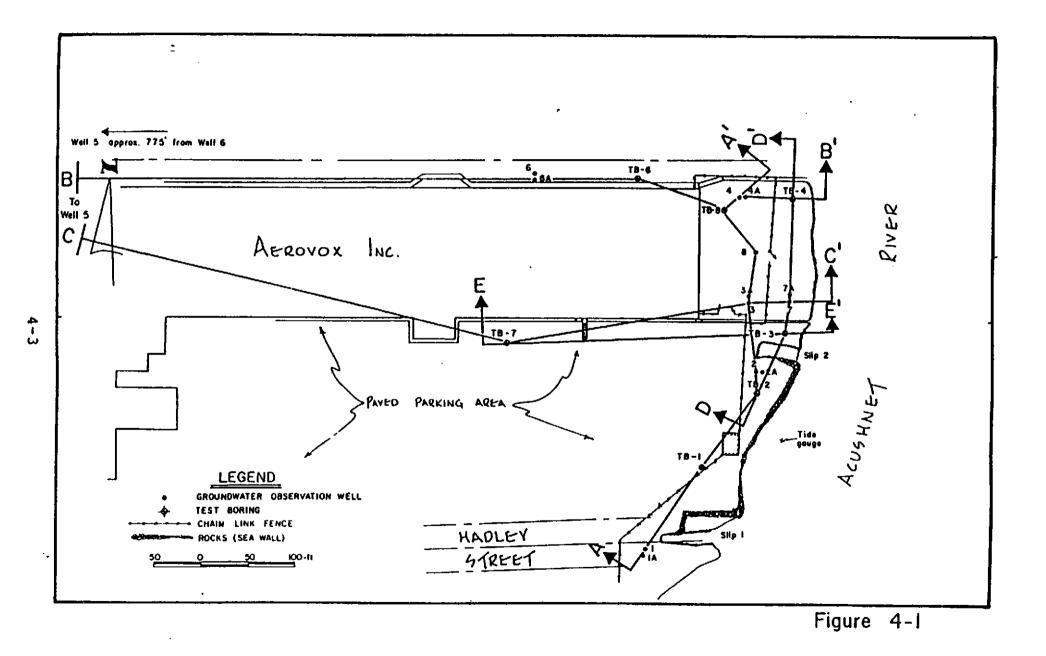
SUBMITTED TO:

AEROVOX INCORPORATED 740 BELLEVILLE AVENUE NEW BEDFORD, MA

PREPARED BY:

GHR ENGINEERING CORPORATION 75 TARKILN HILL ROAD NEW BEDFORD, MA

FEBRUARY 11, 1983





Transmitted Via Facsimile/Federal Express

September 14, 1998

Ms. Kimberly N. Tisa Environmental Scientist USEPA - Region 1 JFK Federal Building (CPT) Boston, MA 02203-0001

Re: Aerovox, Inc. Facility New Bedford, Massachusetts Supplement to the Engineering Evaluation/Cost Analysis (EE/CA) Report Project #: 1638.03855 #2

Dear Ms. Tisa:

Pursuant to your request during our September 11, 1998 telephone conversation, this letter and the corresponding attachments have been prepared as a supplement to the final *Engineering Evaluation/Cost Analysis (EE/CA) Report* that was submitted to the United States Environmental Protection Agency-New England (EPA-New England) on August 27, 1998. To supplement the *EE/CA Report* (August 1998), Blasland, Bouck & Lee, Inc. (BBL) has developed the two additional geologic cross-sections that you requested during our telephone conversation. The locations of these cross-sections are depicted on Figure 1 and the cross-sections are shown on Figures 2 and 3.

As requested, a new cross section (Y-Y') has been developed by revising cross-section X-X' presented in the *EE/CA Report* so that the western end of the new cross-section starts at boring SB-10 which was projected northward on to the section line. Unlike cross-section X-X' presented in the *EE/CA Report*, the subsurface log information for monitoring well MW-4B has not been included on cross-section Y-Y'. The subsurface log for monitoring well MW-4B indicates a drop in the bedrock surface to the north (bedrock at 21 feet below grade), as further documented by the log for monitoring well MW-6, located toward the northern side of the building (bedrock at greater than 45 feet below grade) and log for monitoring well MW-4, located at the northeastern corner of the building, (bedrock at greater than 20 feet below grade) in contrast to the depths to bedrock along cross-section Y-Y' in the vicinity of soil boring SB-11 (possible top of bedrock at 2 feet below grade) and soil boring SB-12 (possible top of bedrock at 6 feet below grade). Addition of the subsurface data for monitoring well MW-4B to the cross section Y-Y' would therefore not likely be reflective of the conditions along this line of section. The logs for each of the aforementioned soil borings and monitoring wells are provided for ease of reference as Attachment 1.

The second cross-section requested (Z-Z') begins at soil boring SB-1 (located near the northwestern corner of the manufacturing building) and extends southward to soil boring SB-8. This cross-section is shown

Ms. Kimberly N. Tisa September 14,1998 Page 2 of 2

on Figure 3. The aforementioned drop in the bedrock surface to the north is also illustrated in cross-section Z-Z', as indicated by the depth to bedrock at soil boring SB-1 and monitoring well MW-5, both located at the northwestern corner of the building. The depth to bedrock at soil boring SB-1 and monitoring well MW-5 is greater than 12 feet below grade and greater than 20 feet below grade, respectively. Copies of the boring/monitoring well logs for SB-1, SB-8, and MW-5 are also provided in Attachment 1 for ease of reference.

In addition to requesting the preparation of two new cross sections, you inquired about including the depths of the sheet pile wall on appropriate cross-sections. This sheet pile wall serves as a vertical barrier to ground water and tidal flow into and out of impacted soils located at the eastern end of the site. This sheet pile wall was installed as part of the remedial action completed in 1984. As discussed during our September 11, 1998 telephone conversation, Aerovox does not have an "as-built" construction drawing for the sheet pile wall. Although specific depths of the sheet pile wall for inclusion on cross-sections are not currently available, known information regarding the depth of the sheet pile wall was included in the report. For example, on page 2-14 of the *EE/CA Report* the following information regarding the depth of the sheet pile wall is presented:

"The sheet piling cutoff wall is from 9 to 13 feet in depth, the actual depth is dictated by the depth to the peat layer into which the wall is keyed."

You also inquired about the disposition of the existing asphalt parking area for each of the alternatives described in the *EE/CA Report*. The proposed capping system described in the *EE/CA Report* would be constructed over the entire facility, including the area where the building is located (after demolition of the building) and the asphalt parking area. As detailed in the *EE/CA Report*, the details of the final capping system for the Aerovox facility will be selected during the design phase based, in part, on site conditions and future reuse of the property.

If you have any questions or require additional information, please do not hesitate to contact me at (315)446-9120.

Sincerely,

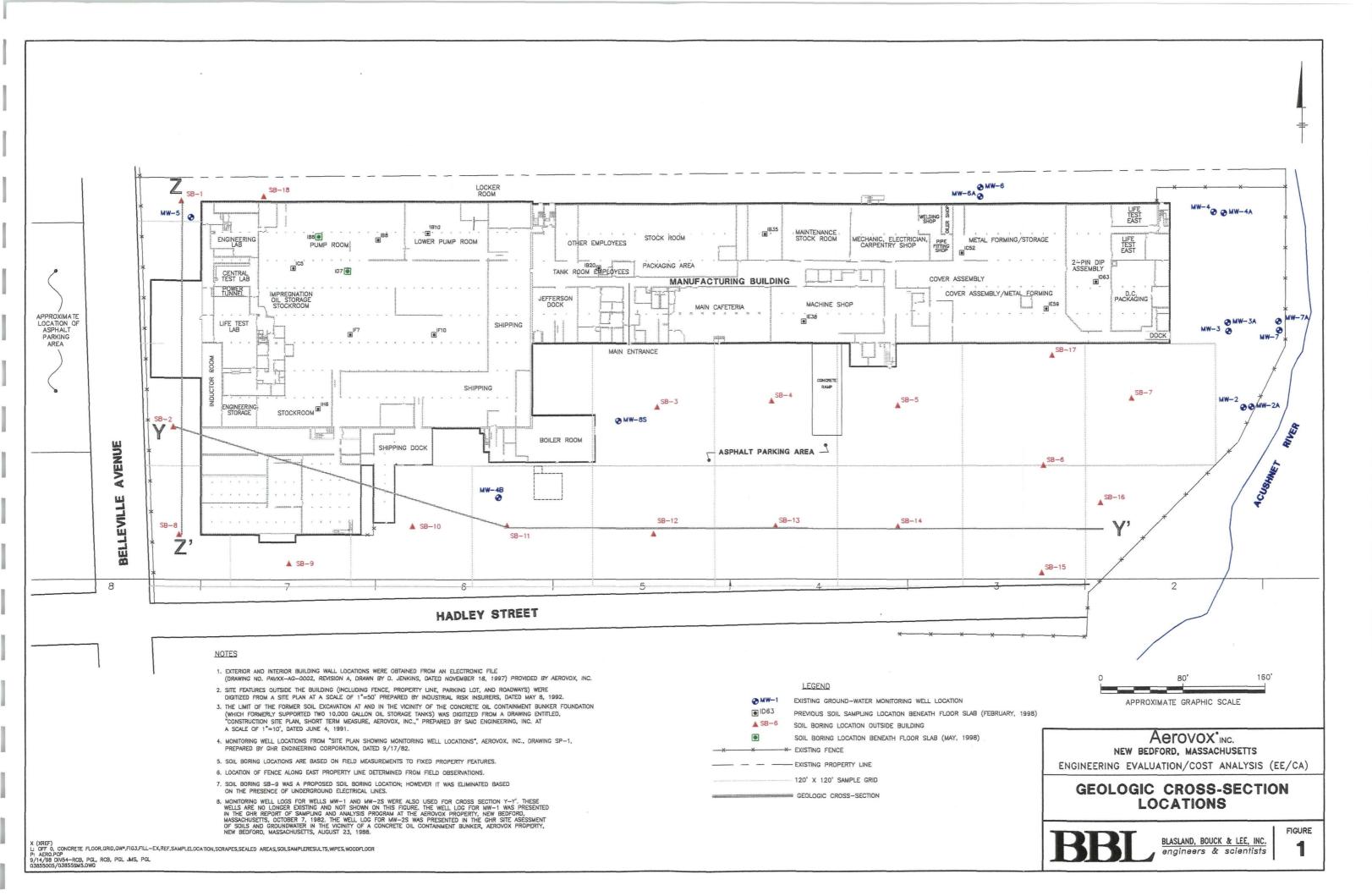
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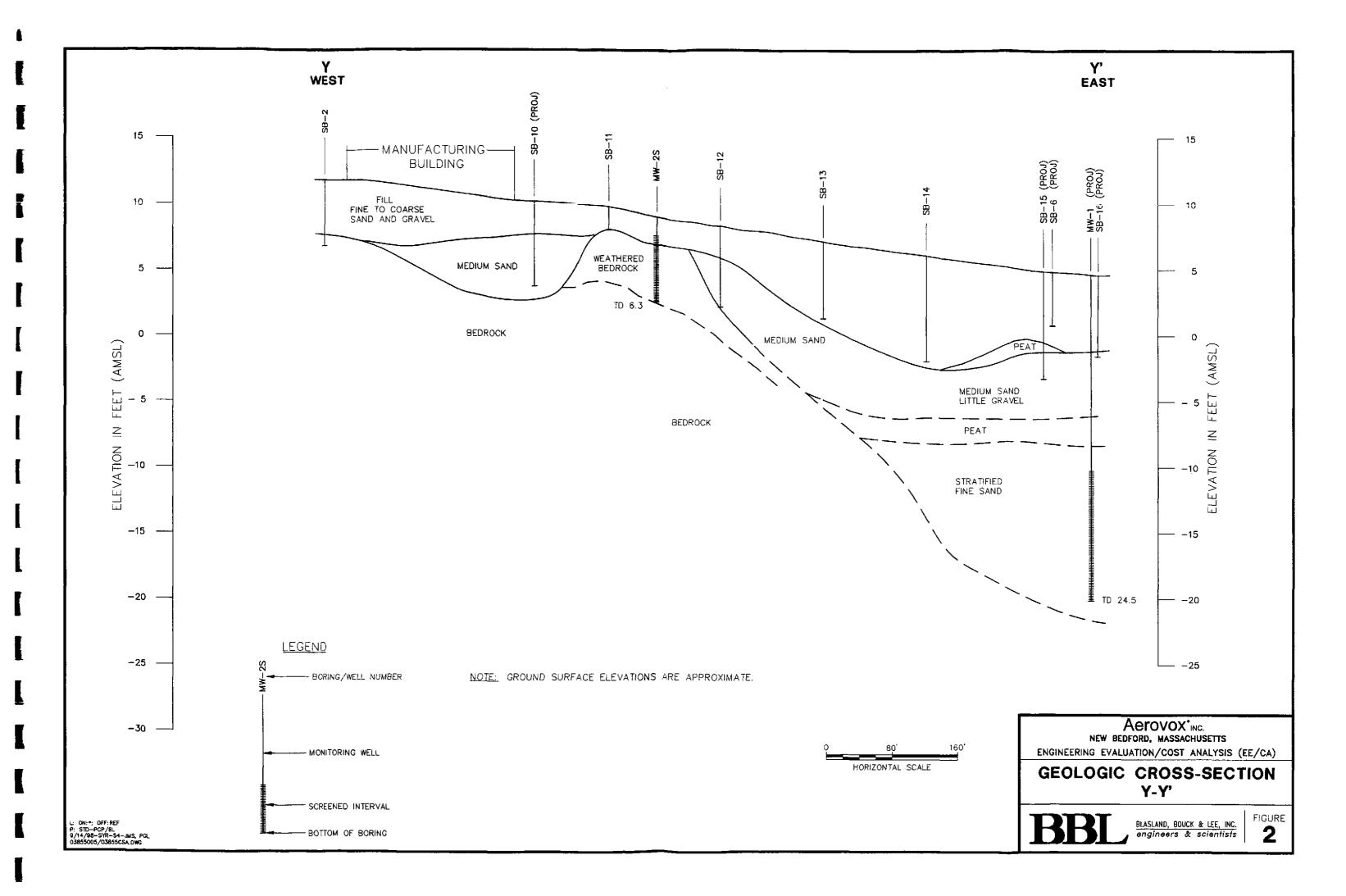
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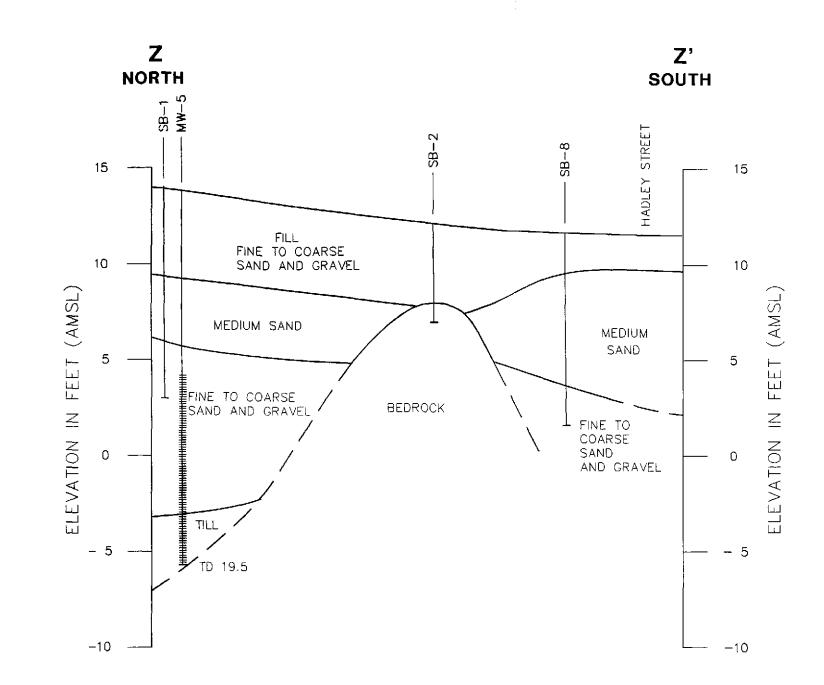
David J. Ulm Vice President

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 cc: Mr. Jonathan E. Hobill, Massachusetts Department of Environmental Protection Mr. Robert D. Elliott, Aerovox, Inc.
 Colburn T. Cherney, Esq., Ropes & Gray

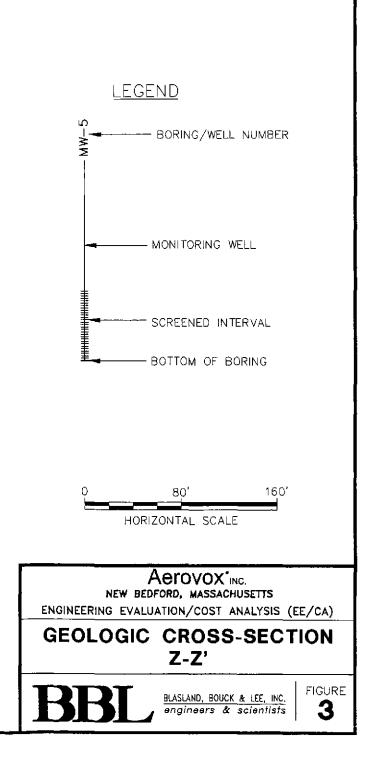






NOTE: GROUND SURFACE ELEVATIONS ARE APPROXIMATE.

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Attachment 1

Boring/Monitoring Well Logs (provided in the order in which they are referenced in BBL's attached September 14, 1998 letter to the EPA-New England)

Driller's Name Driller's Name Orilling Metho Auger Size: I Alg Type: Ac Spoon Size:	nd: Holfon El 4.25 in Ker AD I	e Ster		let		B	6e	ològ	Depth: 5 ft. Locati New Bu at: Doug Ruszczyk	on: edfard, MA.
DEPTH	Semple Run Number	Sample/Int/Type	Blows/G In.	N	Recovery (11.)	PIO Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description	Solf Boring Construction
									GROUND SURFACE	
-	(1	\backslash	4 8 8	P	ιo	م٥			Asphalt Nedlers, orange-brown fine to coarse SAND, little fine Gravel, trace Silt, dry,	e
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Drilling Auger Rig Ty	Size:	od: Hollon ID: 4.25 li :ker AD 1	n.	r Auş	je r		B			Depth: 6 ft. Location: New Bedfo lat: Doug Ruszczyk	icorporated	
DEPTH	ELEVATION	Şample Run Number	Sample/Int/Type	Blows/8 In.	N	Recovery (11.)	PIO Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description	Soil Boring Construction	
	-			25 35	75	6.0	00			GROUND SURFACE Asphalt Very dense, brown/black/tan fine to coarse SAND,		
		(2)		40 19 11 7 5 5	8	10	a0	-		some fine to medium Gravel, dry. Netium, orange-brown/tan fine to medium SAND, little Gravel, dry to moist.		
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		<u>5-1</u>	24*/18*	0.5'-2.5'	35/25/20/30	FILL	F/C Gravel, little Brick Fr Silt		80L	H		Protecto	1
		5-2	24"/15"	2.5'-4.5'	20/20/40/40	FTLL	Tan/Brn F/C SAND and F/C GR little Silt, Brick Fragment		BDL		1 P	Н	
5				·		FILL	Tan/Brn F/C SAND and F/C GP	LAVEL,	BDL	-	7 F	Cement/ Bento- nite	
-	┣	<u>5-3</u>	24"/13"	4.5'-6.5'	48/124/46/62		occasional Cobbles, little				11	Slurry (20:1)	
		5-4	24"/12"	6.5'-8.5'	17/24/25/24	FILL	Tan F/M SAND, some C Sand, F/C Gravel, Silt	ittle	BOL		15	1	
	<u> </u>						No Sample Recovered					2.0* 10	
10		5-5	24*/0*	8.5'-10.5	24/21/16/18						1 [Pvc Riser	
		5-6	24*/12*	10.5'-12.5'	16/15/14/17	GLACIAL OUTWASH	Tan M/C SAND and F/C GRAVE Silt	L, little	80L	F	11	7	
	┣	1 5 7	24"/6"	12 51-14 51	50/40/25/25	GLACIAL OUTWASH	Tan F/C SAND, some F/C Gra little Silt (2") overlying		801,		11		ļ
15		3-7.	24 78	12.3 - 4.3	30/40/23/23		COBBLE (4*)			<u> </u>]	7	
•••		5-8	24*/13*	14.5'-16.5'	34/56/17/18	GLACIAL OUTWASH	Tan VF/C SAND and F/C GRAV	CL,	80L	F] [7	
		<u> </u>	24*/5*	16.5'-18,5'	16/14/9/12	GLACIAL OUTWASH	Tan VF/C SAND, some F/M Gr little Silt	avel,	BOL				
	 	5_10	12"/6"	18,5'-19.5'	15/120	GLACIAL	Tan/Red C SAND and F/C GRA	VEL,	BDL			7	
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	of pear & brick, & tar-		E	AV 79	2-4*	Soil	
_	like material			AV 80	4-6'	Soil: PC	B = 72
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	Peat		<u>6.5' V</u>	AV 81	6-8'	Soil	
	8.7' Medium-coarse			AV 82	8-10'	Soil	<u></u>
10 -	sand & gravel						
				AV 83	10-12'	Soil: PC	<u>B = 23</u>
		F	Well #4, 2" PVC			<u> </u>	
		-		<u> </u>	<u> </u>	<u> </u>	
		-			†		
15 -		-					······
		-					
		L		ļ	ļ	 	
				AV 84	18-20'	Soil: P	CB = Trace
20 -	20.3'		20.0'				
	Bottom of						
	boring @ 20.3'	Ber	tonite seals			<u> </u>	<u> </u>
			stalled: 8-10'			<u>†</u>	
			A 1-1.5'				······································
					<u> </u>		
— ———————————————————————————————————							

Auger Rig Ty	arilling Method: Hollow Stem Auger Auger Size: ID 4.25 in. Aug Type: Acker AD II Spoon Size: 2 in.						8			Oepth: 3 ft. lat: Doug Ruszczyk	Client: Aerovox Incorporated Location: New Bedford, MA.			
DEPTH	ELEVATION	Sample Run Number	Sample/Int/Type	Blows/8 In.	N	Recovery (tt.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description	•		ioir Boring onstruction)
9		(0		5 8 10 50/ 0.r	10	10	112		• • •	GROUND SURFA Asphalt and Cobbles Nedlun, brown/black/tan, the to co some the to medlun Gravel, Rock a dry. Refusal, possible top of rock. Ange 3 feet returning fragments of gness	iarse SAND, t tip of spoon, rrs advanced i			
5														
<u>6</u>	Ŧ	SF	31				Fiema	srks				Satura Date / Time	sted Zon	_

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Driller Driller Drillin Drillin Auger Filg T	g Comp r's Nam g Math r Size:	od: Hollov ID 4.25 ir :ker AD I	ironmi V Stei	ental	Orill		Inc.		·	Depih: 8 ft. lati Doug Ruszczyk	Soll Boring No: SE-12 Client: Aerovox Incorporated Location: New Bedford, MA.
OEPTH	ELEVATION	Sample Run Number	Sample/Int/Type	Blows/8 In.	X	Recovery (11.)	PID Headspace	Geotechnical Test	Geologic Column	Stratigraphic Description	So# Boring Construction
										GROUND SURFAC	Æ
		(1	\setminus	4 9 13	22	ឆេ	u			Asphalt Redian, dark brown/black to orange- coarse SAND, little Gravel, trace Sit, discobration in 0° to 1 interval)	-brown fine to
		121	$\left \right\rangle$	12 15 16 20	38	14	۵٥		•	Nedium, orange-brown to kan tine to trace Silt, dry to damp.	nethun SAND,
5		(3)		20 20 21 14	35	14	00			Medium, orange-brown to tan fine to trace SML, Rock at tip of spoor, dam	nedion SAND, p to noist.
-				17						Refusal, with gneissic schist rock fra spoon, wet.	gnents in
0											
-						}					
-											
6	BLASL/ eng2/	BOUCK	scier	E. IN Xipt: ste: 0	.		Rena	rka	1		Date / Time Elevation De Page: 1

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Driller's Name: Drilling Method: Hollow Stem Auger Auger Size: ID 4.25 in.								Borehole Depth; 12 ft.					
Rig		cker AD I						Ge	oiog	New Bedford, MA.			
DEPTH	ELEVATION	Sample Run Number	Sample/Int/Type	Blows/8 In.		Recovery (11,)	PID Heedspace	Geotechnical Test	Geologic Calumn	Strattgraphic Solf Boring Description Construction			
								-	• •	GROUND SURFACE			
		(8	$\left \right\rangle$	1 3 7	α	0.8	0.5		•	trace Silit and Gravel, dry. Black discobration in 1-2 intervall.			
			$\left \right $	2					•	Loose, grange-brown, fine to coarse SAND, trace SRt and Gravel, dry.			
		(2)	$\left \right\rangle$	352	8	uz	00			· · · · · · · · · · · · · · · · · · ·			
			$\overline{)}$	2					•	Loose, Ian fine to coarse SAND, trace Silt and fine Gravel, dry to damp.			
<u> </u>		[2]	$\left \right\rangle$	3 3 3	8	10	۵J		•••				
-			$\overline{)}$	4					•	Dense, lan fine to coarse SAND, some fine to medium Gravel, trace Sik, damp.			
_		(4)	$ \setminus$	22 19	40	10	LI						
		(5)	\backslash	17 20	38	٥.7	0.1]		Dense, lan ine to course SAND, some fine to neclium Gravel, Irace Sill, damp to moist.			
10		1.08		10 19					•	Dense, tan nedlan to coarse SAND, some fine to			
		(6)	\backslash	14 16 19 17	35	18	NA			nedius Granel, little fine Sand and Sit, vet.			
 .													
6													
		31	3		l L			Na	head	Isoace measurement was obtained Oate / Time Elevation C represence of saturated soli.			

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Orillog Driller Orilling	Comp s Nam Meth	F inish: 06 any: Env e: od: Hollow ID 4.25 in	ironm v Ste	ental	Orill		inc.	iorel	hole	Client: Aerove Depth: 10 ft.	oring No: SB-8 ox Incorporated ion:
	ipe: Ad	cker AD 1						6e	olog	New B	edford, MA.
DEPTH	ELEVATION	Sample Run Number	Sample/Int/Type	Blows/B In.	N	Recovery (ft.)	PID Headspace	Geotechnical Test	Geologic Column	Description	Soll Boring Construction
										GROUND SURFACE	
		EU	$\left \right $	4 0 8 7	и	ы	ม			Asphall Nedlun, orange-brown to Ian, fine to coarse SA Uttle fine to Nedlun Gravel, trace Sit, dry.	
-		(2)	\backslash	1 13 15 17	24	0.8	0.0			Nedium, orange-brown, fine to section SAND, so: fine to medium Gravel, dry.	1¢ .
5		[3]	$\overline{\left \right }$	5 20 25 8	45	u	0.9			Dense, orange-brown, fine to medium SAND, son fine to medium Gravel, dry.	и
		[4]	$\overline{\left \right }$	18 25 32 28	57	12	QI			Very dense, orange-brown, line to nediut SAN some line to medium Gravel, dry to damp.	L,
 		(5)		10 25 48 34	69	Q.4	NA		0.0.0	Yery dense, ian medium to coarse SAND and medium to coarse SRAVEL, wet.	
-											
-											
6				 			Rema	arks			Saturated Zones
	ELSU Bast	SE	5	E. IN			NA	: No	head on th	space measurement was obtained a presence of saturated soll.	Cate / Time Elevation Cer
Prajec	t: 038.			cript: ate: 0		181	<u> </u>				Poge: I

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				MW- S				
E	ORING / OBS	ERV	ATION WE	LL SU	MMARY	LOG	BORING No.	5
	OJECT Aerovo:			-			ET _1OF	1
LO	CATION New Bea	lfor	e, Ma			RACTOR	D.L. Maher	
	ENT Aerovo:					INSTAL	LED July 28,	198
	R FIELD E		G. Hartley,	-	FIELD	SAMP		
DEPT	STRATA DESCRIPTIONS		-		T	<u>r</u>	LE DESCRIPTI	ONS
	Mixed sandy			AV 85	0-2'	Soil		
	fill with pieces of							
	brick		Bentonite	AV 86	2-4'	Soil:	PCB = < 2	
	4.4'		seal	AV 87	4-6'	Soil		
5	Yellow medium		0-0 ·					<u> </u>
	sand			AV 88	6-8'	Soil		
	8.8'							
	o.o Stratified			AV 89	8-10'	Soil	<u>_</u>	
10 -	sand & gravel		Well #5, 2" PVC	AV 90	10-12'	Sail·	PCB = Trace	
			. 2					-
			-					
15 —								
	L7.0'	[AV 91	15-17'	Soil		
	Glacíal cill	┆┝╴		AV 92	17-19'	Soil		
	with clay fines	-	19.5'					
20 -	20.0'							
	Refusal @ 20.0'					 		
	(No peat layer							
	encountered)				+	<u>+</u>		
					-	†		
		1			1	T		
							<u></u>	

Attachment 6

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Field Notes - Monitoring Well Assessment

5/21/98 AEROVOX INC. 038.55 1/14 5/21/90 12/1 AEROVOX INC. 038.55 HONITORING INTEL ASSESSMENT SEATTH TO HATER : Tome Denne 22 50 et (TOP of chasing HW-6 HEADSPACE 0.0 DEPTH TO WATE 7.22 FT 7310- DEPTH: 47.1 FT TOP OF CAKINK, Some services 10 photoh bit ecree Heatsp ANDON OF SEDILIEN DEPOSITOD. 2PM 18 D FT. (Tap of CASULS HW-64 HERDSHALL: 0.0 Sestucia a Botton of when DEPTH TO WATCH . 7.16 Fr TOTAL DOTAL ; 12.49 PT (TOP OF CHEWE) · BEAUT PROTECTUES C MO 34 CLEAN BOTTOH (3FT CARING HT) HEADSPACE APTH TO WATEL MU-24 HERDSHILL 0.0 TOTAL DEPTH 4.98 FT (TOP OF CASAG), ULTAN BOTTON, HEADSPALE. 0.0 [mb 5] - CLEPH WITTOM DEPIT DUAR 1192 Pr TUTAL DEPTH : ZI. OI FR THE SEDIMENT (POTTON OF Stean!

15/21/40 AcROUDY, 140 ... 038.14 13/14 5/21/45 ACROWX, INC. 03.8.44 Ha TA Hw-48 HEARSPACE: 0.0 Doni to Water: 6.40 FT Topac Man Darry 41. 196 FT - SOME SESTMENT (BOTTOM OF NEL KLEED. Clex "WARD " FURH HOUNT CAP @ LIQUID ATTROST HUD HA TINK & ELECTRE CODUTE FIRS-Destal to Warde : Tora Wal Denar HW- 35 HEADSOACE 4 0.0 J In Darni TO WATCH: 3,34 TOPE WER DOTH : 8.48.55 10-41 HEADSPACE - STATINGY OF BOTTOM OF WELL Dirth to extra: 8.36 FT John Mar Derth 23.94 HEADSPACE . 1.0 MW.J DOTH TO WARE 4.80 Er SED HENT O BOTTOM OF WELL Tome Were Deart : 23.94 PT SEDIMENT @ BATTON OR WELL

Attachment 7

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Ground-Water Sampling Logs

Project AGAOUDY Site	Well No. MW-3	Date 5-26-98
Well Depth 18.0 Screen Length/Size 10.0 (0.0	D/O) Well Diameter 2"	Casing MaterialALC
Sampling Device Por ISTALTIC Tubing Type Por	4BTHYLENE	Water Level 4,40
Measuring Point 1 510F OF OUTER COSING Same	pling Personnel MAA PAS	· · · · · · · · · · · · · · · · · · ·
Weather C 80°F SUNNY CS	WTH BREEZE 5-1	5 mpt1
Additional Information ND INNOL WELL C	<u>140</u>	·

Time (Hr \ min.)	pН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)		Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
(320	6.70	19.75	1.467-	113.0	42,92	52.8		5.20.	MONGE (LIGHT)
325	4.22	17.96	1,374	-34.7	+0.91	4,4		5.01	
330	6.30	17.75	1.332	-28.9	+ 0.72	ଜ୍ୟ	1.5	4.96	change propriete
335	6.29	16.95	1.303	- 37.4	+0.61	3.8		5.01	
1340	6.26	17.00	1.295	-39.6	10.57	1.6	¢ 30	5.01	
1345	6.26	16,95	1.218	-36.1	+0.71	1.1		5.02	
1350	6.27	17.01	1.217	-36.9	[†] 0,	2.3		5.01	·
1355	6.26	17.00	1.215	- 39.7	+0,72	1.9	C 4.5	5,00	
1400	6.27	16.99	1.216	-39.6	+0.69	1.8		5.01	
1420	6.26	17.01	1.215	- 37.9	+0.71	2.7		5.02	
(410	SAMPL	f time							
				·					<u></u>

Type of Samples Collected:

VOCS & PCB'S

Additional Notes:

INTTAL PURLIE WASSON - ODANGE COLON (MUN BACTORIA) CLEARISS UP AFTER PURLING

Project_AERWOK Site	Well No	Date 5-26-98
Well Depth 9.44 Screen Length/Size 5.0 (0		Casing Material PUC
Sampling Device		Water Level9.72
Measuring Poin DSIDE WWW CHIM Sar	mpling Personnel MAPAS	
Weather C79°F SUNNY @	SWTH BREEZE 5	- Knph
Additional Information OTEN PLOTEEN	NE CASING BENT C	<u>30°</u>
NO INNER WELL CAP		

Time (Hr \ min.)	pН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)		Dissolved Oxygen (mg \ L)	Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
(320	631	17.45	2.422	-69.8	+0.69	2(9.3	0.5	4.82	REMOLEUM
1323	623	17.65	2:432	-69.3	<u>† 0.93</u>	525.5	21.0	-	(HYDROCHEBON) ODON
1330	6.18	18.46	2 357	-43.6	+2.63	103.7	- 1.5	4.82	<u> </u>
1335	6.13	17.95	2.291	-51.2	+ 2.01	<u>83 9</u>	22.0	4.82	
1340	6.12	18.44	2,322	-54,3	*1.49	62.1	22.50	4.82	
1345	6.12	16.55	2.311	-55.0-	+ 1.11	40.6	*3.W	9.82	
(350	6.12	19.02	2.305	<u>-55,3</u>	+0,97	17.3	= 3.50	4.82	
1400	6.12	9.32	2.304	-55,7	+ 0.63	11.6	140	4.82	· · ·
1410	6.12	18.83	2.292	-53.9	+0,49	10.2	24.5		
1420	6.09	18.46	2.25A	-53.3	+0,23	18.4	25.0	4.84	
1425	SAM	plo	TMG						
					· · · · · · · · · · · · · · · · · · ·				

Type of Samples Collected: $VXS' \neq PCB'_{S}$

Additional Notes:

INITIAL PURGE WATER HAD A DISTINCT HYDROCARBON OBOR PONTICLES OF DOBRIS IN PUNCE WOTEN.

FINAL - PAINT ODOR - WATER WAS CLOBA

Project AEROVOX Site	Well No. MW-7A	Date_ <u>5-26-98</u>
Well Depth 11.24 Screen Length/Size 7,0 (0.		Casing Material_PVC
Sampling Device <u>FallSTALTIC</u> Tubing Type F		Water Level 3.33
Measuring Point N SIDE OF OUTER CHS MA Sa		· · · · · · · · · · · · · · · · · · ·
Weather C70°F PARTLY CLUDY	CSWTH BRIDGE	1-10 MpH
Additional Information NO INNER WELL CO	re	

Time (Hr \ min.)	рН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)		Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
प्रा	6.43	18.67	3.280	- 47.8	0.32	42.4	0.5	3.38	
1502	6.42	19.36	3.292	-87,5	0.53	46-3	1.0	3.39	
190	6.43	18.29	3.656	-89.7	3.05	22.9	*1.5	3,39	
1515	6.40	18.79	3.469	- 89.7	0-88	48.0	12.0	3-38	
1520	6.42	17.71	3.672	-82.5	0.33	13.6	~ 2.5	3.39	
1.525	6.40	18.78	3.533	-83.1	019	26.3	~ 3.0	3.37	
1530	6:40	18.94	3.515	-84.3	3.38	26.8	-3.5	3.38	
1535	6.39	(8.31	3.491	-830	3.14	(1,0	24.0	3A0	
1540	6.39	18.53	3.500	-82.3_	1.96	<u> </u>	14.5	3.38	· · · · · · · · · · · · · · · · · · ·
1545	6.90	<u>(</u> 8.45	3503	-82.3	0.34	23.6	<u> 15.0</u>	3.42	
1550	6.39	18.39	3.972	-01.6	0,24	19.3	*5.5	<u>3.43</u>	
1555		SAMPLE-	TIME						
						·			

Type of Samples Collected:

VOCS & PCBS

Additional Notes:

INITIAL PURCHE WATCH BLACK "POAT" - OTHERNIL OBOR CLEANES UP AFTER A FOW MINUTES FINAL PURCHE WATCH WAS CLEARE.

Project_AbitOM57Site	Well No MW-7	Date <u>5.</u> 26~98
Well Depth 23.94 Screen Length/Size 10.0		Casing Material
Sampling Device CALSTRCTIC Tubing Typ	POLYETHYCONE TUBING	Water Level 7.16
Measuring Poin (1) at 51 cts 12	Sampling Personnel WNA PAS	
Weather @70°F SUNM	·	
Additional Information ND INNER	were cho	

Time (Hr \ min.)	рН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)		Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
1500	6.52	(6.09	1.391	- 42.7	+1.52	62.1	(galions)	7.22	ORANGE
(305	6.33	16.21	1.375	- 49.1	11.40	21.9		7.20	CUEAR
(510	6.27	16.30	1.315	-47.7	+1.26	17.8		7.19	
1515	6.25	16.38	1.278	-46.1	11.18	11.3		7.18	
1520	6.22	(6,83	1.230	-46.9	+hh	2.8	03.0	7.16	
1525	6.20	17.16	1.228	-47.1-	+1.02	3.2		7.15	
1530	6.16	17.46	1.192	-48.9	+0.96	0.3		7.17	
1535	6.18	19.24	1.190	- 44.8	41.00	0.1		7.20	·
1540	6.17	(8,37	1.181	-51.3	+0.86	0.0		7.19	
1545	6.16	16.95	1.187	- 52.9	+0.81	0.1	<u>620</u>	7.19	
1550	6.16	17118	<u>(18)</u>	- 53.8	+0.72	0.0		718	
1610	617	17.23	1.184	-56.7	+ 0, 75	0.2		7.16	
1600	SM	plo-TW	18						
									<u></u>

Type of Samples Collected:

UDC'S ! PCB'S

Additional Notes:

INITIAL PURCHE WATCH - SUGAT ORANGE

Project AEROVOR	_ Site	Well No	Mw-4	Date	5-27-98
Well Depth Screen Leng	gth/Size	Well Diamete	er2''	_ Casing Material_	PUC
Sampling Device PERISTALTIC				_ Water Level	8.44'
Measuring Poin NSING OF WI	on Casing Sampling	Personnel	MLA PAS	· · ·	
Weather <u>C 75° F SU</u>	Why CSar	TK BRGE	1-5	S WPH	
Additional Information MD IN	WER WELL CAR	>			

Time (Hr \ min.)	pН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)		Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
0925	6.51	16.20	0.868	-11.7	+2.76	31.6		8,45	CLEAR
0930	6.39	17.53	0.886	-12.8	+1.67	11.9	~1.0	8.51	
<u>2935</u>	6.34	16.45	0.894	-20,7	+1.53	0.9		g.44	
0940	6.33	(6.76	0.900	-19.8	+1.99	0.5	62.5	8.45	
0945	6.32	16.95	0.901	- 18.3	+1.66	0.3		8.44	
09150	6.32	17.01	0.902	-16.8	+1,74	0.9		8.44	
0955	6.33	16.98	0.903	- 15,7	+1.73	0.8	C 4.0	8.45	
1050	6.32	(7.12	0.903	- 14.8	41.77	0.9		8.44	
1005	6.33	14.66	0.901	-15.0	+1.75	1-1		8.44	
1020	6.32	(7.49	0.903	~14.7	+1.79	0.8		8.45	
	5AV	<u>ріб</u> Піт	6						

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Type of Samples Collected:

VOC'S & PCB'S

Additional Notes:

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Project AEROVOR Site	Well No. MW-6	Date 5-27-98
Well Depth <u>47.1'</u> Screen Length/Size <u>10.0'(0.010)</u>		Casing MaterialPVC
Sampling Device 1011578CTIC Tubing Type 1014 61	MULTING TUBING	Water Level 7.20'
Measuring Point NOINE WITH CASING Sampling F	Personnel WLASPAS	<u> </u>
Weather C75°F SUNNY @SWT7	1 BAGEZE 1-5 MD	H
Additional Information	· · · · · · · · · · · · · · · · · · ·	

Time (Hr \ min.)	рH	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)		Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
1050	5.96	17.99	1.271	+117.8	19.05	47.1		7.26	TRN
(DSS	5,83	16.63	1.258	1 126.4	+ 4.13	12.2	1.0	7.24	
1100	5.70	16.78	1.237	+129.2	+1.5D	6.8		7,22	
1105	5.78	16.89	1.226	+130.5	+2.19	5.0	C2.0	7.21	
1110	5.76	P-16	1.221	+132.4	+2.16	37		7.21	
1115	5,74	16.56	1.219	1134.3	+2.00	1.6		7.20	
1120	5.73	16.00	1.217	+136.6	+2.07	1.9		7.21	
1125	571	16.91	1.218	t140.7	+2.18	2.5		7.20	
1130	5.72	16.90	1.219	+ 191.B	+21193	1.8		7.21	
1135	5.72	16.87	1.218	+ 142.3	12.10	1.6		7.20	
11-30	5,73	16.95	1,219	+ 143,1	+2,13	1.9		7.20	
1(40	S/Mp	E TIM	÷						
						· ~.			

Type of Samples Collected;

UDC'S 2 POBS

Additional Notes:

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Project_AGUND7 Site	Well NoM	N-6A Da	ite_5-27-98
Well Depth 12.49 Screen Length/Size			al NC
Sampling Device ASTASTALTIC Tubing	Type BrybThyLONE	Water Level_	7.78'
Measuring Point NOTCH OUTON CASING	Sampling Personnel		
Weather C75°4 SUNNY	@ SOVTHWEST BIRGE	SET 1-SWPH	·
Additional Information NO INNER	NEU CAP		

рН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)		Dissolved Oxygen (mg \ L)	Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
5.53	15.57	0.174	163.5	3.03	402.2	~,25	7.84	
5.61	IS.9B	0.173	170.4	3,20	264.1	~150	7.89	
5.56	17.05	0.176	(79.6	2.95	62.5	<u>~.75</u>	7.82	
5.57	17.36	0.176	184.0	2.75	45.3	21,00	7.82	
5.13	15.52	0,176	265.0	3.01	42.1	11.25	7.81	
5.32	1630	0.178	259.1	2.62	~	~1.50	7.84	CLOANED YSI
6.22	16.01	0.187	234,4	2.16	12.1	~2.00	7.82	
6.04	16.45	0,189	253.3	2.63	8.2	12.50	7.82	
5.98	(6.35	0.186	260.3	2.66	19.1	12.75	7.84.	
<u>5.88</u>	17.39	0.190	273.3	2.34	2.6	r 3.00	7.84	
SAN	DLG TIN	16 -					·	
				·				
					<u> </u>			
	5.53 5.61 5.56 5.57 5.13 5.32 6.22 6.04 5.9B 5.8B	(°C1°F) 5.53 15.57 5.61 15.9B 5.56 17.05 5.57 17.30 5.13 15.52 5.32 16.30 6.22 16.01 6.04 16.45 5.9B 17.39 17.39 17.39	pH Temperature (°C 1°F) Conductivity (mS 1 cm) 5.53 15.57 0.174 5.61 15.98 0.173 5.56 17.05 0.176 5.57 17.36 0.176 5.54 17.05 0.176 5.57 17.36 0.176 5.13 15.52 0.176 5.32 16.30 0.178 6.22 16.01 0.187 6.04 16.45 0.189 5.98 (6.35) 0.186	pH Temperature (°C \°F) Conductivity (mS \ cm) Reduction (mV) 5.53 15.57 0.174 163.5 5.61 15.9B 0.173 170.4 5.56 17.05 0.176 170.4 5.56 17.05 0.176 170.4 5.56 17.05 0.176 170.4 5.57 17.36 0.176 184.0 5.13 15.52 0.176 265.0 5.32 16.30 0.178 259.1 6.22 16.01 0.187 234.4 6.04 16.45 0.189 253.3 5.9B 17.39 0.190 273.3 5.9B 17.39 0.190 273.3	pH Temperature (°C \°F) Conductivity (mS \ cm) Reduction (mV) Oxygen (mg \ L) 5.53 15.57 O.174 163.5 3.03 5.61 15.9B 0.173 170.4 3,20 5.56 17.05 0.176 170.4 3,20 5.56 17.05 0.176 170.4 3,20 5.57 17.36 0.176 170.4 3,20 5.53 17.05 0.176 170.4 3,20 5.54 17.05 0.176 184.0 2.75 5.13 15.52 0.176 265.0 3.01 5.32 16.30 0.178 259.1 2.62 6.22 16.01 0.187 234.4 2.16 6.04 16.45 0.189 253.3 2.63 5.9B 17.39 0.190 273.3 2.34 6.04 16.35 0.166 260.3 2.66 5.9B 17.39 0.190 273.3 2.34	pHTemperature (°C 1°F)Conductivity (mS \ cm)ReductionOxygenTurbidity (mg \ L) 5.53 15.57 0.174 163.5 3.03 402.2 5.61 $15.9B$ 0.173 170.4 $3,20$ 264.1 5.56 17.05 0.176 170.4 $3,20$ 264.1 5.56 17.05 0.176 170.4 $3,20$ 264.1 5.56 17.05 0.176 170.4 $3,20$ 264.1 5.57 17.36 0.176 184.0 2.75 45.3 5.13 15.52 0.176 265.0 3.01 42.1 5.32 16.30 0.178 259.1 2.62 $ 6.22$ 16.01 0.187 234.4 2.16 12.1 6.04 16.45 0.189 253.3 2.63 8.2 $5.9B$ 17.39 0.190 273.3 2.34 2.6 $5.9B$ 17.394 0.190 273.3 2.34 2.6	pHTemperature (°C 1°F)Conductivity (mS \ cm)ReductionOxygenTurbidity (mg \ L)Removed (gallons) 5.53 15.57 0.174 163.5 3.03 402.2 $\sim,25$ 5.61 $15.9B$ 0.173 170.4 $3,20$ 264.1 ~ 150 5.56 17.05 0.176 (791.6) 2.95 62.5 $\sim,75$ 5.57 17.36 0.176 184.0 2.75 48.3 11.00 5.13 15.52 0.176 268.0 3.01 42.1 11.25 5.32 16.30 0.178 259.1 2.62 \sim ~ 1.50 6.22 16.01 0.187 234.4 2.16 12.1 ~ 2.05 6.24 16.95 0.178 257.1 2.62 $ \sim 1.50$ 6.22 16.30 0.178 257.1 2.62 $ \sim 1.50$ 6.24 16.91 0.187 234.4 2.16 12.1 ~ 2.05 6.24 16.95 0.186 263.3 2.63 8.2 ~ 2.55 $5.9B$ 17.84 0.190 273.3 2.34 2.6 19.1 12.75 $5.9B$ 17.84 0.190 273.3 2.34 2.6 $1.3.00$	pHTemperature ($^{\circ}$ C1 $^{\circ}$ F)Conductivity (mS1cm)Reduction (mV)OxygenTurbidity (NTU)Removed (gallons)Level (ft. BGS)5.5315.57O.174163.53.03402.2~.257.845.6115.9B0.173170.43,20264.1~.1507.845.5617.050.176179.62.9562.5~.757.825.5717.360.176184.02.7545.31.007.825.5315.520.176265.03.0142.111.257.845.5216.300.178259.12.62~~1.895.3216.300.178259.12.62~~1.896.2216.010.187234.42.1612.1~2.007.826.0416.450.189253.32.638.2~2.507.845.9B17.3940.190273.32.342.61.3.007.845.9B17.3940.190273.32.342.61.3.007.84

Type of Samples Collected: NDC's & PCB's

Additional Notes:

INITIAL PURILE WATCH - ONANGE/BROWN COLOR, NO DOOR.

Project AEALNDZ	Well No	N-5 Date	5-27-98
Well Depth 21.01 Screen Length/Size 10.01	0010) Well Diameter	Casing Material	PUC
Sampling Device POILSTALTIC Tubing Type			12.19
Measuring Point Anow on with Chinh Sa	mpling Personnel W(2A		
Weather C 80°F SUNVY CS	SWTH BROSZE	1-Smp4	
Additional Information NO INNON WELL	Cap		

Time (Hr \ min.)]	рН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)		Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
1315	6.48	13.74	0.323	266.4	8.33	74.0	20,25	12.20	
1320	6.29	14.45	0.323	273.6	8,00	53.5	0.50	(2.20	MUMORSO RATE
1325	6.26	14.71	0.323	278.1	<i>ት.</i> ይግ	29.8	~(.00	-	
1330	6.21	15.69	0.323	289,9	7.75	18.0	21.50	12.2	
1335	6.17	15.67	0.324	295.9	7.74	8.2	2.00	[2.2]	
1340	6.16	15.61	0.324	303.B.	7.68	5.4	-2.50	12.21	
1345	6.13	15,48	0.324	307.7	7,65	7.9	1300	12.21	
1350	6.09	(6.25	0.324	312.5	7.57	4.0	n 3.50	12,21	·
1355	6.09	16.41	0.324	316.0	7,51	4,9	7 4.00	2,21	
1400	SAN	pigs	TIME						
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Type of Samples Collected:

VUC'S & PCB'S

Additional Notes:

WELL PITS = 1.1 ppm INITIAL PURGE WATCH - ORIONIAG (BROWN VORY TURBED, NO ODOR

Project	Well No. MW-2	Date_5-27-98
Well Depth 22.50 _ Screen Length/Size 10.0 (0.010	Well Diameter	Casing MaterialPVC
Sampling Device BUSTALTL Tubing Type Row	15THYLENE TUBING	
Measuring Point SIDE OF WITH CASING Sampling	g Personnel WAD PAS	
Weather C75°F SUNNY CSW		DWPH
Additional Information ND INNER WELL	_ CAP	

Time (Hr \ min.)	рН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)		Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
1440	6.33	15,67	3.294	*58.2	1.21	111.4	<u>n 0.10</u>	6.48	
1445	6:42	16.398	3.296	+2.4	0,68	7,3	- 0.25	6.14	
IASD	6.45	16.98	3.305	-15.0	0.53	55	<u>~ 0,75</u>	6.14	
1455	6.55	17.38	3 307	-25.2	0.51	5.3	11.00		- <u></u>
1500	6.48	<u> </u>	3.300	-26.2	0.48	6.2	~ 1.50	t 	CHANLIG BATTORY
1505									
1515	6.50		3,294	-3.2	0.65	0،4	12.0	6.19	
1520	6.50	19.38	3 304	-7.6	0.73	6.3	12.25	6.14	
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1520	SAMP	LE- 7	IMG-		_		_		
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Type of Samples Collected:

UDC'S EPCB'S

Additional Notes:

INITIAL PURGE WATER - ONENGY BROWN Some DEBALS IN WATER * SEA WATER ODOR *

Project_AFROVDX	Well No	MW-2A	Oate_	5-27-98
Well Depth 4,98 Screen Length/Size	Well Diameter_	2"	Casing Material	NC
Sampling Device AGAISTACTIC Tubing Type POR	LY 5TH YLONG		_ Water Level	3.52
Measuring Point NOUTER CABING Samp	ling PersonnelW	NA PAS		
Weather C75°F SUNNY	CSWTH C	16676	1-SMPM	
Additional Information NO INNER WELL	CAP	<u></u>		

Time (Hr \ min.)	рH	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)		Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
(435	6.72	21.98	0.983	-111.2	+3.69	20.9		3.64	dente
1440	6.92	21.36	1.627	-119.7	+ 2.53	11.2		3.64	
1445	7.01	21.69	1.055	-118.1	+2.74	0.5		364	
1450	7.03	21.80	1.101	-119.1	+2.62	0.7	22.0	3.64	
1455	7.04	21.89	1.111	-121.3	+ 2.94	0.9		3.62	
1500	7.03	22.09	1,142	- 122.4	+3.07	0.8		3.62	
1505	7.03	22.06	1.131	-123,2	+3.06	1.1		3.62	
1510	7.03	21.92	1.127	-123-9	<u>+3.</u> 10	0,9		3.62	
1515	7.02	21.84	1.126	-124.1	+3.11	0.6	64,0	363	
1520	7.02	<u>21.80</u>	.126	-125,2	+ 3.12	0.5		3.62	
									· <u> </u>
1525	7.02	21.41	1.125	-126,7		0.9		3.62	
1570	7.02	21.95	1.126	-127.1	4313	ወት		3.62	
1530	SAM	PLG TH	18						
 									
 									
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Type of Samples Collected:

voc's { pcb's

Additional Notes:

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TOTAL USING PURING @ 9.50'

Project	AEROU	07	Site		Well No.	MW	-8 <u>5</u>		Date	5-27	-98
-		Screen Leng			Well Diame	·····	2"	Casing			
		STALITIC				5_700	smy	Water L	evel	340'	
Measuring	g Point 🕖	SIDE OF 1	WITTER CASIN	Sampling	Personnel	WA-	PAS				
Weather_	070	'F SUNN	y C.	SWTH	BREST	5	1-10	MPH			
Additional	I Information	ND	INNER	Wou	CAP						

Time (Hr \ min.)	рН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)	Dissolved Oxygen (mg \ L)	Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comments
1550	8.21	20,91	0.730	$-1(d_{0.0})$	2.23	110.4	21.0	3.98	
1555	8.64	20.82	0.797	-182.5	1.58	101.6	~1.5	3.72	
1600	8.81	20,84	0.8A1	-179.3	+1.38	68.1		3.61	
1605	8.ኅ3	20.87	0.863	-177.0	+ 1.31	31.2	~2.0	3.60	
1610	8.96	20.98	0.869	-177.2	+1.23	29.1		3.60	<u></u>
1615	8.99	20. 80	0.872	-177.1.	+1.17	[3.]		3.60	
1.620	9.02	20.89	0.881	-179.5	t1.11	10.1	23.0	3.50	
1625	9.02	20.90		-171.1	+1.03	9.4		3.58	
1630	9.03	20.91	0.889	-170.9	+1.04	11.2	135	3.58	
1640	Samp	LE TIM	6						

Type of Samples Collected:

VDC'S & PCB's

Additional Notes:

IN THAL PURGE WATTER - BLOCK VORY SILTY WITH DEBRIS

Project_AGROVOK Site	Well No WW- 4#	1 Date_ 5-27-98
Well Depth 9.52 Screen Length/Size 5.0' (0.0(0) Well Diameter 2"	Casing Materialρνc
Sampling Device PELISTACTIC_ Tubing Type	POLYETHYLENE TUBI	<u> M</u> Water Level <u>5.0</u> 4 "
Measuring Point BRRW ON WIGE COSING S	ampling Personnel <u>WA PA</u>	5
Weather C75°F SUNNY C	SWTH BREEZE 1	-Supri
Additional Information NO INNER WELL	CAP.	

Time (Hr \ min.)	рН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)	Dissolved Oxygen (mg \ L)	Turbidity (NTU)	Volume Removed (gailons)	Water Level (ft. BGS)	Comments
0915	6.12	18.35	0,200	(02.3	2.53	10.7	10.25	5.14	
0920	6.15	18.39	0.262	112.1	5.53	4.9	-0,50	6.34	
A25_	6.02	18A1	0.203	121.0	5.97	2.9	-0.75	7.59	
0930	5.91	18.42	0.208	125.0	613	2.6	21,00	8.62	
0935	5.86	19.03	0.216	54.5	4.37	5.9	-1.25	Dry	Dry@ 0937
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SAMP	16-	TIME	5 28	398	0630				
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Type of Samples Collected:

Additional Notes:

INITION PURGE USAL TURBID, ORANZE-RED COLOR "IRON BACTERIA" * WELL WENT DAY AT C. 0937 ON 5/27/98

WILL SAMPLE WHEN WELL RECOVERS

SMPLS: 5/28/98 AT 0630 - JUST GNOVGH WATER

Project_AEROVOX	Site	Well No MV	V-4B	Date	5/28/98
	reen Length/Size_100'			Casing Material	PVC-
	LTIC Tubing Type_			_ Water Level	6.68'
Measuring Point (N) &					
Weather <u>@G8°</u> F_		NEST BREEZE	CI-SMPH		
Additional Information	NO INNER WEL	<u> </u>		·	

Time (Hr \ min.)	рН	Temperature (°C \ °F)	Specific Conductivity (mS \ cm)	Oxidation/ Reduction (mV)		Turbidity (NTU)	Volume Removed (gallons)	Water Level (ft. BGS)	Comme	ents
0655	5.90	15.70	1.511	214.7	4.69	3.1	~0.50	9.42:	RESURE	FLOW
0700	<u>इ.</u> ७५	15.63	1.501	224.8	3.05	9.2	~ 1.00	9.38		
2050	5.77	15.69	1.498	228.5	2.96	2.6	~ 1.25	9.30		
0710	5.73	15.64	1.492	234.7	2.86	1.1	~1.50	୧.୦୦		
5715	5.72	15.41	1,465	291.2	2.82	0.4	-1.75	9.39		
0720	5.55	15.57	1.463	245.8	2.26	0.0	~2.00	9.42		
0725	5.45	15.61	1.448	247.7	1.98	<u> </u>		9.12		
0730	5.43	15.63	1.437	251.7	1.76	1.2	~ 2.50	9.11		
0735	5.36	15.79	1.929	253.6	1.68	0.9	- 2.75	9.43		
0740		SAMPLE	·							
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						·				
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Type of Samples Collected: VOCs, PCBs

Additional Notes:

Notes: INITIAL CLOBE, NO ODOR, NO DEBRIE.

Attachment 8

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Field Notes - Ground-Water Investigation

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AGROVER 1	5/26/98 AERONOR 21
TUBSDAY MAY 26-1998	····· - <mark>┝</mark> ╎ ╎· <mark>╞╶╷╴╛╶┝╞╪╛╌</mark> ┝╶┤╶┿ <u>╄╶</u> ╪╶┽╶┙╌╎╸ <mark>╞╶┼╶┥╧╸</mark> ┯┱┈╺╞╶╎ └ ││ ⁴ O
5ATTAIGE : AM 60°F PORTEY CLUDY	WRA/POD WET PETON SEWASA
P.M. C 80°F SUNNY BREFE	at Attack - Poisa is in
	CHARKIE OF ANY ENVIRONMENT ISSUES
toon Bussiand, Burgh & LOG	ON-5078-
MICHAGE R ARVOULING (MRA)	
PSER A SPENCE (PAS)	STRUGTA GAUST MRD PAS A
	ONCL JOUR UK THE FOLLION
	And criticizes experines
0600	
Mrs & PDS ARRIVE AT PORMOSTER	MADIERS GATHERED ALL EQUIPMENT
INTER MATTONIC ARPENT	PEREM FROM SZWATA'S CAFICET
0635 LEAVE For Now BEAREARD	Also EQUIPMENT TRUE WAR SONT!
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RHODO ISLAND,	
	MON / PASS DEP-SUTS DO GET
	Supplies - WATER BOTTLES / ICF
1130	- PRASTIC/ BIEROTS / BOCK 163
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AT AGRONDE - NEW BENERO	1295
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STRACTING USUS MW-3 LMW-3A	plus @ 75°F Sunny
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1950	GATTHERED DRUMS For Docan/PURGE
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¥.00	0900 FOD GX ACILIUOS
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status AGOVOR 21	5/27/428	AGROVOR	
1645 FINISTON SIMPUNG	Sonpline	1N ROAMATZON	
CUSTER MW-85		╡ ╷ _┍ ╷┨╘ _{╍┿╍┥} ╎┉┠┈╎┠╌ ╿┍ ┿┽╀╶┼╌┦	╶╴╴╎╷╷╷
	2878-	WERL ID	TRIME
* NO WATCH IN MW-4A-	52698	MW-3	1410
Will they IN morenting	5/26/98	1 mus + 3A	1425
	5/26/98	MW-7A	1555
1700	5/26/98	mw-7+	1600
BECAN to ROCK COSTEM			
to BE SHIPPED to LOB	527198	MW-4	(010
	5/27/98	MW-6	[140
1810	5/27/98	mw-6A	1145
MURIT PAS CURSITIE	527 98	MW-5	1400
GNNEW TO to FGD - 5X IN TRUNADOS	5 27 98	MW-2	1520
₩₩A\$\$\$.	5 27 48	MW+2A	1530
1900 ARRIVE AT FOD - THE	5/27/98	MW-85	1640
1915 LOAUS FOD - 52			
	520 90	MW-AA	0630
2005 BERING AT HOTEL	5 20 98	MW - 48	0740
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5/27/98 ACROVI	×	4/	5 AEROVOR
WATER LEVERS			THURSDAY WAY 28- 1998 WATTION : C. 80°F SUNNY
5/26/98			
WELL ID	WATEN LITER	·····	MATIRAS - BARING ON +SITE
MW-2	5.82		
MW-24-	3.52		+ WILL TRUY to sample MW-AA
WW-3	4,40		
MW-34	4.72		0630
MW-4	8.44		Samples MW-94
MW-4A	5.04		NO FIGED PARAMETERS TOKEN
MW-5	1219		LACK US WATON *
MW-G	7.20		
MW-6A	7.78	· ··· ·	RINSF BLANK- PETE RB52898
MW-7	7.16		
MW-7A	3.33		
MW-1B	6.68		MW-9B - 0740
MW-85	340	· •	
· · · · · •			MRO PAS WILL BEGIN TO
			ROCK UP GOULDMENT & RACK
* TOP UF WITCH	casing to		CONTE Ron LAB -
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Attachment 9

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July 15, 1998 letter from GAF Engineering, Inc. Presenting Elevations for Monitoring Well Casings PROFESSIONAL ENGINEERS

PROFESSIONAL LAND SURVEYORS

July 15, 1998

Aerovox 740 Belleville Avenue New Bedford, MA 02745

Attention: Mr. Peter Szwaja

Re: Monitoring Well Elevations G.A.F. Job No. 98-4392

Dear Mr. Szwaja:

G.A.F. Engineering, Inc. completed a level run to determine the elevations of well onesings to monitoring wells placed around the Aerovox Plant at 740 Belleville Avenue. The well locations are shown on a plan entitled "Soil Boring/Groundwater Monitoring Well Locations by BBL Blasland, Bouck & Lee, Figure 5." <u>Please note that MW 6A is marked as MW 6 in the field and MW 6 is marked as MW 6A.</u>

All elevations were taken at the north side of the casings and all elevations are in feet and are referenced to mean sea level datum per the site benchmark of known elevation of 4.76 feet, at a point on sheet piling near Well #2.

Readings were taken at the north side of the well casings and were taken at both the exterior steel casing and the interior PVC casting at each monitoring well site. The results are as follows:

Monitoring Well #	Exterior Steel Casing	Interior PVC Casing
MW 2	6.89	6.30
MW 2A	6.61	· 5.78
MW 3	5.91	6.23
MW 3A	8.13±	6.8±

Note: Well 3A is set at $30^{\circ} \pm$ angle to ground.

Tel. (508) 748-0252

454 Wareham Street + P.O. Box 953 + Marion, MA 02738

Fax (508) 748-0542

Aerovox Page 2 July 15, 1998 Re: Monitoring Well Elevations

Monitoring Well #	Exterior Steel Casing	Interior PVC Casing
MW 4	10.97	8.29
MW 4A	10.73	8.48
MW 4B	8.99	8.86
MW 5	15.48	14.32
MW 6*	9.21	8.16
MW 6A=	9.75	8.80
* As marked in field.		
MW 7	7.54	5.73
MW 7A	7.29	6.42
MW 8S	5.76	5.32

Please contact me if you have any questions and/or require additional information.

Sincerely,

G.A.F. Engineering, Inc.

Kevin W. Forgue

KWF:fd

Precision and Accuracy of Elevation Measurements

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Based on BBL's August 25, 1998 telephone conversation with Mr. Kevin Forgue of G.A.F. Engineering, Inc., the accuracy and precision of the monitoring well elevation measurements (presented in the preceding letter) is 0.01 feet.

Attachment 10

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Aerovox Site Post-Closure Monitoring Program Data



740 Belleville Avenue New Bedford, MA 02745 TEL (508) 994-9661 FAX (508) 999-1000

Page 1 of 46

IF YOU DO NOT RECEIVE ALL PAGES, PLEASE CALL US AS SOON AS POSSIBLE

То	Kathy Geraci	From	Peter Szwaja
Company	BBL	Subject	Well data
Fax No.	171-98	Date	July 24, 1998

Dear Kathy:

Monitoring Well data

March 1998 March 1997 September 1996 March 1996 September 1995 March 1995

Please call me at 508-910-3591 if you require additional data.

Regards,

Fax Message

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JUL-24-98 FRI 10:15 AM AEROVOX, NEW, BEDFORD



01-0827-05-0051-001

March 31, 1998

U.S. Environmental Protection Agency Region 1 John F. Kennedy Building Boston, Massachusetts 02203

Attention: Mr. Frank Ciavattieri

Reference: Aerovox Site Post-Closure Monitoring, March 11, 12 and 13, 1998

Dear Mr. Ciavattieri:

Enclosed are the results of the water level monitoring and cap inspection conducted at the Aerovox site by SAIC Engineering, Inc. during the March 1998 full moon period.

The next inspection and round of water level readings are scheduled for the September 1998 full moon period. Please call if you have any questions.

Sincerely,

SAIC ENGINEERING, INC.

Allen F. Drvis, P.E. Project Manager

Enclosures

cc: G. Monte, DEP/SERO P. Galvani, Ropes & Gray P. Szwaja, Aerovox

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Elgh

Time of Tide: 0629 Data: March 11, 1998 Time of Readings: 0617 - 0705

LOCATION	TOP OF CASING ELEVATION (1) (2)	BASELINE ELEVATION (3)	CURBENT READING	CURRENT ELEVATION	CHANGE IN BLEVATION vs. BASELINE	RANGE OF ELEVATION OVER PREVIOUS 164 MONTHS (4)
Tide Gauge	4.76		2,53	2.23		
Well No. 2	6.92		4.50	2.42		····
Well No. 2A	6.67	2.62	3,34	3.33	0.71	1.51 - 4.00
Well No. 3	6.95		4.57	2.38		
Well No. 3A	8.26	1.86	5.66	2.60	0.74	0.78 - 3.31
Well No. 4	10.99		8.43	2.56		
Well No.4A	10.78	2.28	7.46	3.32	1.04	1.60 - 3.68
Well Xo. 7	7.59		4.99	2.60		
Well No. 7A	7.33	2.60	4.29	3.04	0.44	2,38 - 3,40

NOTES:

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Weather: 25 degrees F, Sunny Readings by: David Minese Aifiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

- (1) All readings and elevations are in feet and are referenced to mean sea level datum.
- (Z) Tide elevation is measured in reference to a known elevator of 4.76 ft, at a point on sheet pilling near Well No. 2.
- Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985. (7)
- (4) (5) Numbers in this column are the range of recorded elevations from July 1984 through March 1998,
- Well 2A was cleaned to remove semi-aqueous encrustation (as reported in the Fall 1997 report) on March 10, 1998 by SAIC Engineering, prior to water level readings (6) Soundings of all wells were conducted by SAIC Engineering on March 13, 1998.
 - The soundings indicate that Well 24 is almost slited in and should be purged; Well 3 is partially slited and also should be purged.

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WATER LEVEL READINGS

AEROVOX PLANT SITE NEW REDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1300

Date: March 11, 1998 Time of Readings: 1235 - 1323

LOCATION	TOP OF CASING ELBVATION (1)(2)	BASELINE ELEVATION (3)	CURRENT BEADING	CURRENT ELEVATION	CHANGE IN ELEVATION VS. BASELINE	RANGE OF ELEVATION OVER PREVIOUS 164 MONTHS (4)
Tide Sauge	4.76		Вгу			
Well No. 2	6.92		5.04	1.88		
Well No. 2A	6.67	2.62	3.35	3.32	0.70	1.51 - 4.00
Well No. 3	6.95		5.43	1.52		1.51 * 208
Well No. 3A	8.26	1.86	5.35	2.91	1.05	0.78 - 3.31
Well No. 4	10.99		10.21	0.78	1.00	0.10 • 3.31
Well NoAA	10.78	2.28	7.47	3.31	1.03	1.60 - 3.88
Well No. 7	7.59		6.88	0.71	2.54	1.00 . 2.66
Well No. 7A	7.33	2.60	1.29	3.04	0.44	2.38 . 3.40

NOTES:

Weather: 25 degrees F. Sunny Readings by: David Minese Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

- (4) All readings and elevations are in feet and are referenced to mean sea level datum.
- $\langle 2 \rangle$ Tide elevation is measured in reference to a known elevaton of 476 ft, at a point on sheet piling near Well No. 2.
- (J) (4) Basellus elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March 1998.

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TABLE 2A

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Bigh Time of Tide: 0710 Date: March 12, 1998 Time of Readings: 0635 - 0732

LOCATION	TOP OF CASING ELEVATION (1)(2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN Elevation 73. Baseline ?	RANGE OF ELEVATION OVER PREVIODS 164 MONTHS (4)
Tide Gauge	4.76		2.62	2.14		
Well No. 2	6.92		4.56	2.36		
Well No. 2A	6.67	2.62	3,46	321	0.59	1.51 - 4.00
Well No. 3	6.95		4.65	2.30		1,07 - 3,00
Well No. 3A	8.26	1.86	5.73	2.53	0.67	0.78 - 3.31
Well No. 4	10.99		8.51	2.48		4.70 - 3.51
Well No.4A	10.78	2.28	7.54	3.24	0.96	1.60 - 3.88
Well No. 7	7.59		5.06	2.53		
Well No. 7A	7.33	2.60	4.32	3.01	0.41	2.38 - 3.40

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NOTES:

Weather: 20 degrees F. Sumy Readings by: David Minese Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

- (1) (2) All readings and elevations are in feet and are referenced to mean sea level daium.
- Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
- (3) (4) Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March 1998.

TABLE 28

WATER LEVEL READINGS

ABROVOX PLANT SITE NEW BRDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1314 Date: March 12, 1998 Time of Readings: 1245 - 1335

LO CATION	TOP OP Casing Blevation (1) (2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN BLEVATION VS. BASELINB	RANGE OF Elevation Over Previous 164 Nontes (4)
Tide Gauge	4.76		Dry			
Well No. 2	6.92		5.10	1.82		
Well No. 2A	6.67	2.62	3.43	3.24	0.62	1.51 - 4.00
Well Na. 3	6.95		5.37	1.58		
Well No. 3A	8.26	1.86	5.83	2,43	0.57	0.78 - 3.31
Well No. 4	10.59		10.17	0,82		
Well Ro.4A	10.78	2.28	7.54	3.24	0.96	1.60 - 3.88
Well No. 7	7.59		G.88	0.71		
Well No. 7A	7.33	2.60	4.31	3.02	0.42	2.38 - 3.40

NOTES:

Weather 20 - 25 degrees F. Cloudy/Flurries Readings by: David Minese Affiliation: SAIC Engineering , Inc. 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

- All readings and elevations are in fest and are referenced to mean sea level datum.
- The elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
- (1) (2) (3) (4) Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March 1998.

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TABLE 3A

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: High Time of Tide: 0749 Date: March 13, 1998 Time of Readings: 0729 - 0825

LOCATION	TOP OF CASING ELEVATION (1) (2)	BASELINE ELEVATION (3)	CURRENT RBADING	CUBRENT BLEVATION	CHANGE IN BLEVATION VS. BASELINE	BANGE OF ELEVATION OVER PREVIOUS 164 MONTHS (4)
Tide Gauge	4.76		3.10	1.66		
Well No. 2	6.92		4.88	2.04		·····
Well No. 2A	§.67	2.62	3.65	3.02	0.40	1.51 - 4.00
Well No. 3	6.95		4.92	2.03		7,01 4,00
Well No. 3A	8.26	1.86	5.84	2.42	0.56	0.78 - 3.31
Well No. 4	10.99		8.80	2.19		41.0 4.01
Well No.4A	10.78	2.28	7.61	3.17	0.89	1.60 - 3.88
Weil No. 7	7.59		5.36	2.23		
Woll No. 7A	7.33	2.G0	4.36	2.97	0.37	2.38 - 3.40

NOTES:

Weather: 10 - 15 degrees F. Sunny/Cold Readings by: David Minese

Milliation: SAIC Engineering , Inc., 101 Bast Grove Street, Middleboro, Massachusetts, 02346

- (1) All readings and elevations are in fest and are referenced to mean sea level datum.
- (2) Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
- Baseline elevations shown for shallow wells Nos. 24, 34, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985. (3) (4)
- Numbers in this column are the range of recorded elevations from July 1984 through March 1998.

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WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACUUSETTS

Tide Stage: Low Time of Tide: 1329 Date: March 13, 1998 Time of Readings: 1300 - 1359

LOCATION	TOP OF CASING BLEVATION (I) (2)	BASELINE ELEVATION (3)	CURDENT READING	CURDENT ELEVATION	CHANGE IN ELEVATION 75. BASELINB	RANGE OF BLEVATION OVER PREVIOUS 164 MONTHS (4)
Tide Gauge	4.76		Dry			
Well No. 2	6.92		4.93	1.99		
Well No. 2A	6.57	2.62	3.55	3.12	0.50	1.51 - 4.00
Well No. 3	6.95		5.65	1.30		
Well No. 3A	8.26	1.86	5.96	2.30	0.44	0.78 - 3,31
Well No. 4	10.99		50.44	0.55		0.10 - 0.31
Well No.4A	10.78	2.28	7.62	3.16	0.68	1.60 - 3.88
Well No. 7	7.59		7.18	0.41		
Woll No. 7A	7.33	2.GD	4.35	2.98	0.38	2.38 - 3.40

NOTES:

Weather: 20 - 25 degrees P. Sunny Readings by: David Minese Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleborn, Massachusetts, 02346

FOOTNOTES:

(1)	AD readings and elevations are in feet and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet pilling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through March 1958.

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SAIC Engineering, Inc. A Subsidiary of Science Applications International Corporation

An Employee-Owned Company

April 16, 1997

01-0827-05-0051-003

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U.S. Environmental Protection Agency Region 1 John F. Kennedy Building Boston, Massachusetts 02203

Attention: Mr. Frank Ciavattieri

Reference: Aerovox Site Post-Closure Monitoring, March 22, 23 and 24, 1997

Dear Mr. Ciavattieri:

Enclosed are the results of the water level monitoring and cap inspection conducted at the Aerovox site by SAIC Engineering, Inc. during the March 1997 full moon period.

The next inspection and round of water level readings are scheduled for the September 1997 full moon period. Please call if you have any questions.

Sincerely,

SAIC ENGINEERING, INC.

Allen F. Davis, P.E.

Project Manager

Enclosures

- cc:
- G. Monte, DEP/SERO P. Galvani, Ropes & Gray P. Szwaja, Aerovox

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WATER LEV	SL READIN	es													
ABROVOX PI NEW BEDFO															
Tide Stage:															• -/

Time of Tide: 0642 Date: March 22, 1997 Time of Readings: 0630 - 0727

LOCATION	TOP OF CASING BLEVATION (1) (2)	BASELINE BLEVATION (3)	CURRENT BEADING	CURRENT ELEVATION	EHANGE IN ELEVATION VS. BASELINE	RANGE OF ELEVATION OVER PREVIOUS 152 MONTHS (4)
Tide Gauge	4.76		1.73	3.03		
Well No. 2	6.92		4.60	2.32		
Well No. 2A	6.67	2.62	3.35	3.32	0.70	1.51 - 4.00
Well No. 3	6,95		4.64	2.31		1.01 - 4.00
Well No. 3A	8,26	1.86	6.03	2.23	0.37	0.78 - 3.31
Well No. 4	10.99		8.70	2.29		0.10 * 0.31
Well No.4A	10.78	2.28	7.69	3.09	0.81	1.60 - 3.88
Well No. 7	7.59		5.03	2.56	0.04	1.00 - 3.00
Well No. 7A	7.33	2.60	4.37	2.96	0.36	2.38 - 3.40

NOTES:

Weather: 40 degrees F. Rain Readings by: Mark Panni Alfiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

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(I)	All readings and elevations are in fect and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through March, 1997.

FAX NO.

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WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1239 Date: March 22, 1997 Time of Readings: 1235 - 1301

LOCATION	TOP OF Casing Elevation (1) (2)	BASELINE BLEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN BLEVATION VS. BASBLINE	RANGE OF ELEVATION OVER FREVIOUS 152 MONTHS (4)
Tide Gauge	4.76		Dry			
Well No. 2	6.92		5.15	1.77		···
Well No. 2A	6.67	2.62	3.35	3.32	0.70	1.51 - 4.00
Well No. 3	6.95		5.38	1.57		
Well No. 3A	8.26	1.86	5.59	2.67	0.81	0.78 - 3.31
Well No. 4	10.99		10.05	0.94		
Well No.4A	10.78	2.28	7.61	3.17	0.89	1.60 - 3.88
Well No. 7	7.59		6.75	0.84		
Well No. 7A	7.33	2.60	4.40	2.93	0.33	2.38 - 3.40

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NOTES:

Weather: 40 degrees F, Partly Sunny Readings by: Mark Panni Affiliation: SAIC Engineering, Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

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(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.

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TABLE 2A												
WATER LEVE	L READIN	65										
ABROVOX PL	ANT SITE											
Tide Stage: E Time of Tide: Date: March I	: 0721											

LOCATION	TOP OF CASING ELEVATION (1) (2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN BLEVATION vs. BASELINE	RANGE OF ELEVATION OVBR PREVIOUS 152 MONTHS (4)
Tide Gauge	4.76		2.09	2.67		17
Well No. 2	6.92		4.79	2.13		
Well Xo. 2A	6.67	2.62	3.57	3.10	0.48	1.51 - 4.00
Well No. 3	6.95		4.87	2.08		1104 1100
Well No. 3A	8.26	1.86	6.13	2.13	0.27	0.78 - 3.31
Well No. 4	10.99		8.74	2.25		0.76-0.01
Well No.4A	10.78	2.28	7.60	3.18	0.90	1.60 - 3.88
Well No. 7	7.59	1	5.19	2.40		1.00 0.00
Well No. 7A	7.33	2.60	4.45	2.88	0.28	2,38 - 3,40

NOTES:

Time of Readings: 0703 - 0747

Weather: 30 degrees F. Clear, Windy Readings by: Mark Panni Alfillation: SAIC Engineering , Inc., 101 Bast Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

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(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	The elevation is measured in reference to a known elevator of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through March, 1997.

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																	· · ·
TABLE 3A																	í +
WATER LEVE	L READING	ъ															
AEROVOX PL NEW BEDFOR		HUSETTS															
Tide Stage: H Time of Tide Date: March Time of Read	: 0800 24, 1997	- 0821						-									
LO	CATION		TOP Cast Eleva:	NG		ASELINE EVATION		URRENT BADING		CURRENT ELEVATION		CHANGE I BLEVATIO VS. BASBLI	N		RANGE OF ELEVATION OVER		

LOCATION	TOP OF CASING ELEVATION (1)(2)	DASELINE BLEVATION (3)	CURRENT RBADING	CURRENT ELEVATION	CHANGE IN BLEVATION vs. BASBLINE	RANGE OF Elevation Over Previous 152 Montes (4)
Tide Gauge	4.76		2.60	2.16		
Well No. 2	6.92		4.98	1.94		
Well No. 2A	6.67	2.62	3.71	2.96	0.34	1.51 - 4.00
Well No. 3	6.95		5.07	1.88		
Well No. 3A	8.26	1.85	6.31	1.95	0.09	0.78 - 3.31
Well No. 4	10.99		8.90	2.09		0.0 0.01
Well No.4A	10.78	2.28	7.69	3.09	0.81	1.60 - 3.88
Well No. 7	7.59		5.39	2.20		100 0.00
Well No. 7A	7.33	2.60	4.47	2.86	0.26	2.38 - 3.40

NOTES:

Weather: 35 degrees F, Clear Readings by: Mark Panni Affiliation: Saic Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

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(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.

TABLE 3B
WATER LEVEL READINGS
WATER LEVER CONDUCTOR
100 AUTO
ARROVOX PLANT SITE
NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1336 Date: March 24, 1997 Time of Readings: 1314 - 1346

LOCATION	TOP OF CASING BLEVATION (1) (2)	BASELINE ELEVATION (3)	CURRENT READINO	CURRENT ELEVATION	CHANGE IN Elevation v9. Baseline	BANGE OF Elevation Over Previous 152 montes (4)
Tide Gauge	4.76		Dry		· · · · · · · · · · · · · · · · · · ·	
Well Ko. 2	6.92		5.20	1.72		
Well No. 2A	6.67	2.62	3.70	2.97	0.35	1.51 - 4.00
Well No. 3	6.95		6.05	0.90		
Well No. 3A	8.26	1.86	5.55	2.71	0.85	0.78 - 3.31
Well No. 4	10.99		9.56	1.43		0.10 - 0.01
Well No.4A	10.78	2.28	7.67	3.11	0.83	1.60 - 3.88
Well No. 7	7.59		7.54	0.05		1.00 3.00
Well No. 7A	7.33	2.60	4.45	2.88	0.28	2.38 - 3.40

NOTES:

Weather: 40 degrees F, Clear, Light Wind Readings by: Mark Panni Affiliation: SAIC Bugineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

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(1)	All readings and elevations are in fect and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through March, 1997.

JUL-24-98 FRI 10:21 AM AEROVOX. NEW, BEDFORD

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SAIC Engineering, Inc. A Subsidiary of Science Applications International Corporation

An Employee-Owned Company

October 24, 1996

2827.961023.011 01-0827-05-0051-003

U.S. Environmental Protection Agency Region 1 John F. Kennedy Building Boston, Massachusetts 02203

Attention: Mr. Frank Ciavattieri

Reference: Aerovox Site Post-Closure Monitoring, September 25, 26, and 27, 1996

Dear Mr. Ciavattieri:

Enclosed are the results of the water level monitoring and cap inspection conducted at the Aerovox site by SAIC Engineering, Inc. during the September 1996 full moon period. We note that at the time of water level monitoring in September 1996 NOAA tide charts for New Bedford show record or near record high and low tide elevations.

The next inspection and round of water level readings are scheduled for the March 1997 full moon period. Please call if you have any questions.

Sincerely,

SAIC ENGINEERING, INC.

Allen F. Davis, P.E. Project Manager

Enclosures

cc: G. Monte, DEP/SERO P. Galvani, Ropes & Gray P. Szwaja, Aerovox

TABLE IA

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Eigh Time of Tide: 0645 Date: Sept. 25, 1998 Time of Readings: 0630 - 0712

LOCATION	TOP OF CASING ELEVATION (1) (2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN BLEVATION VB. BASELINE	RANGE OF RLEVATION OVER PREVIOUS 146 MONTHS (4)
Tide Gauge	4.76		0.93		<u> </u>	
Well No. 2	6.92		3.91	3.01	·	
Well No. 2A	6.67	2.62	3,19	3.48	0.86	1.51 - 4.00
Well No. 3	6.95		4.08	2.87	0.00	1.01 4.00
Well No. 3A	8.26	1.86	5.47	2.79	0.93	0.78 - 3.31
Well No. 4	10.89		7.85	3.14	0.00	0.70 - 0.21
Well No.4A	10.78	2.28	7.53	3.25	0.97	1.60 - 3.88
Well No. 7	7.59		4.29	3.30		1.00 9.00
Well No. 7A	7.33	2.60	4.22	3.11	0.51	2.38 3.40

NOTES:

Weather: 60 degrees F, Windy, Cloudy Readings by: Mark Panni Affiliation: SAIC Engineering , Inc., 101 Bast Grove Street, Middleboro, Massachuseits, 02346

FOOTNOTES:

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(1)	All readings and elevations are in feet and are referenced to mean sea level datom.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through September, 1996.

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TABLE IB

WATER LEVEL READINGS

ABROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1248 Date: Sept. 25, 1996 Time of Readings: 1228 - 1311

LOCATION	TOP OF CASING ELEVATION (I) (2)	BASELINE BLEVATION (3)	CURRENT READING	GURBENT Blevation	CHANGE IN BLEVATION v9. BASELINE	RANGE OF ELEVATION OVER PREVIOUS 146 MONTHS (4)
Tide Gauge	4.76		Dry		<u> </u>	
Well No. 2	6.92		5.15	1.77		
Well No. 2A	6.67	2.62	3.30	3.37	0.75	1.51 - 4.00
Well No. 3	6.95		5.40	1.55		2101 4.00
Well No. 3A	8.26	1.86	5.48	2.78	0.92	0.78 - 3.31
Well No. 4	10.99		10.28	0.71		0.00-0.01
Well No.4A	10.78	2.28	7.53	3.25	0.97	1.60 - 3.88
Well No. 7	, 7.59		6.93	0.66		1.00 0.000
Well No. 7A	7.33	2.60	4.20	3.13	0.53	2.38 · 3.40

NOTES:

Weather: 65 degrees F. Windy, Partly Sunny Readings by: Mark Pami Attillation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

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- (1) All readings and elevations are in feet and are referenced to mean sea level datum.
- The elevation is measured in reference to a known elevaton of 4.76 ft , at a point on sheet plling near Well No. 2. (2)
- (3) Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985. (4)
 - Numbers in this column are the range of recorded elevations from July 1984 through September, 1996.

TABLE 24

WATER LEVEL READINGS

AKROVOX PLANT SITE

NEW BEDFURD, MASSACHUSETTS

Tide Stage: High Time of Tide: 0735 Date: September 26, 1996 Time of Readings: 0714 - 0758

LOCATION	TOP OF CASING ELEVATION (1)(2)	BASELINE ELEVATION (3)	CURBENT BEADING	CURRENT ELEVATION	CHANGE IN RLEVATION vy. BASELINE	RANGE OF ELEVATION OVER PREVIOUS 146 MONTHS (4)
Tide Gauge	4.76		0.85	3.91		
Well No. 2	6.92		4.08	2.84	<u> </u>	
Well No. 24	6.67	2.62	3.44	3.23	0.61	1.51 - 4.00
Well No. 3	6.95		4.19	2.76		101 200
Well No. 3A	8.26	1.86	6.56	2.70	0.84	0.78 - 3.31
Well No. 4	10.99		7.89	3.10		0.10 0.01
Well No.4A	10.78	2.28	7.56	3.22	0.94	1.60 - 3.88
Well No. 7	, 7.59		4.25	3.34		A.00 0,000
Well No. 7A	7.33	2.60	4.23	3.10	0.50	2.38 - 3.40

NOTES:

Weather: 50 degrees F, Cloudy, Light Wind Readings by: Mark Panni Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

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All readings and elevations are in feet and are referenced to mean sea level datum.
 Tide elevation is measured in reference to a known elevator of 4.76 ft, at a point on sheet piling near Well No. 2.
 Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
 Numbers in this column are the range of recorded elevations from July 1984 through September, 1996.

TABLE 2B

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1339 Date: September 26, 1996 Time of Readings: 1233 - 1311

LOCATION	TOP OF CASING ELEVATION (1)(2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN ELEVATION V3. BASELINE	RANGE OF BLEVATION OVER PREVIOUS 146 MONTHS (4)
Tide Gauge	4.76		Dry			
Well No. 2	6.92		5.03	1.89		
Well No. 2A	6.67	2.62	3.40	3.27	0.65	1.51 - 4.00
Well No. 3	8.95		5.05	1.90	<u> </u>	
Well No. 3A	8.26	1.86	5.78	2.48	0.62	0.78 - 3.31
Well No. 4	10.99		10.26	0,73		VIV 0.01
Well No.4A	10.78	2.28	7.58	3.20	0.92	1.60 - 3.88
Well No. 7	; 7.59		7.11	0.48		1.00 . 0.00
Well No. 7A	7.33	2.60	4.22	3.11	0.51	2.38 - 3.40

NOTES:

Weather: 60 degrees F, Cloudy,Windy Readings by: Mark Panni Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

All readings and elevations are in fest and are referenced to mean sea level datum.
 Tide elevation is measured in reference to a known elevator of 4.76 ft, at a point on sheet piling near Well No. 2.
 Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
 Numbers in this column are the range of recorded elevations from July 1984 through September, 1996.

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TABLE 3A

WATER LEVEL READINGS

ABROVOX PLANT SITE

NEW BEDFORD, MASSACHUSETTS

Tide Stage: High Time of Tide: 0823 Bate: September 27, 1996 Time of Readings: 0813 - 0846

LOCATION	TOP DF CASING ELEVATION (1)(2)	BASELINE ELEVATION (3)	CURRENT READING	GURRENT Elevation	CHANGE IN BLEVATION vs. BASELINE	BANGE OF ELEVATION OVER PREVIOUS 146 MONTHS (4)
l'ide Gaoge	4.76		0.95	3.81	<u> </u>	
Well No. 2	6.82		4.11	2.81	<u>├──</u>	
Well No. 2A	6.67	2.62	3.45	3.22	0.60	1.51 - 4.00
Well No. 3	6.95		4.28	2.67	0.00	1.01-400
Nell No. 3A	8.26	1.85	5.80	2.46	0.60	0.78 • 3.31
Yell No. 4	10.99		8.00	2.99		0.70-0.01
Well No.4A	10.78	2.28	7.64	3.14	0.86	1.60 - 3.88
Hell No. 7	7.59		4.33	3.26	0.00	1.00.00
Well No. 7A	7.33	2.60	4.25	3.08	0.48	2.38 - 3.40

NOTES:

Weather: 55 degrees F, Cloudy,Windy Readings by: Mark Panni Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

- [1] All readings and elevations are in feet and are referenced to mean sea level datum. (2) (3) Tide elevation is measured in reference to a known elevaton of 4.76 ft , at a point on sheet piling near Well No. 2. Baseline elevations shown for shallow wells Nos. 24, 34, 44, and 7A are average monthly readings recorded for July 1984 through June 1985.
- (4) Numbers in this column are the range of recorded elevations from July 1984 through September, 1996.

TABLE 3B

WATER LEVEL READINGS

AEROVOX PLANT SITE

NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide:1426 Date: September 27, 1996

Time of Readings: 1413 - 1445

LOCATION	TOP OF Casing Blevation (1)(2)	BASELINE BLEVATION (3)	CURBENT READING	CURRENT RLEVATION	CHANGE IN RLEVATION VS. BASELINE	EANGE OF ELEVATION OVER PREVIOUS 146 MONTHE (4)
Tide Gauge	4.76		Dry			
Well No. 2	6.92		5.07	1.85		
Well No. 2A	6.67	2.62	3.41	3.26	0.64	1.51 - 4.00
Well No. 3	6,95		5.60	1.35		
Well No. 3A	8.26	1.86	5.47	2.79	0.93	0.78 - 3.31
Well No. 4	10.99		10.30	0.69		
Well No.4A	10,78	2.28	7.62	3.16	0.88	1.60 - 3.88
Well No. 7	7.59		7.22	0.37		
Well No. 7A	7.33	2.60	4.25	3.08	0.48	2.38 - 3.40

NOTES:

Weather: 70 degrees F. Parithy Sunny, Windy Beadings by: Mark Panni Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middlebore, Massachusetts, 02346

- (i) All readings and elevations are in feet and are referenced to mean sea level datum.
- (2) Tide elevation is measured in reference to a known elevaton of 4.76 R , at a point on sheet plling near Well No. 2.
- (3) Baseline elevations shown for shallow wells Nos. 24, 34, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- (4) Numbers in this column are the range of recorded elevations from July 1984 through September, 1996.

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April 16, 1996

2827.960311.013 01-0827-05-0051-003

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U.S. Environmental Protection Agency Region 1 John F. Kennedy Building Boston, Massachusetts 02203

ATTENTION: Mr. Frank Ciavattieri

REFERENCE: Aerovox Site Post-Closure Monitoring, March 4,5,6, 1996

Dear Mr. Ciavattieri:

Enclosed are the results of the water level monitoring and cap inspection conducted at the Aerovox site by SAIC Engineering, Inc. during the March 1996 full moon period. We are also enclosing corrected copies of the water level readings taken on March 17, 1995, Tables 3A and 3B. The low and high tide readings were switched in some of the entries in these tables.

The next inspection and round of water level readings are scheduled for the September 1996 full moon period. Please call if you have any questions.

Sincerely,

SAIC ENGINEERING, INC.

Allen F. Davis, P.E. Project Manager

Enclosures

cc: G. Monte, DEP/SERO P. Galvani, Ropes & Gray P. Szwaja, Aerovox WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: High Time of Tide: 0705 Date: March 4, 1996 Time of Readings: 0643 - 0742

LOCATION	TOP OF CASING BLBYATION (1) (2)	BASELINB ELEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGR IN BLEVATION VS. BASELINE /	RANGE OF BLEVATION OVBR PREVIOUS 140 MONTES (4)
Tide Gauge	4.76		Dry (5)			
Well No. 2	6.92		5.40	1.52		
Well No. 2A	6.67	2.62	3.40	3.27	0.65	1.51 - 4.00
Well No. 3	6.95		5.40	1.55		
Weil No. 3A	8.26	1.86	6.29	1.97	0.11	0.78 - 3.31
Well No. 4	10.99		9.47	1.52		
Well No.4A	10.78	2.28	7.84	2.94	0.66	1.60 - 3.88
Well No. 7	7.59		6.02	1.57		······································
Well No. 7A	7.33	2.60	4.56	2.77	0.17	2.38 - 3.40

NOTES:

Weather: 20 degrees F, Windy, Overcast. Readings by: David J. Minese Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

- All readings and elevations are in feet and are referenced to mean sea level datum. (1)
- Tide elevation is measured in reference to a known elevaton of 4.76 ft., at a point on sheet piling near Well No. 2.
- (2) (3) (4) Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March, 1996.
- Tide guage reading reported as "dry" is considered anomalous. (5)

WATER LEVEL READINGS

ABROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1240 Date: March 4, 1996 Time of Readings: 1220 - 1311

LOCATION	TOP OP Casing Elevation (1) (2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT BLEVATION	CHANGE IN BLBVATION 75. BASELINE /	RANGE OF ELEVATION OVBR PREVIOUS 140 MONTHS (4)
Tide Gauge	4.76		Dry			
Well No. 2	6.92	{	5.45	1.47		
Well No. 2A	6.67	2.62	3.39	3.28	0.66	1.51 - 4.09
Well No. 3	6.95		5.45	1.50		
Well No. 3A	8.26	68.1	6.30	1.96	0.10	0.78 - 3.31
Well No. 4	10.99		10.88	0.11		
Well No.4A	10.78	2.28	7.84	2.94	0.66	1.60 3.88
Well No. 7	7.59		7.57	0.02	·····	
Well No. 7A	7.33	2.60	4.55	2.78	0.18	2.38 - 3.40

NOTES:

Weather: 20 degrees F. Windy, Overcasi. Readings by: David J. Minese Affiliation: SAIC Engineering , Inc., 101 Bast Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES.

- (1) (2) All readings and elevations are in feet and are referenced to mean sea level datum.
- The elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
- (3) (4) Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March, 1996.

TABLE 2A

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEOFORD, MASSACHUSETTS

١. Tide Stage: High Time of Tide: 0745 Date: March 5, 1996 Time of Readings: 0725 - 0812

LOCATION	TOP OF Casing Blevation (1) (2)	BASELINE ELEVATION (3)	CURRENT READING	CURBENT ELEVATION	CHANGE IN ELEVATION V3. BASELINE /	RANGE OF BLEVATION OVER FREVIOUS 140 MONTES (4)
Ide Gauge	4.78		3.34	1.42		
Vell No. 2	8.92		5.22	1.70		
Vell Ho. 2A	6.57	2,62	3A7	3,20	0.58	1.51 - 4.00
Vell No. 3	6.95		5.25	1.70	1	
Vell No. 3A	8.26	1,86	6.28	1.98	0.12	0.78 - 3.31
Vell No. 4	10.99		9.18	1.81		
Vell No.4A	10.78	2.28	7.87	2.91	0.63	1.60 - 3.88
fell No. 7	7.59		5.70	1.89		
Vell No. 7A	7.33	2.60	4.56	2.77	0.17	2.38 - 3.40

NOTES:

Weather: 25 to 30 degrees F, Snow, Overcast Readings by: David J. Minere Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

- All readings and elevations are in feet and are referenced to mean sea level datum. (1)
- Tide elevation is measured in references to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
- ;2) (3) (4) Beseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March, 1996.

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1310 Date: March 5, 1996 Time of Readings: 1250 - 1335

LOCATION	TOP OF CASING ELEVATION (1) (2)	BASELINE BLEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN BLEVATION VS. BASELINE	RANGE OF BLEVATION OVER PREVIOUS 140 MONTHS (4)
Tide Gauge	4.76		Dry	-		t/
Well No. 2	6.92		5,45	1.47		
Well No. 2A	6.67	2.62	3,39	3.28	0.66	1.51 4.00
Well No. 3	6.95		5.50	1.45	1	
Well No. 3A	8,26	1.86	6,24	2.02	0.16	0.78 - 3.31
Well No. 4	10.99		10.45	0.54	1	
Well No.4A	10.78	2.28	7.85	2.93	0.65	1.60 · 3.88
Well No. 7	7,59	ł	7.03	0.59	1	
Well No. 7A	7.33	2.60	4.54	2.79	0.19	2.38 - 3.40

NOTES:

Weather: 25 to 30 degrees F, Snow, Overcast Readings by: David J. Minese Affiliation: SAIC Engineering , Inc. 101 East Grove Street, Middleboro, Massachusetts, 02346

- (1) All readings and elevations are in feet and are referenced to mean sea level datum.
- (Z) (3) (4) Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet pilling near Well No. 2.
- Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March, 1996.

TABLE 3A

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: High Time of Tide: 0825 Date: March 6, 1996 Time of Readings: 0805 · 0840

LOCATION	TOP OF Casing Elevation (1) (2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN ELEVATION 75. BASELINE /	RANGE OF BLBVATION OVER PRBVIOUS 140 MONTHS (4)
Tide Gauge	4.76		2.65	2.11		
Well No. 2	6.92		4.84	2.08		
Well No. 2A	6.67	2.62	2.96	3.71	1.09	1.51 - 4.00
Well No. 3	6,95	1	4.93	2.02		
Well No. 3A	8.26	1.85	6.17	2.09	0.23	0.78 - 3.31
Well No. 4	10.99	<u></u>	8.75	2.24		
Well No.4A	10.78	2.28	6.97	3.81	1.53	1.60 - 3.88
Well No. 7	7.59		5.26	2.33		
Well No. 7A	7.33	2.60	4.48	2,85	0.25	2.38 3,40

NOTES:

Weather: 35 to 40 degrees F, Heavy Rain. Readings by: David J. Minese Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

- All readings and elevations are in feet and are referenced to mean sea level datum. (1) (2) (3) (4) The elevation is measured in reference to a known elevaton of 4.76 ft , at a point on sheet plling near Well No. 2. Baseline elevations shown for shallow wells Nos. 24, 34, 44, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March, 1996.

WATER LEVEL READINGS

AEROVOX PLANT SITE **NEW BEDFORD, MASSACHUSETTS**

Tide Stage: Low Time of Tide:1343 Date: March 6, 1896 Time of Readings: 1323 - 1400

LOCATION	TOP OF CASING BLEVATION (1) (2)	RASELINE BLEVATION (3)	CURRENT READING	CURRENT BLEVATION	CHANGE IN ELBVATION VS. BASELINE	BANGE OF BLEVATION OVER PREVIOUS 140 MONTHS (4)
Tide Gauge	4.76		Dry	M		
Well No. 2	6.92		5.45	1.47		
Well No. 2A	6.67	2.62	2.92	3.75	1.13	1.51 - 4.00
Well No. 3	6.95		5.59	1,36		
Well No. 3A	8.26	1.86	6.15	2.11	0.25	0.78 - 3.31
Well No. 4	10.89		10.52	0.47		
Well No.4A	10.78	2.28	6.91	3.87	1.59	1.60 - 3.88
Well No. 7	7.59		7.25	0.34		
Well No. 7A	7.33	2.60	4.46	2.87	0.27	2.38 - 3.40

NOTES:

Weather: 35 to 40 degrees P, Heavy Rain. Readings by: David J. Minese Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

- (1) (2) All readings and elevations are in feet and are referenced to mean sea level datum.
- Tide elevation is measured in reference to a known elevaton of 4.76 ft., at a point on sheet piling near Well No. 2.
- (3) (4) Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March, 1996.



SAIC Engineering, Inc. A Subsidiary of Science Applications International Corporation

An Employee-Owned Company

September 18, 1995

2827.950913.009 01-0827-05-0051-003

U.S. Environmental Protection Agency Region 1 John F. Kennedy Building Boston, Massachusetts 02203

RECEIVED

ATTENTION:- Mr. Frank Ciavattieri

REFERENCE:

Aerovox Site Post-Closure Monitoring, September 7,8,9, 1995

Dear Mr. Ciavattieri:

Enclosed are the results of the water level monitoring and cap inspection conducted at the Aerovox site by SAIC Engineering, Inc. during the September 1995 full moon period. The next inspection and round of water level readings are scheduled for March 1996 full moon period. Please call if you have any questions.

Sincerely,

SAIC ENGINEERING, INC.

Allen F. Davis, P.E. Project Manager

Enclosures

- cc: G. Monte, DEP/SERO
 - P. Galvani, Ropes & Gray

P. Szwaja, Aerovox

TABLE 1A

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACRUSETTS

Tide Stage: High

Time of Tide: 0647 Date: September 7, 1935 Time of Readings: 9530 - 0702

LOCATION	TOP OF CASINQ ELEVATION (1) (2)	BASELINE BLEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN ELEVATION V8. BASELINE	RANGE OF BLEVATION OVER PREVIOUS 134 MONTHS (4)
Fide Gauge	4.76		1.37	3.39		
17ell No. 2	6.92		4.30	2.62		
Well No. 2A	6.67	2.62	4.40	2.27	-0.35	1.51 - 4.00
Yell No. 3	6.95		5.05	1.90		
Well No. 3A	8.26	1.86	6.06	2.20	0.34	0.78 - 3.31
7e)1 No. 4	10.99		8.74	2.25		
Yell No.4A	10.78	2.28	8.59	2.19	-0.09	1.60 - 3.88
Fell No. 7	7.59	1	5.10	2.49		
Vell No. 7A	7.33	2.60	4,69	2,64	D.04	2.38 . 3.40

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NOTES:

Weather: 70 degrees F, Partly Oldy.

Readings by: Cortland Ridings Alfiliation: SAIC Engineering, inc. 101 East Grove Street, Midddleboro, Massachusetts, 02346

FOOTNOTES:

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(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet pilling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through September, 1995.

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TABLE 1B

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1258 Date: September 7, 1995 Time of Readings: 1240 - 1328

LOCATION	top of Casing Elevation (1)(2)	BASELINE BLEVATION (3)	current Readin g	CURRENT BLEVATION	CHANGE IN BLEVATION 93. BASELINB	RANGE OF BLEVATION OVER PREVIOUS 134 MONTHS (4)
Tide Gauge	4.76	1	Dry		•	
Well No. 2	6.92		5.65	1.27		
Well No. 2A	6.67	2.62	4.37	2.30	-0.32	1.51 - 4.00
Well No. 3	6.95		8.05	0.90		
Well No. 3A	8.26	1.86	6.40	1.86	-0.00	0.78 - 3.31
Well No. 4	10.99		10.85	0.14		
Well No.4A	10.78	2.28	8.59	2.19	•0.09	1.60 - 3.88
WEU NO. 7	7.59	1	7.50	0.09		
Well No. 7A	7.33	2.60	4.65	2.68	0.08	2.38 • 3.40

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f.

Weather: 70 degrees F, Partly Cldy. Readings by: Cortland Ridings Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Midddleboro, Massachusetts, 02346

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FOOTNOTES:

(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 24, 34, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
	Numbers in this column are the range of recorded elevations from July 1984 through September, 1995.

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TABLE 24

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: High Time of Tide: 0736 Date: September 8, 1995 Time of Readings: 0715 - 0759

LO CATION	TOP OF CASING ELBVATION (1) (2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT BLEVATION	change in Blevation VS. Baseline	RANGE OF ELEVATION OVER PREVIOUS 134 MONTHS (4)
Tide Gauge	4.76		1.44	-		
Well No. 2	6.92		3,83	3.09		
Well No. 2A	6.67	2.62	4.12	2.55	-0.07	1.51 4.00
Well No. 3	6.95	· · · · · · · · · · · · · · · · · · ·	5.10	1.85	· · · · · · · · · · · · · · · · · · ·	
Well No. 3A	8.26	1.86	5.77	2.49	0.63	0.78 - 3.31
Well Ko. 4	10.99	1	8.69	2.30		
Well No.4A	10.78	2.28	8.61	2.17	-0.11	1.60 - 3.88
Viell No. 7	, 7.59		5.02	2.57		
Well No. 7A	7,38	2.60	4.67	2.66	0.06	2.38 3.40

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Weather: 70 degrees F, Cldy. Readings by: Cortland Ridings Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

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FOOTNOTES:

(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet pliing near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 24, 34, 44, and 7A are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through September, 1995.

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TABLE 2B

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WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

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Tide Stage: Low Time of Tide: 1346 Date: September 8, 1995 Time of Readings: 1325 - 1359

LOCATION	TOP OF CASING ELEVATION (1) (2)	BASELINB BLEVATION (3)	CURRENT READINO	CURRENT Elevation	Change in Elevation VS. Baseline	RANGE OF BLEVATION OVER PREVIOUS 134 MONTHS (4)
lde Gauge	4.76		Dry	-		
Vell No. 2	6.92		5.95	0.97		
Yell No. 2A	6.67	2.62	4.23	2.44	-0.18	1.51 - 4.00
lell No. J	6.95		6.36	0.59		
Tell No. 3A	8.28	1.86	6.40	1.86	-0.00	0.78 - 3.31
7ell No. 4	10.99		11.14	-0.15		
7011 No.4A	10.78	2.28	8.61	2.17	-0.11	1.60 - 3.88
ell No. 7	7.59		7.76	-0.17		
Pell No. 7A	7,83	2,60	4.73	2,60	-0,00	2.38 · 3.40

NOTES:

Weather: 70 degrees F, Cldy. Readings by: Cortland Ridings Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Midddleboro, Massachusetts, 02346

(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	Tige elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 24, 34, 44, and 74 are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through September, 1995.
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WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: High Time of Tide: 0823 Date: September 9, 1995 Time of Readings: 0805 - 0844

LOCATION	TOP OF CASING ELEVATION (1)(Z)	BASELINE Elevation (3)	CURRENT READING	CURRENT Elevation	CHARGE IN ELEVATION VS. DASELINE	RANGE OF BLEVATION OVER PREVIOUS 134 MONTHS (4)
Tide Gauge	4.76		1.23	3.53	11	
Well No. 2	6.92		4.75	2.17		
Well No. 2A	6.67	2.62	4,23	2.44	-0.18	1.51 - 4.00
Well No. 3	8.95	•	5.10	1.85		
Well No. 3A	8.26	1.86	6.10	2.15	0.30	0.78 - 3.31
Well No. 4	10.99		8.62	2.37		
Well No.4A	10.78	2.28	8.61	2.17	-0.11	1.60 - 3,88
Well No. 7	7.59		4.95	2,64		
Well No. 7A	7.33	2,60	4.67	2.66	30,0	2.38 • 3.40

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NOTES:

Weather: 65 degrees F, Cldy.

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Readings by: Cortland Ridings Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Midddleboro, Massachusetts, 62346

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FOOTNOTES:

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TABLE 3B

ASROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1428 Date: September 9, 1995

Time of Readings: 1410 - 1432

LOCATION	TOP OF Casing Blevation	BASELINE ELEVATION	CURRENT READING	CURRENT ELEVATION	CHANGE IN BLEVATION V3. BASELINE	RANGE OF ELEVATION OVER
	(1)(2)	(3)	•			PREVIOUS 134 Months (4)
Tide Gauge	4.76		Dry	-	[·	
Well No. 2	6,92		5.15	1,17		
Well No. 2A	6.67	2.62	4.21	2,46	-0.16	1.51 - 4,00
Well No. 3	6.95		6.10	0,85		
Well No. 3A	8.26	1.86	6.31	1.95	0.09	0.78 - 3.31
Well No. 4	10.99		10.87	0.12		
Well No.4A	. 10.78	2.28	8.62	2.16	-0.12	1.60 - 3.88
Rel) No. 7	7.59		7.57	0.02		
Vell No. 7A	7,33	2.60	4.79	2.54	-0.05	2.38 - 3.40

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NOTES:

Weather: 70 degrees F, Sun/Cldy

Readings by: Cortland Ridings

Affiliation: SAIC Engineering . Inc., 101 East Grove Street, Midddleboro, Massachusetts, 02346

FOOTNOTES:

(1)	All readings and elevations are in fest and are referenced to mean sea level dainm.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft , at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are avorage monthly readings recorded for Joly 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through September, 1995.

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SAIC Engineering, Inc. A Subsidiary of Science Applications International Corporation

An Employee-Owned Company

March 24, 1995

2827.950323.001 01-0827-05-0051-003

U.S. Environmental Protection Agency Region 1 John F. Kennedy Building Boston, Massachusetts 02203

ATTENTION: Mr. Frank Ciavattieri

REFERENCE: Acrovox Site Post-Closure Monitoring, March 15, 16, 17, 1995

Dear Mr. Ciavattieri:

Enclosed are the results of the water level monitoring and cap inspection conducted at the Aerovox site by SAIC Engineering, Inc. during the March 1995 full moon period. The next inspection and round₁ of water level readings are scheduled for the September 1995 full moon period. Please call if you have any questions.

Sincerely,

SAIC ENGINEERING, INC.

Allen F. Davis, P.E. Project Manager

Enclosures cc: G. Monte, DEP/SERO P. Galvani, Ropes & Gray P. Szwaja, Aerovox

101 East Grove Street, Middleboro, Massachusetts 02346 + (508) 946-3500 + FAX: (508) 946-3509

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: High Time of Tide: 0625 Date: Mar. 15, 1995 Time of Readings: 0605 - 0635

LOCATION	TOP OF CASING ELEVATION (1) (2)	BASELINE ELEVATION (3)	CUBRENT READING	CURRENT ELEVATION	CHANSE IN BLEVATION VS. BASELINE	BANGE OF ELEVATION OVER PREVIOUS 128 MONTES (4)
Tide Gauge	4.76		2.70	2.06		<u></u>
Well No. 2	6.92		5.10	1,82		· · · · · · · · · · · · · · · · · · ·
Well No. 2A	6.67	2.62	3.72	2.95	0.33	1.51 - 4.00
Well No. 3	6.95		5.18	1.77		
Well No. 3A	8.26	1.85	5.55	2.71	0.85	0.78 - 3.31
Well No. 4	10.99		9,10	1.89		
Well No.4A	10.78	2.28	7.90	2.88	0.60	1.60 - 3.88
Vell No. 7	, 7.59	<u>.</u>	5.56	2.03		
Well No. 7A	7.33	2.60	4.66	2.57	0.07	2.38 - 3.40

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NOTES:

Weather: 50 degrees F, Cidy. Readings by: Cortland Ridings Affiliation: SAIC Engineering, Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 476 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through March, 1995.

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ABROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1157 Date: Mar. 15, 1995 Time of Readings: 1140 - 1206

LOCATION	TOP OF CASING ELEVATION (1) (2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT BLEVATION	CHANGE IN ELEVATION V5. BASELINE	BANGE OF BLEVATION OVER PREVIOUS 128 MONTHS (4)
Tide Gauge	4.76		Dry	-		
Well No. 2	6.92		5.22	1.70		
Well No. 2A	6.67	2.62	3.74	2,93	0.31	1.51 • 4.00
Well No. 3	6.95		5,84	1.11		
Well No. 3A	8.26	1.86	6.41	1.85	-0.D1	0.78 - 3.31
Well No. 4	10,99		10.60	0.39		
Well No.4A	10.78	2.28	7.91	2.87	0.59	1.60 - 3.88
Well No. 7	/ 7.59		7.34	0.25		1.00 0.00
Well No. 7A	7.33	2.60	4.66	2.67	0.07	2.38 - 3.40

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NOTES:

Weather: 50 degrees F, Cidy. Readings by: Cortland Ridings Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Midddleboro, Massachusetts, 02346

FOOTNOTES:

(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	The elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 24, 34, 44, and 74 are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through March, 1995.

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TABLE 2A

ABROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Eigh Time of Tide: 0709 Date: Mar. 16, 1995 Time of Readings: 0650 - 0717

LOCATION	TOP OF CASING BLEVATION (1)(2)	BASELINE ELEVATION (3)	CURRENT RBADING	CURRENT ELEVATION	CHANGE IN ELEVATION VS. BASELINE	RANGE OF ELEVATION OVER PREVIOUS 128 MONTHS (4)
Tide Gauge	4.76		2.00	2.76		
Well No. 2	6.92		4.92	2.00	· · · ·	
Well No. 2A	6.67	2.62	3.74	2.93	0.31	1.51 - 4.00
Well No. 3	6.95		5.00	1.95		
Well No. 3A	8.26	1.86	6,33	1.93	0.07	0.78 - 3.31
Well No. 4	10,99		8.86	2.13		
Well No.4A	10.78	2.28	7,90	2.88	0.60	1.60 - 3.88
Well No. 7	7.59		5.27	2.32		
Well No. 7A	7.33	2.60	4.66	2.67	0.07	2.38 - 3.40

NOTES:

Weather: 50 degrees F, Cldy. Readings by: Cortland Ridings Asililation: SAIC Engineering, Inc., 101 East Grove Street, Midddleboro, Massachusetts, 02346

FOOTNOTES:

(1)	All readings and elevations are in feet and are referenced to mean sea level datum.
(2)	Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
(3)	Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
(4)	Numbers in this column are the range of recorded elevations from July 1984 through March, 1995.

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AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1238 Date: Mar. 16, 1995 Time of Readings: 1220 - 1248

LOCATION	TOP OF CASING BLEVATION (1)(2)	BASELINE ELEVATION (3)	CURRENT READING	CURRENT BLEVATION	CHANGE IN ELEVATION VS. BASELINE	RANGE OF ELEVATION OVER PREVIOUS 128 MONTHS (4)
Tide Gauge	4,76		Dry	-		······
Well No. 2	6,92		5.41	1.61		
Well No. 2A	6,67	2.62	3,76	2.91	0.29	1.51 - 4.00
Well No. 3	6.95		5.85	1.10		
Woll No. 3A	8.26	1.86	6.40	1.86	-0.00	0.78 - 3.31
Well No. 4	10.99		10.67	0,32		
Well No.4A	10.78	2.28	7.94	2.84	0.56	1.60 - 3.88
Well No. 7	7.59	······································	7.39	0.23		
Well No. 7A	/ 7.33	2.60	4.66	2.57	0.07	2.38 - 3.40

NOTES:

Weather: 50 degrees F, Cldy Readings by: Cortland Ridings Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Midddleboro, Massachusetts, 02346

FOOTNOTES:

- (1) All readings and elevations are in feet and are referenced to mean sea level datum.
- Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet pilling near Well No. 2.
- (2) (3) (4) Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985.
- Numbers in this column are the range of recorded elevations from July 1984 through March, 1995.

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Table 3A

ABROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: High Time of Tide: 0753 Date: Mar. 17, 1995 Time of Readings: 0740 · 0807

LOCATION top of BASELINE CORRENT CURRENT CHANGE IN RANGE OF CASING ELEVATION READING BLEVATION BLEVATION RLEVATION ELEVATION **VS. BASELINE** OVBR (3) PREVIOUS (1)(2) 128 MONTHS (4) 4.76 Tide Gauge 2.10 2.66 Well No. 2 6.9Z 477 2.15 Well No. 2A 6.67 2.62 3.47 3.20 0.58 1.51 4.00 Well No. 3 6,95 4.97 1.98 Well No. 3A 8.28 1.88 6.35 1.91 0,05 0.78 - 3.31 Well No. 4 10.99 8.92 2.07 Well No.4A 2.88 0.60 1.60 - 3.88 10.78 2.28 7.90 7.59 Well No. 7 6.35 2.24 Well No. 7A 7.33 2.60 4.66 0.07 $2.38 \cdot 3.40$ 2.67

NOTES:

Weather: 40 degrees F. Rain **Readings by: Cortiand Ridings** Alilliation: SAIC Engineering , Inc., 101 Bast Grove Street, Midddleboro, Massachusetts, 02345

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Corrected 3/26/95 (5)

FOOTNOTES:

- (1) All readings and elevations are in feet and are referenced to mean sea level datum.
- (2) Tide elevation is measured in reference to a known elevaton of 4.76 it, at a point on sheet piling near Well No. 2.
- Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings recorded for July 1984 through June 1985. (3)
- Numbers in this column are the range of recorded elevations from July 1984 through March, 1995.
- (4) (5) Current readings and associated current elevation data and change in elevation vs. baseline data reported earlier incorrectly, corrected 3/25/96

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TABLE 3A

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Eigh Time of Tide: 0753 Date: Mar. 17, 1995 Time of Readings: 0740 - 0807

LOCATION	TOP OF CASING ELEVATION (1) (2)	BASELINE BLEVATION (3)	CURRENT RBADING	CURRENT ELEVATION	CHANGE IN ELEVATION VS. BASELINE	RANGE OF ELEVATION OVER PREVIOUS 128 MONTES (4)
Tide Gauge	4.76	_	Dry	4.76		<u></u>
Well No. 2	6.92		5.35	1.57	·	
Well No. 2A	6,57	2.62	3.48	3.19	0.57	1.51 - 4.00
Well No. 3	6.95		5.45	1.50		
Well No. 3A	8.26	1.86	6,36	1.90	0.04	0.78 - 3.31
Well No. 4	10.99		10.64	0.35		0.0.0.0.01
Well No.4A	10.78	2.28	7.91	2.87	0.59	1.60 - 3.88
Well No. 7	7.59		7.37	0.22		100-0.00
Well No. 7A	7.33	2.60	4.65	2.68	0.08	2.38 - 3.40

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NOTES:

Weather: 40 degrees F, Rain Readings by: Cortland Bidings Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Middleboro, Massachusetts, 02346

FOOTNOTES:

(1) All readings and elevations are in feet and are referenced to mean sea level datum.	
(2) Tide elevation is measured in reference to a known elevation of 4.76 ft, at a point on sheet piling p	ear Well No. 2.
(3) Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly readings	recorded for July 1984 through June 1985.
(4) Numbers in this column are the range of recorded elevations from July 1984 through March, 1995.	· · · · · · · · · · · · · · · · · · ·

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TABLE 3B

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Corrected 3/26/95 (6)

Tide Stage: Low Time of Tide: 1320 Date: Mar. 17, 1985 Time of Readings: 1310 - 1337

LOCATION	TOP OF Casing Elbyation (1) (2)	BASELINE BLEVATION (3)	CURRENT READING	CURRENT ELEVATION	CHANGE IN ELEVATION VS. BASELINE	RANGE OF BLEVATION OVER PREVIOUS 128 MONTHS (4)
Tide Gauge	4.76		Dry	-		
Well No. 2	6.92		5.35	1.57		
Well No. 2A	6.67	2.62	3.48	3.19	0.57	1.51 - 4.00
Well No. 3	6,95		5.45	1.50		
Well No. 3A	8.26	1.86	6.36	1.90	0.04	0.78 - 3.31
Well No. 4	10.99		10.64	0.35		
Well No.AA	10,78	2.28	7.91	2.87	0.59	1.60 - 3.88
Well No. 7	7.59		7.37	0.22		
Well No. 7A	7.33	2.60	4.65	2.68	0.08	2.38 . 3.40

NOTES:

Weathor: 40 degrees F, Cldy with Rain Readings by: Cortland Ridings — Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Midddleboro, Massachusetts, 02346

FOOTNOTES:

- All readings and elevations are in fact and are referenced to mean sea level datum.
 Tide elevation is measured in reference to a known elevator of 4.76 ft, at a point on sheet pilling near Well No. 2.
 Baseline elevations shown for shallow wells Nos. 2A, 3A, 4A, and 7A are average monthly teadings recorded for July 1984 through June 1985.
 Numbers in this column are the range of recorded elevations from July 1984 through March, 1995.
- (5) Current readings and associated current elevation data and change in elevation vs. baseline data reported earlier incorrectly, corrected 3/25/96

TABLE 3B

WATER LEVEL READINGS

AEROVOX PLANT SITE NEW BEDFORD, MASSACHUSETTS

Tide Stage: Low Time of Tide: 1320 Date: Mar. 17, 1995 Time of Readings: [304 · 1327

LOCATION	TOP OF CASING ELBVATION (1)(2)	BASELINE BLEVATION (3)	current Reading	CURRENT BLEVATION	CHANGE IN BLEVATION VS. BASELINE	RANCE OF ELEVATION OVER PREVIOUS 128 MONTHS (4)
Tide Gauge	4.76		Dry			
Well No. 2	6.92		4.85	2.07		<u> </u>
Well No. 2A	6.67	2.62	3,51	3.16	0.54	1.51 - 4.00
Well No. 3	6.95		5.63	1.32		
Well No. 3A	8.26	1.86	6.27	1.99	0.13	0.78 - 3.31
Well No. 4	10.99		10.18	0.81		
Well No.4A	10.78	2.28	8.37	2.41	0.13	1.60 - 3.88
Well No. 7	. 7.59		6.89	0.70		
Well No. 7A	7.33	2.60	4.57	2.76	0,16	2.38 - 3.40

NOTES:

Weather: 40 degrees F, Cldy

Readings by: Cortland Ridings

Affiliation: SAIC Engineering , Inc., 101 East Grove Street, Midddleboro, Massachusetts, 02346

FOOTNOTES:

- All readings and elevations are in feet and are referenced to mean sea level datum.
- Tide elevation is measured in reference to a known elevaton of 4.76 ft, at a point on sheet piling near Well No. 2.
- (1) (2) (3) (4) Baseline elevations shown for shallow wells Nos. 24, 34, 44, and 7A are average monthly readings recorded for July 1984 through June 1985. Numbers in this column are the range of recorded elevations from July 1984 through March, 1995,

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Attachment 11

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Building Material Volume and Mass Calculations

Attachment 11

Aerovox, Inc. Facility New Bedford, Massachusetts

Building Material Volume and Mass Calculations

The calculations presented in Tables 11-1 through 11-8 were performed in order to estimate the mass and volume of materials which would be generated during the demolition activities of the Aerovox, Inc. (Aerovox) facility, located in New Bedford, Massachusetts. These calculations are approximate and are intended for the purpose of estimating the cost of remedial measures which can be applied to address the presence of polychlorinated biphenyls (PCBs) at the Aerovox facility. It should be noted that calculations are based on the average densities of select solids⁽¹⁾, and no voids (empty spaces) were assumed in the materials. Therefore, the actual volume of the materials to be generated during the demolition activities will increase from those presented in Tables 11-1 through 11-8. As such, a volume bulking factor of 1.5 has been applied to volumes presented in Tables 11-1 and 11-2 for wood material in order to better estimate transportation and disposal costs. A description and explanation of the terms used in Tables 11-1 through 11-8 is presented below.

Basic Units:

For ease of calculation and manipulation of volume/mass estimates, "basic units" were created. A "basic unit" is specified in the column labeled "Unit", and may be a linear foot (lin ft) of the structure, such as wall, steel beam, etc., a square foot (sq ft) of a structure, such as wall, floor, etc., or individual "unit" (each), such as window, wooden column, etc. Based on the average densities and known dimensions of the "basic unit", the volume (Volume per Unit) and mass (Mass per Unit) of the "basic unit" were calculated. In cases, where "basic unit" consisted of material with the same average density, but the size of the "basic unit" varies (for example 4" thick and 5" thick brick wall), the appropriate dimensions were listed in column labeled "Size".

Volume/Mass Calculations:

The facility was divided into Eastern Section and Western Section, and then each section was divided by floors (levels). This layout provides a mechanism to determine the volume/mass of the separate sections of the building, as needed.

In order to determine the volume/mass of the structure(s) (such as brick wall), the number of the "basic units" (sq ft) of which the structure(s) consist was determined, and then multiplied by the "Volume per Unit" and "Mass per Unit", respectively. The results of the mass and volume calculations created the basis for demolition/cleanup cost presented in Table 15, 16, and 17 of this document.

Assumptions:

- 1. ⁽¹⁾ Average densities of the select materials based on data presented in "Handbook of Chemistry and Physics", 76th Edition, 1996.
- 2. Each level's volume and mass do not include the ceiling (except for the roof of the building). The volume/mass of each ceiling is calculated as the floor of the next higher level.

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Table 11-1

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Aerovox, Inc. Facility

Building Material Volume and Mass Calculations

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Basic Units:							Western	Section:		
				'		1st Floor			2nd Floo	_
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [Ib]	Unit	No. of Units	Volume [cf]	Mass [lb]	No. of Units	Volume [cf]	Mass _[lb]
Base Concrete Wall:	1' thick	3	540	lin. ft.	1432	4296	773280		0	0
Concrete Floor:	6" thick	0.5	90	sq. ft,	96920	48460	8722800	15000	7500	1350000
Brick Walls:	12" thick 16" thick	1 1.333	112 150	sq. ft. sq. ft.	5064 13239	5064 17647.59	567168 1985850	3006 4704	3006 6270.432	336672 705600
						0	0		0	0
Wooden Walls/Floor	4" thick 5" thick	0.333 0.41 6	9 11.25	sq. ft. sq. ft.	5986	1993.338 0	53874 0	81650	0 33966.4	0 918562.5
						0	0		0	0
Drywall:	9' high	0.91	36.5	lin. ft.	1100	1001	40150		0	• 0
2"X4" stud every 2'	10' high	1.01	40.5	lin. ft.	180	181.8	7290	2500	2525	101250
<u></u>	12' high	1.22	48.7	<u>íin. ft.</u>	550	<u>671</u>			0	0
Wooden Columns	9' high	3.14	138	each	176	552.64	24288		0	0
8" diameter	10' high	3.5	154	each	25	87.5	3850		õ	ŏ
	12" high	4.18	184	each	108	451.44	19872		õ	ŏ
	16' high	5.6	246	each		0	0	84	470.4	20664
Steel Beams:	W21 x 62	0.127	62	lin. fl.	9320	1183.64	577840	4583	0 582.041	0 284146
						0	0		0	0
Steel Plate	0.5" thick	0.04	19.48	sq. ft.		0	0	3925	157	76459
						0	0		0	0
Windows:	1" plyw'd	5.83	221	each		0	0		0	0
6' X 11'	1/64" met	0.09	44.7	each		0	0 D	20	0 153.92	0
Total square feet/por	unds:	0.82	203.7	each		81589.95	1.3E+07	26		6908.2 3800262
Total cubic yards: Total Tons:		<u></u>				3022.092 6401.524				cu. yds.

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Roof - Western Section:

1875 cubic yards 658 Tons

Table 11-1 (cont.)

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Aerovox, Inc. Facility

Building Material Ve	olume and	Mass Cald	ulations	•				E	stern Sect	lon			
Basic Units:						1st Floor			2nd Floor	•		3rd Floor	-
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [ib]	Unit	No. of Units	Volume [cf]	Mass [lb]	No. of Units	Volume [cf]	Mass [ib]	No, of Units	Volume [cf]	Mass [Ib]
Base Wall:	1' thick	3	540	lin. A.	1425	4275	769500	0	0	0	0	0	
Concrete Floor:	6" thick	0.5	90	<u>sq. ft.</u>	85214	42607	7669260	0	<u>0 </u>	0	<u> </u>	0	-
						0	0		0	0		0	
Brick Walls:	12" thick	1	112	sq. fl.	2246	2246	251552	2325	2325	260400	3525	3525	394800
	16" thick	1.333	150	<u>sq. li.</u>	4194	5590.602 0	629100 0	7650	0	<u>1147500</u>	0110	<u>10818.63</u> 0	
Wooden Walls/Floor:	4" thick	0.333	9	5q. fl.	3564	1186.812	-	0	Ď	0		0	-
AAOOOBU AASIIS/LIOOL	5" thick	0.333	11.25	२५. ॥. \$q. १ .	0	0	0	86182	35851.71	-	86182	35851.71	-
	U DUCK	0.410	11.20	<u>oq</u>	v		0		0	0		0000111	(
Drywali:	9' high	0.91	36.5	lin. ft.	0	ŏ	ŏ	0	ŏ	õ		ŏ	
2"X4" slud every 2 ft		1.01	40.5	lin. fl.	ō	õ	ō	ō	Ō	ŏ		Ō	
	12' high	1.22	48.7	lin. N.	ā	ō	õ	Ō	Ō	Ō		Ő	Ċ
						0	0		0	0	· · · ·	0	(
Particle Board Wall:	10° high	1.01	36.4	lin. ft.				2365	2388.65	86086	2320	2343.2	84448
0.5" thick board	12' high	1.22	43.68	lin. ft.	3100	3782	135408	0	0	G		0	0
2"X4" stud every 2'	16' high	1.62	58.24	lin. fl.		0	0	0	0	0		0	
									0	0		0	
Wooden Columns	9' high	3.14	138	each	0	0	0	0	0	0		0	· (
8" diameter	10' high	3.5	154	each	0	.0	0	0	0	0		0	(
	12' high	4.18	184	each	220	919.6	40480	0	0	0		0	(
	16' high	5.6	246	each		0	0	220	1232	54120		0	(
	<u>17' high</u>	5.95	261.8	each							220		57590
Steel Beams:	14/24 02	0 407	62	lin. ft.	7535	956.945	467170	7535	956.945	467170	7535	0 956.945	467170
Steel Deams:	W21 x 62	0.127	02	<u> </u>	7535	300.340	40/1/0	1000	500.540	40/1/0	1000	330.343	40/1/0
Steel Plate:	0.5" thick	0.04	19.48	sq. ft.		0	0	4728	189.12	92101.44		0	0
									Ó	0		0	0
Windows:	1" plyw'd	8.91	338	each		0	0		0	0		0	C
6' X 13'	1/64" met	0.14	68	each		0	0		0	0		0	0
		9.05	406	each	56	506.8	22736	119	1076.95	48314	119		
Tolai square feet/pou	inds:					62070.76	1E+07		54217.83	3125239		55881,44	3239276
Total cubic yards:						2299.101			2008.228				cu. yds.
Total Tons:					<u>.</u>	5008.641	1005		1562.619	1005		1619.638	1005
Roof - Eastern Sect	llon:	1474	cubic yard	a -									
		517	Tons										

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14771.81 cubic yards 17667.55 Tons TOTAL BUILDING MATERIAL VOLUME: TOTAL BUILDING MATERIAL MASS:

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Table 11-2

Aerovox, Inc. Facility

Materials to TSCA Landfill Under Option #1 (Excluding Concrete Floor at Grade)

							Western	Section:	-	
Basic Units:				1		1st Floor			2nd Floor	-
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [Ib]	Unit	No. of Units	Volume [cf]	Mass [lb]	No. of Units	Volume [cf]	Mass [ib]
Base Concrete Wall:	1' thick	3	540	lin. ft.		0	0		0	0
Concrete Floor:	6" thick	0.5	90	sq. ft.	<u> </u>	0	0	15000	7500	1350000
Brick Walls:	12" thick 16" thick	1 1.333	112 150	sq. ft. sq. ft.		0	0 0		0 0	0 0
· · · · · · · · · · · · · · · · · · ·	<u></u>					0	0		0	0
Wooden Walls/Floor:	4" thick 5" thick	0.333 <u>0.41</u> 6	9 11.25	sq. fl. sq. fl.	3186	1060.938 0	28674 0	56650	0 23566.4	0 637312.5
Drywali:	9' high	0.91	36.5	lin. ft.		0 0	0 0		0	0 0
2"X4" stud every 2'	10' high	1.01	40.5	lin. ft.		0	0	•	0	0
	12' high	1.22	48.7	lin. ft.		0	0		0	0
Wooden Columns 8" diameter	9' high 10' biab	3.14 3.5	138 154	each each		0	0 0		0 0	0 0
	10' high 12" high	4.18	184	each		Ő	0		0	0
	16' high	5.6	246	each		0	0		0	0
Steel Beams:	W21 x 62	0.127	62	lin. ft.		0	0		0	0
Steel Plate	0.5" thick	0.04	19.48	sq. ft.		0	0		0	U
Windows:	1" plyw'd	5.83	221	each		0	0 0		0	0
6' X 11'	1/64" met	0.09	44.7	each		Ō	0		Ō	Ō
Total square feet/pou	inds:	5.92	265.7	each		0 1060.938	0 28674		0 31066.4	0 1987313
Total cubic yards: Total Tons:						39.29714 14.337	cu. yds. Tons		1150.699 993.6563	cu. yds. Tons

Table 11-2 (cont.)

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Aerovox, Inc. Facility

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Materials to ISCA L	angan On	der Option	I #1 (EXCIU	ding Concrete Floor at	t Grade)			Ea	stem Sect	ion			
Basic Units:					-	1st Floor			2nd Floor	-		3rd Floor	
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [lb]	Unit	No. of Units	Volume [cf]	Mass [lb]	No. of Units	Volume [cf]	Mass [lb]	No. of Units	Volume [cf]	Mass [lb]
Base Wall:	1' thick	3	540	lin. ft.		0	0		0	0		0	0
Brick Walls:	12" thick	1	112	sq. ft.		0	0 0		0	0 0		0 0	- (
······································	16" thick	1.333	150	sq. ft.		0	0		0	0		<u> </u>	(
Wooden Walls/Floor:	4" thick 5" thick	0.333 <u>0.41</u> 6	9 11.25	sq. ft. sq. ft.	3564	1186.812 0	32076 0	86182	ŏ	0 969547.5	86182	0 35851.71	(969547.5
Drywall:	9' high	0.91	36.5	lin, ft.		0	0		0	0		0	(
2"X4" stud every 2 ft	10' high 12' high	1.01 1.22	40.5 48.7	lin. ft. lin. ft.		0	0		0	0		0	((
Particle Board Wall:	10' high	1. 01	36.4	lin. ft.		0	0		0	0		0	
0.5" thick board 2"X4" stud every 2'	12' high 16' high	1.22 1.62	43.68 58.24	lin. ft		0	0 0		0	0	<u> </u>	0	(
Wooden Columns	9' high	3.14	138	each		0	0		0	0		0	l
8" diameter	10' high 12' high	3.5 4.18	154 184	each each		0	0		0	0		0	l
.	16' high 17' high	5.6 5.95	246 261.8	each each		0	0		0	0		0	i
Steel Beams:	W21 x 62	0.127	62	lin. ft.		0	0		0	0		0	(
Steel Plate:	0.5" thick	0.04	19.48	sq. ft		0	0	<u></u>	0	0		0	
Windows:	1" plyw'd	8.91	338	each		0	0		0	0		0	. (
B' X 13'	<u>1/64" met</u>	0.14 9.05	<u>68</u> 406	each each		0	0		0 0	0 0		0	. (
Total square feel/pou	inds:					1186.812	32076	· · · · · · · · · · · · · · · · · ·	35851.71	969547.5		35851.71	969547.3
Total cubic yards: Total Tons:				_,		43.95952 16.038			1327.947 484.7738			1327.947 484.7738	
TOTAL TSCA MATE TOTAL TSCA MATE				3889.851 cubic yards 1993.679 Tons	8								

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Table 11-3

Aerovox, Inc. Facility

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Materials to TSCA Landfill Under Option #2 (Including a Portion of the Concrete Floor at Grade)

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• • • • •							Western	Section:	-	
Basic Units:						1st Floor			2nd Floor	-
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [Ib]	Unit	No. of Units	Volume [cf]	Mass [ib]	No. of Units	Volume [cf]	Mass [lb]
Base Concrete Wall:	1' thick	3	540	lin. ft.		0	0		0	0
Concrete Floor:	6" thick	0.5	90	<u>sq. fl.</u>	96920	48460	8722800	15000	7500	1350000
Brick Walls:	12" thick 16" thick	1 1.333	112 150	sq. ft. sq. ft.		0 0	0 0		0 0	0 0
Wooden Walls/Floor:	-	0.333	9	sq. ft.	3186	0 1060.938	0 28674		0 0	0
	5" thick	0.416	11.25	<u>sq. ft.</u>	, <u> </u>	0	0	56650	<u>23566.4</u> 0	<u>637312.5</u> 0
Drywall: 2"X4" stud every 2'	9' high 10' high	0.91 1.01	36.5 40.5	lin. ft. lin. ft.		0	0		0	0
T VA SING AACIA T	12' high	1.22	48.7	lin. ft.		Ő	0		0	Ő
					· · · · · · · · · · · · · · · · · · ·	0	0		0	0
Wooden Columns 8" diameter	9' high 10' high	3.14 3.5	138 154	each each		0 0	0 0		0 0	0 0
-	12" high 16' high	4.18 5.6	184 246	each each		0	0		0 0.	0
Steel Beams:	W21 x 62	0.127	62	lin. ft.		0	0	<u> </u>	 0	0
····						0	0		0	0
Steel Plate	0.5" thick	0.04	19.48	<u>sq. ft.</u>		0	0		0	0
Windows:	1" plyw'd	5.83	221	each		Ō	0		0	Ō
6' X 11'	1/64" met	0.09	<u>44.7</u> 265.7	each each		0 0	0 0		0	0 0
Total square feet/pou	inds:	0.02	200.1			49520.94	-	······································	31066.4	1987313
Total cubic yards: Total Tons:	•					1834.256 4375.737			1150.699 993.6563	

Table 11-3 (cont.)

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Aerovox, Inc. Facility

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Materials to TSCA Landfill Under Option #2 (Including a Portion of the Concrete Floor at Grade) Eastern Section

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Basic Units:						1st Floor			2nd Floor	-		3rd Floor	-
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [Ib]	Unit	No. of Units	Volume [cf]	Mass [ib]	No. of Units	Volume [cf]	Mass [ib]	No. of Units	Volume [cf]	Mass [ib]
Base Wall:	1' thick	3	540	lin. ft.		0	0		0	0		0	0
						0	0		0	0		0	0
Brick Walls:	12" thick	1	112	sq. ft.		0	0		0	0		0	Ö
	16" thick	1.333	150	sq. ft.		0	0		0	0		<u> </u>	0
						0	0		0	0		0	0
Wooden Walls/Floor:	4" thick	0.333	9	sq. ft.	35 6 4	1186.812	32076		0	0		0	0
	5" thick	0.416	11.25	sq. ft.		0	0	86182	35851.71		86182	35851.71	969547.5
	_		-			0	0		0	0		0	0
Drywall:	9' high	0.91	36.5	lin. ft.		0	Q		0	0		0	0
2"X4" stud every 2 ft	10' high	1.01	40.5	lin. ft.		0	0		0	0		0	0
	12' high	1.22	48.7	<u>lin. ft.</u>		0	0		0	0		0	0
						0	0		0	Ō		0	0
Particle Board Wall:	10' high	1.01	36.4	lin. ft.		_	_		0	0		0	0
0.5" thick board	12' high	1.22	43.68	lin. ft.		0	0		0	0		0	0
2"X4" stud every 2"	16' high	1.62	58.24	<u>lin. ft.</u>		0	0		0	0		<u> </u>	0
							•		0	0		0	0
Wooden Columns	9' high	3.14	138	each		0	0		0	0		0	0
8" diameter	10' high	3.5	154	each		0	0		0	0		0	0
	12' high	4.18	184	each		0	0		0 0	0		0	0
_	16' high	5.6	246	each		0	0		U	0		0	0
	17' high	5.95	261.8	each				<u></u>		;		0	0
Steel Beams:	W21 x 62	0.127	62	lin. ft.		0	0	·····	0	0		Ŏ	0
Steel Plate:	0.5" thick	0.04	19.48	sq. ft.		0	0		0	0		0	0
									0	0		0	0
Windows:	1" plyw'd	8.91	338	each		0	0		0	0.		0	0
8' X 13'	1/64" met	0.14	68	each		0	0		0	0		0	0
		9.05	406	each		0	0		0	0		0	0
Total square feet/pou	inds:					1186.812	32076		35851.71	969547.5		35851.71	969547.5
Total cubic yards: Total Tons:						43.95952 16.038			1327.947 484.7738			1327.947 484.7738	
TOTAL TSCA MATE				5684.809 cubic yard 6354.979 Tons	5								

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Table 11-4

Aerovox, Inc. Facility

Materials to TSCA Landfill Under Option #3 (Including Entire Concrete Floor at Grade)

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				I			Western	Section:	-	
Basic Units:						1st Floor			2nd Floor	-
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [ib]	Unit	No. of Units	Volume [cf]	Mass [Ib]	No. of Units	Volume [cf]	Mass [lb]
Base Concrete Wall:	1' thick	3	540	lin. ft.		0	0		0	0
Concrete Floor:	6" thick	0.5	90	<u>sq. ft.</u>	96920	48460	8722800	15000	7500	1350000
Brick Walls:	12" thick	1	112	sq. fl.		0	0		0	0
	16" thick	1.333	150	sq. ft.		0	0		0	0
· · · · · · · · · · · · · · · · · · ·						0	0		0	0
Wooden Walls/Floor:	4" thick	0.333	9	sq. ft.	3186	1060.938	28674		0	0
	5" thick	0.416	11.25	sq. ft.		0	0	56650	23566.4	637312.5
						0	0		0	0
Drywall:	9' high	0.91	36.5	lin. ft.		0	0		0	0
2"X4" stud every 2'	10' high	1.01	40.5	lin. ft.		0	0		0	0
·	12' high	1.22	48.7	lin. ft.		0	0		0	0
						0	0		0	0
Wooden Columns	9' high	3.14	138	each		0	0		0	0
6" diameter	10' high	3.5	154	each		0	0		Ð	0
-	12" high	4,18	184	each		0	0		0	0
	16' high	5.6	246	each		0	0		0	0
	_								0	0
Steel Beams:	W21 x 62	0.127	62	lin. ft.		0	0		0	0
			·····			0	0		0	0
Steel Plate	0.5" thick	0.04	19.48	sq. ft.		0	0		0	0
						0	0		0	0
Windows:	1" plyw'd	5.83	221	each		0	0		0	0
6' X 11'	1/64" met	0.09	44.7	each		0	0		0	0
		5.92	265.7	each		0	00		0	0
Total square feet/pou	inds:	<u></u>				49520.94	8751474		31066.4	1987313
Total cubic yards: Total Tons:						1834.256 4375.737	cu. yds. Tons		1150.699 993.6563	•

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Eastern Section



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Table 11-4 (cont.)

Aerovox, Inc. Facility

Materials to TSCA Landfill Under Option #3 (Including Entire Concrete Floor at Grade)

Volume Per Unit [cf] 3 0.5 1 1.333 0.333 0.416 0.91 1.01 1.22 1.01 1.22	Mass Per Unit [ib] 540 90 112 150 9 11.25 36.5 40.5 48.7 36.4	Unit L lin. ft. sq. ft. 8 sq. ft. sq. ft.	lo. of Inits 5214 3564	1st Floor Volume [cf] 0 42607 0 0 0 1186.812 0 0 0 0 0 0 0 0 0 0 0 0 0	Mass [lb] 0 7669260 0 0 0 0 32076 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	No. of Units 86182	2nd Floor [cf] 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Mass [Ib] 0 0 0 0 0 0 0 969547.5 0 0	<u>3rd Flo</u> No. of Volum Units [cf]	Mass [lb] 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1 969547.5 0
Per Unit [cf] 3 0.5 1 1.333 0.333 0.416 0.91 1.01 1.22 1.01	Per Unit [ib] 540 90 112 150 9 11.25 36.5 40.5 48.7	Unit L lin. ft. sq. ft. 8 sq. ft. 8 8 sq. ft. 1 1 lin. ft. 1 1	5214	[ct] 0 42607 0 0 0 0 1186.812 0 0 0	[ib] 0 7669260 0 0 0 0 32076 0 0 0 0 0	Units	[cf] 0 0 0 0 0 35851.71	[ib] 0 0 0 0 0 0 0 0	Units [cf]	[lb] 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1 969547.5
0.5 1 1.333 0.333 0.416 0.91 1.01 1.22 1.01	90 112 150 9 11.25 36.5 40.5 48.7	sq. ft. 8 sq. ft. sq. ft. sq. ft. 3 sq. ft. 3 lin. ft. lin. ft.	····	42607 0 0 0 1186.812 0 0 0	7669260 0 0 0 32076 0 0 0 0	86182	0 0 0 0 35851.71	0 0 0 0 0 0		0 0 0 0 0 0 0 0 0 0 0 0 1 969547.5 0 0
1 1.333 0.333 0.416 0.91 1.01 1.22 1.01	112 150 9 11.25 36.5 40.5 48.7	sq. ft. sq. ft. sq. ft. : sq. ft. lin. ft. lin. ft.	····	0 0 0 1186.812 0 0 0	0 0 0 32076 0 0 0	86182	0 0 0 0 35851.71 0	0 0 0 0 0		0 0 0 0 0 0 0 0 0 0 1 969547.5 0 0
1.333 0.333 0.416 0.91 1.01 1.22 1.01	9 11.25 36.5 40.5 48.7	sq. ft. sq. ft. sq. ft. : sq. ft. lin. ft. lin. ft.	3564	0 0 1186.812 0 0 0	0 0 32076 0 0 0	86182	0 0 0 35851.71 0	0 0 0 0		0 0 0 0 0 0 0 0 1 969547.5 0 0
1.333 0.333 0.416 0.91 1.01 1.22 1.01	9 11.25 36.5 40.5 48.7	sq. ft. sq. ft. : : sq. ft. lin. ft. lin. ft.	3564	0 0 1186.812 0 0 0	0 0 32076 0 0 0	86182	0 0 0 35851.71 0	0 0 0		0 0 0 0 0 0 <u>1 969547.5</u> 0 0
0.333 0.416 0.91 1.01 1.22 1.01	9 11.25 36.5 40.5 48.7	sq. ft	3564	0 1186.812 0 0 0	0 32076 0 0 0	86182	0 35851.71 0	0	86182 358517	0 0 0 0 <u>1 969547.5</u> 0 0
0.416 0.91 1.01 1.22 1.01	11.25 36.5 40.5 48.7	sq. fl. lin. fl, lin. fl,	3564	1186.812 0 0 0	32076 0 0 0	86182	0 35851.71 0	ō	86182 35851 7	0 0 <u>1 969547.5</u> 0 0
0.416 0.91 1.01 1.22 1.01	11.25 36.5 40.5 48.7	sq. fl. lin. fl, lin. fl,		0 0 0	0 0 0	86182	<u>35851.71</u> 0	0 969547.5 0 0	86182 35851 7	<u>1 969547.5</u> 0 0
0.91 1.01 1.22 1.01	36.5 40.5 48.7	lin. ft, lin. ft,		0	0	80102	Ó	909547.5 0 0	00102 30001.4	0 0
1.01 <u>1.22</u> 1.01	40.5 48.7	lin. ft.		ō	0		-	0		
1.01 <u>1.22</u> 1.01	40.5 48.7	lin. ft.		-						vv
<u>1.22</u> 1.01	48.7						ก	0		0 0
1.01	· · · ·	······		0	Ō		Õ	õ		0 0
	26.4			0	0	······	<u>ō</u>	0		0 0
	30.4	lin. ft.		•	•		ŏ	ō		ō ō
1.22	43.68	lin. fi.		0	0		ō	ō		ōō
1.62	58.24	lin. ft.		0	0		0	0		o o
					·		0	0		0 0
3.14	138	each		0	0		0	0		00
3.5	154	each		0	0		Ō	0		00
	184	each		0	0		0	0		00
	246	each		0	0		0	0		00
5.95	261.8	each								0 0
2 0.127	62	lin. ft.		0	0		0	0		0 0 0 0
0.04	19.48	sq. ft.		0	0		0	0		0 0
						· · · · · · · · · · · · · · · · · · ·	0	0		0 0
		each		0	-		0	0		00
				•	-		•	0		00
9.05	406	each			•		<u>v</u>	0		0 0
				43793.81	7701336		35851.71	969547.5	35851.7	1 969547.5
										7 cu. yds. 9 Tone
	3.5 4.18 5.6 5.95 2 0.127	3.5 154 4.18 184 5.6 248 5.95 261.8 2 0.127 62 x 0.04 19.48 3 8.91 338 1 0.14 68 9.05 406	3.5 154 each 4.18 184 each 5.6 248 each 5.95 261.8 each 2 0.127 62 lin. ft. x 0.04 19.48 sq. ft. 3 8.91 338 each 9.05 406 each 9.05 406 each	3.5 154 each 4.18 184 each 5.6 248 each 5.95 261.8 each 2 0.127 62 lin. ft. x 0.04 19.48 sq. ft. 3 8.91 338 each at 0.14 68 each 9.05 406 each	3.5 154 each 0 4.18 184 each 0 5.6 248 each 0 2 0.127 62 lin. ft. 0 2 0.127 62 lin. ft. 0 x 0.04 19.48 sq. ft. 0 x 0.04 19.48 sq. ft. 0 x 0.14 68 each 0 9.05 406 each 0 43793.81 1622.123 3850.668 0 DLUME: 7262.973 cubic yards	3.5 154 each 0 0 4.18 184 each 0 0 5.6 248 each 0 0 2 0.127 62 lin. ft. 0 0 x 0.04 19.48 sq. ft. 0 0 x 0.04 19.48 sq. ft. 0 0 3 8.91 338 each 0 0 3 8.91 338 each 0 0 4 0.14 68 each 0 0 9.05 406 each 0 0 0 Hight 2.123 cu. yds. 3850.668 Tons	3.5 154 each 0 0 4.18 184 each 0 0 5.6 248 each 0 0 2 0.127 62 lin. ft. 0 0 2 0.127 62 lin. ft. 0 0 x 0.04 19.48 sq. ft. 0 0 x 0.04 19.48 sq. ft. 0 0 x 0.04 19.48 sq. ft. 0 0 x 0.14 68 each 0 0 y 0.5 406 each 0 0 43793.81 7701336 1622.123 cu. yds. 3850.668 DLUME: 7262.973 cubic yards 3850.668 1622.123	3.5 154 each 0 0 0 4.18 184 each 0 0 0 5.6 248 each 0 0 0 2 0.127 62 lin. ft. 0 0 0 4 0.04 19.48 sq. ft. 0 0 0 4 0.14 68 each 0 0 0 4 0.14 68 each 0 0 0 43793.81 7701336 35851.71 1622.123 cu. yds. 1327.947 3850.668 Tons 484.7738 484.7738 184.7738	3.5 154 each 0 0 0 0 4.18 184 each 0 0 0 0 5.6 248 each 0 0 0 0 2 0.127 62 lin. ft. 0 0 0 0 2 0.127 62 lin. ft. 0 0 0 0 2 0.127 62 lin. ft. 0 0 0 0 2 0.127 62 lin. ft. 0 0 0 0 4 0.04 19.48 sq. ft. 0 0 0 0 4 0.14 68 each 0 0 0 0 9.05 406 each 0 0 0 0 0 43793.81 7701336 35851.71 969547.5 1622.123 cu. yds. 1327.947 cu. yds. 3850.668 Tons 484.7738 Tons 484.7738 Tons	3.5 154 each 0 0 0 0 0 4.18 184 each 0 0 0 0 0 5.6 248 each 0 0 0 0 0 5.95 261.8 each 0 0 0 0 0 2 0.127 62 lin. ft. 0 0 0 0 2 0.127 62 lin. ft. 0 0 0 0 2 0.127 62 lin. ft. 0 0 0 0 4 0.04 19.48 sq. ft. 0 0 0 0 4 8.91 338 each 0 0 0 0 9.05 406 each 0 0 0 0 0 9.05 406 each 0 0 0 0 0 1622.123 cu. yds. 1327.947 cu. yds. 1327.94 1327.94 3850.668 Tons

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Table 11-5

Aerovox, Inc. Facility

Materials to Non-TSCA Landfill Under Option #1

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							Western	Section:	-	
Basic Units:						1st Floor			2nd Floor	
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [lb]	Unit	No. of Units	Volume [cf]	Mass [Ib]	No. of Units	Volume [cf]	Mass [lb]
Base Concrete Wall:	1' thick	3	540	lin. ft.	1432	4296	773280		0	0
Concrete Floor:	6" thick	0.5	90	sq. ft.		0	0		0	0
Brick Walls:	12" thick 16" thick	1 1.333	112 150	sq. ft. sq. ft.	5064 13239	5064 17647.59	567168 1985850	3006 4704	3006 6270.432	336672 705600
	10 01100	1.000				0	0		0	0
Wooden Walls/Floor	4" thick 5" thick	0.333 0.416	9 11.25	sq. ft. sq. ft.	2800	932.4 0	25200 0	25000	0 10400	0 281250
· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·					Ō	0		0	0
Drywall:	9' high	0.91	36.5	lin. ft.	1100	1001	40150		0	0
2"X4" stud every 2'	10' hìgh	1.01	40.5	lin. ft.	180	181.8	7290	2500	2525	101250
<u> </u>	12' high	1.22	48.7	lin. ft.	550	671	<u>26785</u> 0		0	0
Wooden Columns	0 blab	.3.14	138	each	176	0 552.64	24288		0	0
8" diameter	9' high 10' high	3.14	154	each	25	87.5	3850		Ő	ŏ
	12" high	4.18	184	each	108	451.44	19872		ŏ	õ
	16' high	5.6	246	each	100	0	0	84	470.4	20664
						- ··			0	0
Steel Beams:	W21 x 62	0.127	62	lin. ft.		0	0		0	0
						Ö	0		0	0
Steel Plate	0.5" thick	0.04	<u>19.48</u>	<u>sq. ft.</u>		0	0		0	0
						0	0		0	0
Windows:	1" plyw'd	5.83	221	each		0	0	26	151.58	5746
6' X 11'	1/64" met	0.09	<u>44.7</u> 265.7	each each		0	0		0	0 0
Total square feet/po	unds:	5.92	205.7	Bach		30885.37	•		22823.41	-
Total cubic yards: Total Tons:						1143.994 1736.867			845.3792 725.591	-

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Roof - Western Section:

cubic yards 1875 658

Tons

Table 11-5 (cont.)

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Aerovox, Inc. Facility

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Materials to Non-TS	CA Landi	ill Under (Option #1				<u> </u>	stern Sect	ion					
Basic Units:						1st Floor			2nd Floor		3rd Floor			
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [lb]	Unit	No. of Units	Volume {cf}	Mass [lb]	No. of Unite	Volume [cf]	Masa (ib)	No. of Units	Volume [cf]	Mass [Ib]	
Base Wall:	1' thick	3	540	lin. ft.	1425	4275	769500		0	0		0		
		.				0	0		0	0		0	C	
Brick Walls:	12" thick	1	112	sq. ft.	2246	2246	251552	2325	2325	260400	3525	3525	394800	
<u></u>	16" thick	1.333	150	sq. ft.	4194	5590.602		7650	10197.45		<u> </u>	10818.63		
			-			0	0		0	0		0	~ (
Wooden Walls/Floor		0.333	9	s q. ft.		0	0		0	0		0	0	
	5" thick	0.416	11.25	sq. ft.		0	0		0	0		0	0	
D		0.04	36.6	11		0	0		0	0		0	0	
Drywall:	9' high	0.91	36.5 40.5	lin. ft.		0	0		0	0		0	0	
2"X4" stud every 2 ft		1.01 1.22	40.5	lin. ft. lin. ft.		0	ő		ŏ	0		Ö	C	
· · · · · · ·	12 high	1.22			·····	<u> </u>	<u> </u>				<u>_</u>	0		
Particle Board Wall:	10' high	1.01	36.4	lin. ft.		v	U	2365	2388.65	86086	2320	2343.2	84448	
0.5" thick board	12' high	1.22	43.68	lin, ft.	3100	3782	135408		0	0		0	C	
2"X4" stud every 2'	16' high	1.62	58.24	lin. ft.	•••••	0	0		Ō	Ō		Ō	C	
	To high				^				0	0	<u></u>	0	0	
Wooden Columns	9' high	3.14	138	each		0	0		0	0		0	C	
8" diameter	10' high	3.5	154	each		0	0		0	0		0	0	
	12' high	4.18	184	each	220	919.6	40480		0	0		0	0	
	16' high	5.6	246	each		0	0	220	1232	54120		0	0	
	17 high	5.95	261.8	each							220	1309	57596	
Steel Beams:	W21 x 62	0.127	62	lin, ft.		0	0		0	0		0	C C	
Steel Plate:	0.5" thick	0.04	19.48	sq. ft.		Q	0		0	0		0		
									0	0		0	0	
Windows:	1" plyw'd	8,91	338	each	56	498.96	18928	119	1060.29	40222	119	1060 29	40222	
8' X 13'	1/64" met	0,14	68	each		0	0		0	0		0	0	
		9.05	406	each		0	0		0	0		0 19056 12	0	
Total square feet/pou	nds:					17312.16	1044900		17203.39	1000020		19030.12	1194400	
Total ouble verder						641.2425	cu vds		637.2136	cu, vda.		705.8386	cu, vds.	
Total cubic yards: Total Tons:						922.484	Tons		794.164	Tons		897.233		
Roof - Eastern Sect	ion:	1474 517	cubic yard	is										

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 TOTAL NON-TSCA MATERIAL VOLUME:
 7322.668 cubic yards

 TOTAL NON-TSCA MATERIAL MASS:
 6251.339 Tons

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Table 11-6

Aerovox, Inc. Facility

Materials to Non-TSCA Landfill Under Options #2 and #3

Desta Alasta							Western	Section:		
Basic Units:						1st Floor	-		2nd Floor	
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [ib]	Unit	No. of Units	Volume [cf]	Mass [ib]	No. of Units	Volume [cf]	Mass [lb]
Base Concrete Wall: Concrete Floor:	1' thick 6" thick	3 0.5	540 90	fin. ft. sq. ft.		0 0	0		0	0
Brick Walis:	12" thick 16" thick	1 1.333	112 150	sq. ft. sq. ft.		0 0	0 0		0 0	0 0
Wooden Walls/Floor	4" thick 5" thick	0.333 0.416	9 11.25	sq. fl. sq. fl.	2800	0 932.4 0	0 25200 0	25000	0 0 10400	0 0 281250
Drywall: 2"X4" stud every 2'	9' high 10' high 12' high	0.91 1.01 1.22	36.5 40.5 48.7	lin. ft. lin. ft. lin. ft.	1100 180 550	0 1001 181.8 671	0 40150 7290 26785	2500	0 0 2525 0	0 0 101250 0
Wooden Columns 8" diameter	9' high 10' high 12" high	3.14 3.5 4.18 5.6	138 154 184 246	each each each	176 25 108	0 552.64 87.5 451.44 0	0 24288 3850 19872 0	84	0 0 0 0 470.4	0 0 0 0 20664
Steel Beams:	16' high W21 x 62	<u> </u>	62	lin. ft.		0	0	04	0	0
Steel Plate	0.5" thick	0.04	19.48	sq. ft.		0 0 0	0 0 0		0	0 0
Windows: 6' X 11'	1" plyw'd 1/64" met	5.83 0.09 5.92	221 44.7 265.7	each each each		0 0 0	0 0 0	26	0 151.58 0 0	5746 0 0
Total square teet/po	unds:					3877.78	147435		13546.98	_
Total cubic yards: Total Tons:						143.633 73.7175	cu. yds. Tons		501.7801 204.455	

Roof - Western Section:

1875 cubic yards 658 Tons

Table 11-6 (cont.)

Aerovox, Inc. Facility

Materials to Non-TS	CA Landf	ill Under C	Options #2	and #3			Ea	ion							
Basic Units:						1st Floor			2nd Floor			3rd Floor			
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [Ib]	Unit	No. of Units	Volume [cf]	Mass [ib]	No. of Units	Volume [cf]	Mass [ib]	No. of Units	Volume [cf]	Mass {ib]		
Base Wall:	1' thick	3	540	lin. ft.		0	0		0	0		0			
						<u> </u>			<u> </u>	0		0			
Brick Walls:	12" thick	1	112	sq. ft.		0	0		0	0		0	(
	16" thick	1.333	150	sq. ft.	•	0	0		0	0		0	(
						0	_0		0	0		0	-		
Wooden Walls/Floor	4" thick	0.333	9	sq. ft.		0	0		0	0		0			
	5" thick	0.416	11.25	sq. ft.	_ <u></u>	0	0		0	0		0			
.						0	0		- 0	0		0	(
Drywall:	9' high	0.91	36.5	lin. ft.		U	0		0	0 0		0	Č		
2"X4" stud every 2 ft		1.01	40.5	lin. ft.		U	0		0	0		0	Č		
·	12' high_	1.22	48.7	lin, ft.		0	0		<u> </u>			0	<u> </u>		
Particle Board Wall:	10' high	1.01	36.4	lin. ft,		U	U	2365	2388.65	86086	2320	2343.2	84448		
0.5" thick board	12' high	1.22	43.68	lin. R.	3100	3782	135408	2000	2300.03	0	2520	2040.2	(
2"X4" stud every 2'	12 high	1.62	43.00 58.24	lin. ft.	5100	0	0		ŏ	ŏ		ŏ			
Z NA BIOD BARRY Z	to ngu	1.02	50.24	<u></u>		V			ō	<u> </u>		<u> </u>			
Wooden Columna	9' high	3.14	138	each		0	0		Ō	Õ		Ō	Ó		
8" diameter	10' high	3.5	154	each		õ	ō		Õ	0		0	(
	12' high	4.18	184	each	220	919.6	40480		0	0		0	(
	16' high	5.6	246	each		0	0	220	1232	54120	•	0			
	17' high	5.95	261.8	each		_					220	1309	57596		
												Ó			
Steel Beams:	W21 x 62	0.127	62	lin. ft.		0	0	· · · ·	0	0		0	(
Steel Plate:	0.5" thick	0.04	19.48	sq. ft.		. 0	0		0	0		0	(
									0	0		0	(
	1" plyw'd	8.91	338	each	56	498.96	18928	119	1060.29	40222	119	1060.29	40222		
8' X 13'	1/64" met	0.14	68	each		0	0		0	0		0	(
		9.05	406	each		0	0		0	0		0	(
Total square feet/pou	nds					5200.56	194816		4680.94	180428		4712.49	182260		
Total cubic yards:						192.6287 97.408			173.382 90.214			174.5506 91.133			
Total Tons:		·· ·· ·			<u> </u>	#1,900		······································							
Roof - Eastern Sect	ion:	1474 517	cubic yard Tons	is											

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 TOTAL NON-TSCA MATERIAL VOLUME:
 4534.975 cubic yards

 TOTAL NON-TSCA MATERIAL MASS:
 1731.928 Tons

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Table 11-7

Aerovox, Inc. Facility

Non-TSCA Materials to be used as Backfill Under Options #2 and #3

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							Western	Section:	=	
Basic Units:						1st Floor			2nd Floor	-
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [Ib]	Unit	No. of Units	Volume [cf]	Mass [ib]	No. of Units	Volume [cf]	Mass [Ib]
Base Concrete Wall:	1' thick	3	540	lin. ft.	1432	4296	773280		0	0
Concrete Floor:	6" thick	0.5	90	<u>sq. ft.</u>	,,	0	0		0	0
Brick Walls:	12" thick 16" thick	1 1.333	112 150	8q. ft.	5064 13239	5064 17647.59	567168 1985850	3006 4704	3006 6270.432	336672 705600
	ID THICK	1.333	150	sq. fl	13238	0	0	4/04	0270.432	0000
Wooden Walls/Floor	4" thick	0.333	9	sq. fl.		ō	0		ō	Ō
	5" thick	0.416	11.25	sq. fl.		0	0		0	0
						0	0		0	0
Drywall:	9' high	0.91	36.5	lin. ft.		0	0		0	0
2"X4" stud every 2'	10' high	1.01	40.5	lin. ft.		0	0		0	0
	12' high	1.22	48.7	lin. ft.		0	0		0	0
Wooden Columns	9' high	3.14	138	each		0	0		0	Ő
8" diameter	10' high	3.14	154	each		ŏ	õ		ŏ	ŏ
	12" high	4.18	184	each		Ö	õ		Ō	ō
	16' high	5.6	248	each		ŏ	õ		ō	Ō
······································							···		0	0
Steel Beams:	W21 x 62	0.127	62	lin. ft.		0	0		0	0
·						0	0		0	0
Steel Plate	0.5" thick	0.04	19.48	sq. ft.		0	0		0	0
				•		0	0		0	0
Windows:	1" plyw'd	5.83	221	each		0	0		0	0
6' X 11'	1/64" met	0.09	44.7	each		0	0		0	0 0
Total square feet/poi	inds:	5.92	265.7	each		27007.59			9276.432	_
										-
Total cubic yards: Total Tons:						1000.361 1663.149			343.599 521.136	cu. yds Tons

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Table 11-7 (cont.)

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Aerovox, Inc. Facility

Non-TSCA Materials to be used as Backfill Under Options #2 and #3							т.	<u> </u>	stem Sect	ion				
Basic Units:						1st Floor			2nd Floor		3rd Floor			
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [[b]	Unit	No. of Units	Volume [cf]	Mass [Ib]	No. of Units	Volume [cf]	Mass [Ib]	No. of Units	Volume [cf]	Mass [lb]	
Base Wall:	1' thick	3	540	lin. ft.	1425	4275	769500		0	0	_	0	(
Brick Walls:	12" thick 16" thick	1 1.333	112 150	8q. ft. 8q. ft.	2246 4194	0 2246 5590.602		2325 7650	0 2325 10197.45	0 260400 1147500	3525 8116	0 3525 10818.63		
Wooden Walls/Floor	4" thick 5" thick	0.333 0.416	9 <u>11.25</u>	sq. ft. sq. ft.		0 0 0	0 0 		0	0		0	- (
Drywall: 2"X4" stud every 2 ft	9' high 10' high 12' high	0.91 1.01 1.22	36.5 40.5 48.7	lin. ft. lin. ft. lin. ft		0 0 0	0 0 0 0		0 0 0	0 0 0		000000000000000000000000000000000000000		
Particle Board Wall: 0.5" thick board 2"X4" stud every 2"	10' high 12' high 16' high	1.01 1.22 1.62	36.4 43.68 58.24	lin. fl. lin. fl. lin. fl.		0 0 0	0 0 0	<u> </u>	0 0 0	0 0 0 0		0 0 0	(
Wooden Columns 8" diameter	9' high 10' high 12' high 16' high	3.14 3.5 4.18 5.6	138 154 184 246	each each each each		0 0 0 0	0 0 0 0		0 0 0 0	0 0 0 0		0 0 0 0		
Steel Beams: "	<u>17' high</u> W21 x 62	<u>5.95</u> 0.127	<u>261.8</u> 62	each lin. fl.		0	_0		0	0	·_ · · · · · · · · · · · · · · · · · ·	0 0 0	(
Steel Plate:	0.5" thick	0.04		sq. ft.		0	0	· · · · · · · · · · · · · · · · · · · ·	0			0	(
Windows: 8' X 13'	1" plyw'd 1/64" met	8.91 0.14 9.05	338 68 406	each each each		0 0 0	0 0 0		0 0 0	0 0 0		0 0 0 0	(
Total square teet/poi Total cubic yards: Total Tons:	unds:					12111.6 448.6137 825.076			12522.45 463.8315 703.95	cu. yds.		14343.63 531.288 806.1	cu. yds.	

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Roof - Eastern Section:

cubic yards Tons

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TOTAL NON-TSCA BACKFILL MATERIAL VOLU 2787.693 cubic yards TOTAL NON-TSCA BACKFILL MATERIAL MASS: 4519.411 Tons

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Table 11-8

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Aerovox, Inc. Facility

Materials to Steel Smelting Facility

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0							Western	n Section:		
Basic Units:						1st Floor	-		2nd Floor	-
Structure	Size	Volume Per Unit [cf]	Mass Per Unit [lb]	Unit	No. of Units	Volume [cf]	Mass [ib]	No. of Units	Volume [cf]	Mass [lb]
Base Concrete Wall:	1' thick	3	540	lin. ft.		0	0		0	0
Concrete Floor:	6" thick	0.5	90	sq. ft.		00	0		0	0
Brick Walls:	12" thick 16" thick	1 1.333	112 150	sq. ft. sq. ft.		0	0 0		0	0 0
		1.555	100	əy. ii.			<u> </u>	· <u> </u>	0	<u>0</u>
Wooden Walls/Floor:	4" thick 5" thick	0.333 0.416	9 11.25	sq. ft. sq. ft.		0	0		0	0
					<u> </u>	0	0		0	0
Drywall:	9' high	0.91	36.5	lin. ft.		0	0		0	0
2"X4" stud every 2'	10° high	1.01	40.5	lin. ft.		0	0		0	0
	12' high	1.22	48.7	<u>iin. ft.</u>		0	0		0	0
						0	0		0	0
Wooden Columns	9' high	3.14	138	each		0	0	•	0	0
8" diameter	10' high	3.5	154	each		0	0		0	0
-	12" high	4.18	184	each		0	0		0	0
	<u>16' high</u>	5.6	246	each		0	0		0	0
Steel Beams:	W21 x 62	0.127	82	lin. ft.	9320	1183.64	577840	4583	0 582.041	0 284146
						0	0		0	0
Steel Plate	0.5" thick	0.04	19.48	sq. ft.		0	0	3925	157	76459
						0	0	-	0	0
Mindows:	1" piyw'd	5.83	221	each		0	0		0	0
6' X 11'	1/64" met	0.09	44.7	each		0	0	26	2.34	1162.2
		5.92	265.7	each		0	0		0	0
Total square feet/pou	inds:					1183.64	577840		741.381	361767.2
Total cubic yards: Total Tons:						43.84203 288.92	cu. yds. Ton s		27.46075 180.8836	

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Table 11-8 (cont.)

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Aerovox, Inc. Facility

Materials to Steel S	melting Fa	cility						EI	stern Sect	ion			
Basic Units:						1st Floor			2nd Floor			3rd Floor	
Structure	Size	Volume Per Unit [cf]	Mass Per Unit {ib]	Unit	No. of Units	Volume [cf]	Mass [ib]	No. of Units	Volume [cf]	Mass [ib]	No. of	Volume [cf]	Mass [ib]
Base Wall:	- 1' thick	3	540	lin. ft.		0	0		0	0	<u> </u>	0	<u> </u>
Brick Walls:	12" thick 16" thick	1 1.333	112 150	sq. fl. sq. fl.		0 0 0	0 0 0		0 0 0	0 0 0		- 0	0 0
Wooden Walls/Floor:	4" thick 5" thick	0.333 0.416	9 11.25	sq. ft. sq. ft.		0 0 0	0 0 0		0 0 0	0 0 0		0 0 0	0 0 0
Drywall: 2"X4" stud every 2 ft	9' high 10' high 12' high	0.91 1.01 1.22	36.5 40.5 48.7	lin. ft. IIn. ft. lin. ft.		0 0 0 0	0 0 0 0		0 0 0 0	0 0 0		0 0 0	0 0 0
Particle Board Wall: 0.5" thick board	10' high 12' high	1.01	36.4 43.68	lin. ft. lin. ft.		0 0	0		0	0		000000000000000000000000000000000000000	0000
2"X4" stud every 2' Wooden Columns	16' high 9' high	<u>1.62</u> 3.14	<u>58.24</u> 138		<u> </u>	<u> </u>	<u>0</u> 0	<u></u>	0 0 0	0 0 0		0 0 0	0 0 0
8" diameter	10' high 12' high 16' high	3.5 4.18 5.6	154 184 246	each each each		0 0 0	0 0. 0		0 0 0	0 0 0		0 0 0	0 0 0 0
Steel Beams:	<u>17' high</u> W21 x 62	<u>5.95</u> 0.127	261.8 62	each	7535	956.945	467170	7535	956.945	467170	7535	0 956.945	0
Steel Plate:	0.5" thick	0.04	19.48	sq. fi		0	0	4728	<u>189.12</u> 0	<u>92101.44</u> 0		0	<u> </u>
Windows: B' X 13'	1" plyw'd 1/64" met	8.91 0.14 9.05	338 68 406	each each each	56	0 7.84 0	0 3808 0	119	0 16.66 0	0 8092 0	119	0 16.66 0	8092 0
Total square feet/pou	inds:					954.785	470978			567363.4		973.605	
Total cubic yards: Total Tons:						35.73564 235.489			43.06733 283.6817	cu, yds. Tons		36.05233 237.631	
TOTAL STEEL VOL				186.1681 cubic yard 1226.605 Tons	5								

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D. SOUCK & LEF. NC BUECT DATE Aerovox, Inc. Facility - building Denslitun Alternative 03855.004 3/25/98 1/4 CEG JCB ; DATE 3/26/98 (ALCS. BY _____; DATE ____; CHECKED BY Calculations for Connete Floor Slab: cea 622 325 A26= AZA= A. = 126 = 12A 3 V. 7 220 337 285 e kus 3445 0'665 4665 Area Calculations: A1= A. =(220)(325) = 71500 sf √3= 107 H2=(622)(137) = 85,214 st AzA = (285)(137') = 39.045 5+ 158 A26 = (337 X137 = 46,169 st A3 = (103') 46 = 4.738 st se of Concrete Floor below Grade Ay = (107') (158' - 16,906 sf Assumptions: Aras A. As, Ay, A5 beb Gone 6' As = (20 X 80) + (36)(56) = 3,776 sf Area ADA below Grande 3.5' Acca A26 below Grade 1.5' (= (71,500 sf) (6') = 429,000 cf = 15,858.9 cy HTOTAL = 182, 134 st (excluding) = 20,237 541 √2p = (39,045,51) (3.5') = 136,657.5cf = 5,061.4 cy $V_{2R} = (46, 169 \text{ s}^2)(15) = 69, 253.5 \text{ c}^2 = 2,564.9 \text{ cy}$ V2 = V2+ +V2e = 136,657.5cf +69,253.5cf = 205,911 cf = 7,626.3cy $V_3 = (4,733 \text{ sf})(6') = 28,429 \text{ cf} = 1052.9 \text{ cr}$ Vy = (16,906) (6) = 101,436 cf = 3756.9 cr ■V5 = (3,776) (6) = 22,656 cf = 839.1 cr TOTAL = 787.431 cf = 29, 164.1 cy v

- Acrovor, Inc. Facility - Building Demolition Altarnative 03955004 CEG 3/25/18 CHECKED BY JCB ; DATE 3/26/98 4 ALCS. BY _____; DATE _____ - Amount of Backfill Required: Thickness of Cap to be Installedi 6" sand + 6" genel + 4" asphalt = 1'4" _ Assumption: · Building Area will be backfilled up to I toot below Grade with the remaining volume to be filled with cap material. - Concrete Floor Slab Thickness is Ginches Option #1: Volune of Denolition Matrials to be Used as Backfill Under Volume of Backfill Required Optims #2 and #3 is 2787.704 $= \sqrt{\sqrt{71500}(5') + (39,045)(2.5') + (46,169)(0.5) + (4738)(5) +$ (16,906)(5) + (3776)(5) = 605,297 cf = 22,448.4 cy / Area of Concrete to be ferrived $A_{concentration} = A_{, +} + A_{3} + A_{4} + A_{5} = (71,500) + (4,733) + (16,906) + (3,776) = 96,920 = 1$ - Volume of Fill Required Vor +2 = (71,50) (5+05) + (4,738) (5+05) + (16,506) 5+05) + (3776) (5+05) + (39045) (25) + (46169) (05) = 653,757 CF = 24,213,2 CY Option #31 2,787.7 64 VOPT#2 = 21,425.5 CY Aren of Concrete to be Removed Aconcernon = A + A2A+A2B+A3+Ay+A5 = ATOTAL = 182, 134 sf Volone . + Fill Required VOPTH3 = (71500) (5+05) + (39,045) (25+05) + (46,169) (05+05) + (4730) (5+0.5) + (16,906)(510,5) + (3776) 5+0.5) = 696,364 CF = 25, 711.3 CY 2 787 7CX Vort#3 = 23,003.6 CY J



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BLASLAND, BOUCK & LEE, INC.

engineers & scientists